07

STATE OF CALIFORNIA

DEPARTMENT OF TRANSPORTATION

NOTICE TO BIDDERS

AND

SPECIAL PROVISIONS

FOR CONSTRUCTION ON STATE HIGHWAY IN LOS ANGELES COUNTY IN LA CANADA FLINTRIDGE, GLENDALE AND PASADENA FROM DUNSMORE AVENUE UNDERCROSSING TO NORTH LOS ROBLES AVENUE OVERCROSSING

In District 07 On Route 210

Under

***Bid* book dated July 21, 2014**	***Standard Specifications* dated 2010**
Project plans approved May 12, 2014	***Standard Plans* dated 2010**

Identified by

Contract No. 07-2881U4

07-LA-210-R16.1/R25.8

Project ID 0714000085

Federal-Aid Project
ACNHPI-210-1(823)E

Electronic Bidding Contract

AADD

Bids open Thursday, September 11, 2014 **OSD**

Dated July 21, 2014 **IH**

****

SPECIAL NOTICES

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- This is an electronic bidding project.
- For federal-aid projects, the Department is modifying its DBE program.
- The Department has revised its HMA specifications to the Superpave design method. See section 39 of the RSS.

CONTRACT NO. 07-2881U4

The special provisions contained herein have been prepared by or under the direction of the following Registered Persons.

HIGHWAYS

REGISTERED CIVIL ENGINEER

REGISTERED PROFESSIONAL ENGINEER
Andy Liao
No. C 57416
Exp. 12-31-2015
CIVIL
STATE OF CALIFORNIA

MAINTAINING TRAFFIC (FREEWAY)

REGISTERED CIVIL ENGINEER

REGISTERED PROFESSIONAL ENGINEER
Dennis Do
No. C 68587
Exp. 9-30-2015
CIVIL
STATE OF CALIFORNIA

ELECTRICAL

REGISTERED ELECTRICAL ENGINEER

REGISTERED PROFESSIONAL ENGINEER
Cecilio Burciaga
No. E 015302
Exp. 3-31-2015
ELECT.
STATE OF CALIFORNIA

CONTRACT NO. 07-2881U4

The special provisions contained herein have been prepared by or under the direction of the following Registered Persons.

STRUCTURES

REGISTERED CIVIL ENGINEER

REGISTERED PROFESSIONAL ENGINEER
James Choi
No. C 50777
Exp. 09/30/2015
CIVIL
STATE OF CALIFORNIA

LANDSCAPE

LICENSED LANDSCAPE ARCHITECT

LICENSED LANDSCAPE ARCHITECT
KEITH SELLERS NO. 5288
SIGNATURE
04-30-15
RENEWAL DATE
6/9/14
DATE
STATE OF CALIFORNIA

ARCHITECT

LICENSED ARCHITECT

LICENSED ARCHITECT
Carlos Adolfo Sanchez
No. C 15202
Exp. 12/31/2015
STATE OF CALIFORNIA

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STANDARD PLANS LIST

The standard plan sheets applicable to this Contract include those listed below. The applicable revised standard plans (RSPs) listed below are included in the project plans.

A10A	Abbreviations (Sheet 1 of 2)
RSP A10B	Abbreviations (Sheet 2 of 2)
A10C	Lines and Symbols (Sheet 1 of 3)
A10D	Lines and Symbols (Sheet 2 of 3)
A10E	Lines and Symbols (Sheet 3 of 3)
A20A	Pavement Markers and Traffic Lines, Typical Details
A20B	Pavement Markers and Traffic Lines, Typical Details
RSP A20C	Pavement Markers and Traffic Lines, Typical Details
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A24B	Pavement Markings - Arrows and Symbols
RSP A24C	Pavement Markings - Symbols and Numerals
A24D	Pavement Markings - Words
RSP A24E	Pavement Markings - Words, Limit and Yield Lines
A62A	Excavation and Backfill - Miscellaneous Details
A62B	Limits of Payment for Excavation and Backfill - Bridge Surcharge and Wall
A62C	Limits of Payment for Excavation and Backfill - Bridge
A62D	Excavation and Backfill - Concrete Pipe Culverts
A62F	Excavation and Backfill - Metal and Plastic Culverts
A73B	Markers
A73C	Delineators, Channelizers and Barricades
A76A	Concrete Barrier Type 60
A76B	Concrete Barrier Type 60
RSP A76C	Concrete Barrier Type 60F
A76D	Concrete Barrier Type 60G
A76E	Concrete Barrier Type 60G
RSP A77L1	Midwest Guardrail System Standard Railing Section (Wood Post with Wood Block)
RSP A77L2	Midwest Guardrail System Standard Railing Section (Steel Post with Notched Wood or Notched Recycled Plastic Block)
RSP A77L3	Metal Beam Guard Railing Reconstruct Installation
RSP A77M1	Midwest Guardrail System Standard Hardware

RSP A77N1	Midwest Guardrail System Wood Post and Wood Block Details
RSP A77N2	Midwest Guardrail System Steel Post and Notched Wood Block Details
RSP A77N3	Midwest Guardrail System Typical Line Post Embedment and Hinge Point Offset Details
RSP A77N4	Midwest Guardrail System Typical Railing Delineation and Dike Positioning Details
RSP A77N5	Midwest Guardrail System Typical Vegetation Control Standard Railing Section
RSP A77N6	Midwest Guardrail System Typical Vegetation Control for Terminal System End Treatments
RSP A77N7	Midwest Guardrail System Typical Vegetation Control at Structure Approach
RSP A77N8	Midwest Guardrail System Typical Vegetation Control at Fixed Object
RSP A77Q1	Midwest Guardrail System Typical Layouts for Structure Approach
RSP A77Q2	Midwest Guardrail System Typical Layouts for Structure Approach and Between Structures
RSP A77R3	Midwest Guardrail System Typical Layouts for Roadside Fixed Objects
RSP A77S1	Midwest Guardrail System End Anchor Assembly (Type SFT)
RSP A77S2	Midwest Guardrail System Rail Tensioning Assembly
RSP A77S3	Metal Railing Anchor Cable and Anchor Plate Details
RSP A77T2	Midwest Guardrail System Buried Post End Anchor
RSP A77U1	Midwest Guardrail System Connections to Bridge Railings without Sidewalks Details No. 1
RSP A77U2	Midwest Guardrail System Connections to Bridge Railings without Sidewalks Details No. 2
RSP A77U4	Midwest Guardrail System Transition Railing (Type WB-31)
A78C1	Thrie Beam Barrier - Standard Hardware Details
RSP A78C2	Thrie Beam Barrier - Post and Block Details
RSP A78C3	Single Thrie Beam Barrier - Typical Vegetation Control Standard Barrier Railing Section
A78F2	Single Thrie Beam Barrier - Connections to Bridge Railings without Sidewalks
A78G	Single Thrie Beam Barrier - Connections to Abutments and Walls
A85	Chain Link Fence
A85A	Chain Link Fence Details
RSP A85B	Chain Link Fence Details
RSP A87A	Curbs and Driveways
RSP A87B	Hot Mix Asphalt Dikes
RSP A88A	Curb Ramp Details
RSP A88B	Curb Ramp and Island Passageway Details
RSP P1	Jointed Plain Concrete Pavement New Construction
RSP P2	Jointed Plain Concrete Pavement (Widened Lane) New Construction
RSP P3A	Jointed Plain Concrete Pavement Lane & Shoulder Addition or Replacement
RSP P3B	Jointed Plain Concrete Pavement (Widened Lane) Lane and Shoulder Addition or

	Replacement
RSP P4	Continuously Reinforced Concrete Pavement
RSP P8	Individual Slab Replacement with Rapid Strength Concrete
RSP P10	Concrete Pavement Dowel Bar Details
RSP P12	Concrete Pavement Dowel Bar Basket Details
RSP P14	Continuously Reinforced Concrete Pavement Transverse Construction Joint
RSP P15	Concrete Pavement - Tie Bar Details
RSP P16	Continuously Reinforced Concrete Pavement Tie Bars and Joint Details
RSP P17	Concrete Pavement Tie Bar Basket Details
RSP P18	Concrete Pavement Lane Schematics and Isolation Joint Detail
RSP P20	Joint Seals
RSP P30	Concrete Pavement - End Panel Pavement Transitions
P45	Concrete Pavement - Drainage Inlet Details No. 1
P46	Concrete Pavement - Drainage Inlet Details No. 2
RSP P75	Pavement Edge Treatments - Overlays
RSP P76	Pavement Edge Treatments - New Construction
RSP D73	Drainage Inlets
D74A	Drainage Inlets
D74B	Drainage Inlets
D74C	Drainage Inlet Details
D75B	Concrete Pipe Inlets
D75C	Pipe Inlets - Ladder and Trash Rack Details
RSP D77A	Grate Details No. 1
D79A	Precast Reinforced Concrete Pipe - Direct Design Method
D97H	Reinforced Concrete Pipe or Non-Reinforced Concrete Pipe - Standard and Positive Joints
D98A	Slotted Corrugated Steel Pipe Drain Details
D98B	Slotted Corrugated Steel Pipe Drain Details
D98C	Grated Line Drain Details
D102	Underdrains
RSP H1	Landscape and Erosion Control Abbreviations
RSP H2	Landscape and Erosion Control Symbols
RSP H5	Landscape Details
RSP H6	Landscape Details
RSP H7	Landscape Details
RSP H9	Landscape Details

RSP H9A	Landscape Details
T1A	Temporary Crash Cushion, Sand Filled (Unidirectional)
T1B	Temporary Crash Cushion, Sand Filled (Bidirectional)
T2	Temporary Crash Cushion, Sand Filled (Shoulder Installations)
T3A	Temporary Railing (Type K)
T3B	Temporary Railing (Type K)
T4	Temporary Traffic Screen
T5	Temporary Terminal Section (Type K)
RSP T9	Traffic Control System Tables for Lane and Ramp Closures
RSP T10	Traffic Control System for Lane Closure on Freeways and Expressways
RSP T10A	Traffic Control System for Lane Closures on Freeways and Expressways
RSP T14	Traffic Control System for Ramp Closure
T51	Temporary Water Pollution Control Details (Temporary Silt Fence)
T53	Temporary Water Pollution Control Details (Temporary Cover)
T56	Temporary Water Pollution Control Details (Temporary Fiber Roll)
T58	Temporary Water Pollution Control Details (Temporary Construction Entrance)
T59	Temporary Water Pollution Control Details (Temporary Concrete Washout Facility)
T61	Temporary Water Pollution Control Details (Temporary Drainage Inlet Protection)
T62	Temporary Water Pollution Control Details (Temporary Drainage Inlet Protection)
T63	Temporary Water Pollution Control Details (Temporary Drainage Inlet Protection)
T64	Temporary Water Pollution Control Details (Temporary Drainage Inlet Protection)
B0-1	Bridge Details
B0-3	Bridge Details
RSP B3-1A	Retaining Wall Type 1 (Case 1)
RSP B3-1B	Retaining Wall Type 1 (Case 2)
RSP B3-1C	Retaining Wall Type 1 (Case 3)
B6-21	Joint Seals (Maximum Movement Rating = 2")
B11-7	Chain Link Railing
RSP B11-47	Cable Railing
RSP B11-56	Concrete Barrier Type 736
RS1	Roadside Signs, Typical Installation Details No. 1
RS2	Roadside Signs - Wood Post, Typical Installation Details No. 2
RS4	Roadside Signs, Typical Installation Details No. 4
RSP S1	Overhead Signs - Truss, Instructions and Examples
RSP S2	Overhead Signs - Truss, Single Post Type - Post Types II thru IX

S3	Overhead Signs - Truss, Single Post Type - Base Plate and Anchorage Details
S4	Overhead Signs - Truss, Single Post Type - Structural Frame Members Details No. 1
S5	Overhead Signs - Truss, Single Post Type - Structural Frame Members Details No. 2
S6	Overhead Signs - Truss, Gusset Plate Details
S8	Overhead Signs - Truss, Single Post Type - Round Pedestal Pile Foundation
S12	Overhead Signs - Truss, Structural Frame Details
S13	Overhead Signs - Truss, Frame Juncture Details
S15	Overhead Signs - Truss, Two Post Type - Round Pedestal Pile Foundation
S16	Overhead Signs - Walkway Details No. 1
S17	Overhead Signs - Walkway Details No. 2
S17A	Overhead Signs - Walkway Details No. 3
S18	Overhead Signs - Walkway Safety Railing Details
S90	Channel and Bolt Hole Location, Overhead Formed Sign Panel
S91	Overhead Sign - Formed Sign Panel, Type A-3 Mounting Hardware
S92	Overhead Sign - Formed Sign Panel
S93	Framing Details for Framed Single Sheet Aluminum Signs, Rectangular Shape
S94	Roadside Framed Single Sheet Aluminum Signs, Rectangular Shape
S95	Roadside Single Sheet Aluminum Signs, Diamond Shape
RSP ES-1A	Electrical Systems (Legend and Abbreviations)
RSP ES-1B	Electrical Systems (Legend and Abbreviations)
RSP ES-1C	Electrical Systems (Legend and Abbreviations)
ES-2A	Electrical Systems (Service Equipment)
ES-2C	Electrical Systems (Service Equipment Notes, Type III Series)
ES-2E	Electrical Systems (Service Equipment Enclosure and Typical Wiring Diagram, Type III - B Series)
ES-2F	Electrical Systems (Service Equipment Enclosure and Typical Wiring Diagram Type III - C Series)
ES-3C	Electrical Systems (Controller Cabinet Foundation Details)
ES-3F	Electrical Systems (Telephone Demarcation Cabinet, Type C)
ES-5A	Electrical Systems (Detectors)
RSP ES-5B	Electrical Systems (Detectors)
RSP ES-5D	Electrical Systems (Curb Termination and Handhole)
ES-6A	Electrical Systems (Lighting Standard, Types 15 and 21)
ES-6E	Electrical Systems (Lighting Standard, Types 30 and 31)
ES-6F	Electrical Systems (Lighting Standard, Slip Base Plate)
ES-7M	Electrical Systems (Signal and Lighting Standard - Detail No. 1)

ES-7N	Electrical Systems (Signal and Lighting Standard - Detail No. 2)
ES-7O	Electrical Systems (Signal and Lighting Standard - Detail No. 3)
RSP ES-8A	Electrical Systems (Non-Traffic Pull Box)
RSP ES-8B	Electrical Systems (Traffic Pull Box)
RSP ES-10A	Electrical Systems (Isofootcandle Diagrams)
RSP ES-10B	Electrical Systems (Isofootcandle Diagrams)
RSP ES-11	Electrical Systems (Foundation Installations)
ES-13A	Electrical Systems (Splicing Details)
ES-13B	Electrical Systems (Fuse Rating, Kinking and Banding Detail)
ES-15A	Electrical Systems (Sign Illumination Equipment)
ES-15C	Electrical Systems (Sign Illumination Equipment)
ES-15D	Electrical Systems (Lighting and Sign Illumination Control)

CANCELED STANDARD PLANS LIST

The standard plan sheets listed below are canceled and not applicable to this contract.

Plan No.	Date Canceled	Plan No.	Date Canceled	Plan No.	Date Canceled
A77A1	07-19-13	A77J4	07-19-13	ES-6J	07-19-13
A77A2	07-19-13	A77K1	07-19-13	ES-7I	07-19-13
A77B1	07-19-13	A77K2	07-19-13	ES-8	01-20-12
A77C1	07-19-13	P3	07-19-13	ES-10	07-20-12
A77C2	07-19-13	C8A	07-19-13		
A77C3	07-19-13	C8B	07-19-13		
A77C4	07-19-13	C8C	07-19-13		
RSP A77C5	07-19-13	B3-1	04-20-12		
RSP A77C6	07-19-13	B3-2	04-20-12		
RSP A77C7	07-19-13	B3-3	04-20-12		
RSP A77C8	07-19-13	B3-4	04-20-12		
RSP A77C9	07-19-13	B3-7	04-20-12		
RSP A77C10	07-19-13	B3-8	04-20-12		
A77E1	07-19-13	S7	07-19-13		
A77E2	07-19-13	S14	07-19-13		
A77E3	07-19-13	S41	07-19-13		
A77E4	07-19-13	S42	07-19-13		
A77E5	07-19-13	S43	07-19-13		
A77E6	07-19-13	S44	07-19-13		
A77F1	07-19-13	S45	07-19-13		
A77F2	07-19-13	S46	07-19-13		
A77F3	07-19-13	S47	07-19-13		
A77F4	07-19-13	S120	07-19-13		
A77F5	07-19-13	S121	07-19-13		
A77G1	07-19-13	S122	07-19-13		
A77G2	07-19-13	S123	07-19-13		
A77G3	07-19-13	S124	07-19-13		
A77G4	07-19-13	S125	07-19-13		
A77G5	07-19-13	S126	07-19-13		
A77G6	07-19-13	S127	07-19-13		
A77G7	07-19-13	S128	07-19-13		
A77G8	07-19-13	S129	07-19-13		
A77H1	07-19-13	S130	07-19-13		
A77H2	07-19-13	S131	07-19-13		
A77H3	07-19-13	S132	07-19-13		
A77I1	07-19-13	S133	07-19-13		
A77I2	07-19-13	S134	07-19-13		
A77J1	07-19-13	S135	07-19-13		
A77J2	07-19-13	ES-6H	07-19-13		
A77J3	07-19-13	ES-6I	07-19-13		

NOTICE TO BIDDERS

Bids open Thursday, September 11, 2014

Dated July 21, 2014

General work description: Construct concrete pavement, precast concrete panels, and slabs.

The Department will receive sealed bids for CONSTRUCTION ON STATE HIGHWAY IN LOS ANGELES COUNTY IN LA CANADA FLINTRIDGE, GLENDALE AND PASADENA FROM DUNSMORE AVENUE UNDERCROSSING TO NORTH LOS ROBLES AVENUE OVERCROSSING.

District-County-Route-Post Mile: 07-LA-210-R16.1/R25.8

Contract No. 07-2881U4

The Contractor must have either a Class A license or one of the following Class C licenses: C-8, C-12.

The DBE Contract goal is 11 percent.

Federal-aid project no.:

ACNHPI-210-1(823)E

For the Federal training program, the number of trainees or apprentices is 29.

Bids must be on a unit price basis.

Complete the work within 640 working days.

The estimated cost of the project is $120,000,000.

No prebid meeting is scheduled for this project.

The Department will receive bids until 2:00 p.m. on the bid open date via Bid Express web site. Bids received after this time will not be accepted. For more information refer to the Electronic Bidding Guide at the Bidder's Exchange web site.

The Department will open and publicly read the bids at 1727 30th Street, Bidders' Exchange, MS 26, Sacramento, CA 95816 immediately after the specified closing time.

District office addresses are provided in the *Standard Specifications*.

Present bidders' inquiries to the Department and view the Department's responses at:

http://www.dot.ca.gov/hq/esc/oe/inquiry/bid_inquiries.php

Questions about alleged patent ambiguity of the plans, specifications, or estimate must be asked before bid opening. After bid opening, the Department does not consider these questions as bid protests.

Submit your bid with bidder's security equal to at least 10 percent of the bid.

Prevailing wages are required on this Contract. The Director of the California Department of Industrial Relations determines the general prevailing wage rates. Obtain the wage rates at the DIR Web site, http://www.dir.ca.gov, or from the Department's Labor Compliance Office of the district in which the work is located.

The federal minimum wage rates for this Contract as determined by the United States Secretary of Labor are available at http://www.dot.ca.gov/hq/esc/oe/federal-wages.

If the minimum wage rates as determined by the United States Secretary of Labor differs from the general prevailing wage rates determined by the Director of the California Department of Industrial Relations for similar classifications of labor, the Contractor and subcontractors must not pay less than the higher wage rate. The Department does not accept lower State wage rates not specifically included in the federal minimum wage determinations. This includes helper, or other classifications based on hours of experience, or any other classification not appearing in the federal wage determinations. Where federal wage determinations do not contain the State wage rate determination otherwise available for use by the Contractor and subcontractors, the Contractor and subcontractors must not pay less than the federal minimum wage rate that most closely approximates the duties of the employees in question.

The Department has made available Notices of Suspension and Proposed Debarment from the Federal Highway Administration. For a copy of the notices, go to http://www.dot.ca.gov/hq/esc/oe/contractor_info. Additional information is provided in the Excluded Parties List System at https://www.epls.gov.

Department of Transportation

D07AS

BID ITEM LIST

Item No.	Item Code	Item Description	Unit of Measure	Estimated Quantity
1	070030	LEAD COMPLIANCE PLAN	LS	LUMP SUM
2	080050	PROGRESS SCHEDULE (CRITICAL PATH METHOD)	LS	LUMP SUM
3	090100	TIME-RELATED OVERHEAD (WDAY)	WDAY	640
4	120090	CONSTRUCTION AREA SIGNS	LS	LUMP SUM
5	120100	TRAFFIC CONTROL SYSTEM	LS	LUMP SUM
6	120120	TYPE III BARRICADE	EA	190
7	120165	CHANNELIZER (SURFACE MOUNTED)	EA	1,620
8	120199	TRAFFIC PLASTIC DRUM	EA	430
9	121161	TEMPORARY TERMINAL SECTION (TYPE K)	EA	5
10	128651	PORTABLE CHANGEABLE MESSAGE SIGN (EA)	EA	2
11	129000	TEMPORARY RAILING (TYPE K)	LF	283,000
12	129110	TEMPORARY CRASH CUSHION	EA	810
13	130100	JOB SITE MANAGEMENT	LS	LUMP SUM
14	130300	PREPARE STORM WATER POLLUTION PREVENTION PLAN	LS	LUMP SUM
15	130310	RAIN EVENT ACTION PLAN	EA	55
16	130320	STORM WATER SAMPLING AND ANALYSIS DAY	EA	16
17	130330	STORM WATER ANNUAL REPORT	EA	3
18	130505	MOVE-IN/MOVE-OUT (TEMPORARY EROSION CONTROL)	EA	12
19	130570	TEMPORARY COVER	SQYD	10,000
20	130620	TEMPORARY DRAINAGE INLET PROTECTION	EA	200

Item No.	Item Code	Item Description	Unit of Measure	Estimated Quantity
21	130640	TEMPORARY FIBER ROLL	LF	26,000
22	130650	TEMPORARY GRAVEL BAG BERM	LF	25,000
23	130680	TEMPORARY SILT FENCE	LF	15,000
24	130710	TEMPORARY CONSTRUCTION ENTRANCE	EA	48
25	130730	STREET SWEEPING	LS	LUMP SUM
26	130900	TEMPORARY CONCRETE WASHOUT	LS	LUMP SUM
27	141110	WORK AREA MONITORING (BRIDGE)	LS	LUMP SUM
28	141120	TREATED WOOD WASTE	LB	554,000
29	150100	PUBLIC SAFETY PLAN	LS	LUMP SUM
30	150312	REPAIR SPALLED SURFACE AREA	SQFT	2,268
31	150608	REMOVE CHAIN LINK FENCE	LF	18
32	150661	REMOVE GUARDRAIL	LF	28,100
33	150685	REMOVE IRRIGATION FACILITY	LS	LUMP SUM
34	150711	REMOVE PAINTED TRAFFIC STRIPE	LF	699,000
35	150714	REMOVE THERMOPLASTIC TRAFFIC STRIPE	LF	340,000
36	150715	REMOVE THERMOPLASTIC PAVEMENT MARKING	SQFT	6,090
37	150722	REMOVE PAVEMENT MARKER	EA	66,700
38	150730	REMOVE CHANNELIZERS	EA	170
39	150742	REMOVE ROADSIDE SIGN	EA	80
40	150748	REMOVE ROADSIDE SIGN PANEL	EA	4

Item No.	Item Code	Item Description	Unit of Measure	Estimated Quantity
41	150757	REMOVE SIGN STRUCTURE (EA)	EA	46
42	150771	REMOVE ASPHALT CONCRETE DIKE	LF	146,000
43	150812	REMOVE PIPE (LF)	LF	3,500
44	150820	REMOVE INLET	EA	73
45	150853	REMOVE CONCRETE PAVEMENT (SQYD)	SQYD	303,000
46	150860	REMOVE BASE AND SURFACING	CY	750
47	150870	REMOVE CONCRETE DECK SURFACE	SQFT	1,810
48	027672	SALVAGE PRECAST CONCRETE SLABS	EA	84
49	151251	SALVAGE IRRIGATION FACILITY	LS	LUMP SUM
50	152390	RELOCATE ROADSIDE SIGN	EA	1
51	152430	ADJUST INLET	EA	2
52	152454	ADJUST PULL BOX	EA	40
53	152604	MODIFY INLET	EA	14
54	153103	COLD PLANE ASPHALT CONCRETE PAVEMENT	SQYD	71,600
55	153221	REMOVE CONCRETE BARRIER	LF	31,600
56	153225	PREPARE CONCRETE BRIDGE DECK SURFACE	SQFT	13,770
57	153226	REFINISH BRIDGE DECK	SQFT	7,610
58	153227	FURNISH POLYESTER CONCRETE OVERLAY	CF	1,550
59 (F)	153228	PLACE POLYESTER CONCRETE OVERLAY	SQFT	13,770
60	155003	CAP INLET	EA	1

Item No.	Item Code	Item Description	Unit of Measure	Estimated Quantity
61	156585	REMOVE CRASH CUSHION	EA	2
62	157561	BRIDGE REMOVAL (PORTION), LOCATION A	LS	LUMP SUM
63	157562	BRIDGE REMOVAL (PORTION), LOCATION B	LS	LUMP SUM
64	157563	BRIDGE REMOVAL (PORTION), LOCATION C	LS	LUMP SUM
65	157564	BRIDGE REMOVAL (PORTION), LOCATION D	LS	LUMP SUM
66	157565	BRIDGE REMOVAL (PORTION), LOCATION E	LS	LUMP SUM
67	157566	BRIDGE REMOVAL (PORTION), LOCATION F	LS	LUMP SUM
68	157567	BRIDGE REMOVAL (PORTION), LOCATION G	LS	LUMP SUM
69	157568	BRIDGE REMOVAL (PORTION), LOCATION H	LS	LUMP SUM
70	157569	BRIDGE REMOVAL (PORTION), LOCATION I	LS	LUMP SUM
71	157570	BRIDGE REMOVAL (PORTION), LOCATION J	LS	LUMP SUM
72	157571	BRIDGE REMOVAL (PORTION), LOCATION K	LS	LUMP SUM
73	157572	BRIDGE REMOVAL (PORTION), LOCATION L	LS	LUMP SUM
74	157573	BRIDGE REMOVAL (PORTION), LOCATION M	LS	LUMP SUM
75	044684	BRIDGE REMOVAL (PORTION), LOCATION N	LS	LUMP SUM
76	044685	BRIDGE REMOVAL (PORTION), LOCATION 0	LS	LUMP SUM
77	044686	BRIDGE REMOVAL (PORTION), LOCATION P	LS	LUMP SUM
78	160102	CLEARING AND GRUBBING (LS)	LS	LUMP SUM
79	190101	ROADWAY EXCAVATION	CY	226,000
80	190105	ROADWAY EXCAVATION (TYPE Z-2) (AERIALLY DEPOSITED LEAD)	CY	1,230

Item No.	Item Code	Item Description	Unit of Measure	Estimated Quantity
81 (F)	192003	STRUCTURE EXCAVATION (BRIDGE)	CY	1,127
82 (F)	192037	STRUCTURE EXCAVATION (RETAINING WALL)	CY	985
83 (F)	193003	STRUCTURE BACKFILL (BRIDGE)	CY	1,127
84 (F)	193013	STRUCTURE BACKFILL (RETAINING WALL)	CY	515
85 (F)	193031	PERVIOUS BACKFILL MATERIAL (RETAINING WALL)	CY	110
86	194001	DITCH EXCAVATION	CY	730
87	200002	ROADSIDE CLEARING	LS	LUMP SUM
88	202039	SLOW-RELEASE FERTILIZER	LB	130
89	204099	PLANT ESTABLISHMENT WORK	LS	LUMP SUM
90	206400	CHECK AND TEST EXISTING IRRIGATION FACILITIES	LS	LUMP SUM
91	206402	OPERATE EXISTING IRRIGATION FACILITIES	LS	LUMP SUM
92	206560	CONTROL AND NEUTRAL CONDUCTORS	LS	LUMP SUM
93	206562	1" REMOTE CONTROL VALVE	EA	1
94	206563	1 1/4" REMOTE CONTROL VALVE	EA	5
95	206564	1 1/2" REMOTE CONTROL VALVE	EA	15
96	208446	RISER SPRINKLER ASSEMBLY (GEAR DRIVEN)	EA	42
97	208576	2 1/2" GATE VALVE	EA	1
98 (F)	208594	3/4" PLASTIC PIPE (SCHEDULE 40) (SUPPLY LINE)	LF	380
99 (F)	208595	1" PLASTIC PIPE (SCHEDULE 40) (SUPPLY LINE)	LF	455
100 (F)	208596	1 1/4" PLASTIC PIPE (SCHEDULE 40) (SUPPLY LINE)	LF	630

Item No.	Item Code	Item Description	Unit of Measure	Estimated Quantity
101 (F)	208597	1 1/2" PLASTIC PIPE (SCHEDULE 40) (SUPPLY LINE)	LF	250
10 (F)2	208598	2" PLASTIC PIPE (SCHEDULE 40) (SUPPLY LINE)	LF	1,020
103 (F)	208599	2 1/2" PLASTIC PIPE (SCHEDULE 40) (SUPPLY LINE)	LF	870
104	208683	BALL VALVE	EA	4
105	260303	CLASS 3 AGGREGATE BASE (CY)	CY	80,900
106	027673	AGGREGATE BASE CEMENT SLURRY	CY	17,700
107	027674	ALTERNATE TREATED BASE	CY	58,900
108	280015	LEAN CONCRETE BASE RAPID SETTING	CY	4
109	280200	REPLACE BASE	CY	360
110	360200	BASE BOND BREAKER	SQYD	429,000
111	027675	HIGH FRICTION SURFACE TREATMENT	SQYD	18,200
112	390095	REPLACE ASPHALT CONCRETE SURFACING	CY	7,790
113	390300	HOT MIX ASPHALT, SUPERPAVE (TYPE A)	TON	3,730
114	390301	RUBBERIZED HOT MIX ASPHALT, SUPERPAVE (GAP GRADED)	TON	7,660
115	394073	PLACE HOT MIX ASPHALT DIKE (TYPE A)	LF	125,000
116	394074	PLACE HOT MIX ASPHALT DIKE (TYPE C)	LF	2,380
117	394075	PLACE HOT MIX ASPHALT DIKE (TYPE D)	LF	670
118	394077	PLACE HOT MIX ASPHALT DIKE (TYPE F)	LF	930
119	400050	CONTINUOUSLY REINFORCED CONCRETE PAVEMENT	CY	150
120	027676	CONTINUOUSLY REINFORCED CONCRETE PAVEMENT - RAPID STRENGTH CONCRETE	CY	540

Item No.	Item Code	Item Description	Unit of Measure	Estimated Quantity
121	401050	JOINTED PLAIN CONCRETE PAVEMENT	CY	104,000
122	401055	JOINTED PLAIN CONCRETE PAVEMENT (RSC)	CY	180
123	027677	PRECAST JOINTED CONCRETE PAVEMENT (PJCP)	CY	33,600
124	027678	SHOULDER CONCRETE PAVEMENT	CY	2,610
125	027679	INDIVIDUAL PRECAST SLAB REPLACEMENT (IPSR)	CY	520
126	420201	GRIND EXISTING CONCRETE PAVEMENT	SQYD	391,000
127	498052	60" CAST-IN-DRILLED-HOLE CONCRETE PILE (SIGN FOUNDATION)	LF	1,060
128 (F)	510053	STRUCTURAL CONCRETE, BRIDGE	CY	193
129 (F)	510060	STRUCTURAL CONCRETE, RETAINING WALL	CY	360
130	510081	AGGREGATE BASE (APPROACH SLAB)	CY	743
131	510087	STRUCTURAL CONCRETE, APPROACH SLAB (TYPE R)	CY	7,445
132 (F)	510502	MINOR CONCRETE (MINOR STRUCTURE)	CY	294
133	510800	PAVING NOTCH EXTENSION	CF	4,225
134	511106	DRILL AND BOND DOWEL	LF	545
135	511118	CLEAN EXPANSION JOINT	LF	760
136	519081	JOINT SEAL (MR 1/2")	LF	3,327
137	519088	JOINT SEAL (MR 1")	LF	2,300
138	519091	JOINT SEAL (MR 1 1/2")	LF	1,385
139	519100	JOINT SEAL (MR 2")	LF	629
140 (F)	520102	BAR REINFORCING STEEL (BRIDGE)	LB	25,500

Item No.	Item Code	Item Description	Unit of Measure	Estimated Quantity
141 (F)	520103	BAR REINFORCING STEEL (RETAINING WALL)	LB	78,410
142 (F)	560218	FURNISH SIGN STRUCTURE (TRUSS)	LB	904,229
143 (F)	560219	INSTALL SIGN STRUCTURE (TRUSS)	LB	904,229
144	560233	FURNISH FORMED PANEL SIGN (OVERHEAD)	SQFT	11,500
145	560244	FURNISH LAMINATED PANEL SIGN (1"-TYPE A)	SQFT	5,050
146	560245	FURNISH LAMINATED PANEL SIGN (1"-TYPE B)	SQFT	750
147	560248	FURNISH SINGLE SHEET ALUMINUM SIGN (0.063"-UNFRAMED)	SQFT	120
148	560249	FURNISH SINGLE SHEET ALUMINUM SIGN (0.080"-UNFRAMED)	SQFT	640
149	560251	FURNISH SINGLE SHEET ALUMINUM SIGN (0.063"-FRAMED)	SQFT	88
150	560252	FURNISH SINGLE SHEET ALUMINUM SIGN (0.080"-FRAMED)	SQFT	770
151	566011	ROADSIDE SIGN - ONE POST	EA	51
152	566012	ROADSIDE SIGN - TWO POST	EA	28
153	568001	INSTALL SIGN (STRAP AND SADDLE BRACKET METHOD)	EA	24
154	568016	INSTALL SIGN PANEL ON EXISTING FRAME	SQFT	5,050
155	027680	INSTALL SIGN PANEL SOUND WALL/BARRIER MOUNTED	EA	2
156	044687	PREPARE AND CLEAN CONCRETE SURFACES	SQFT	164,099
157	590280	PAINT CONCRETE SURFACES	SQFT	164,099
158 (F)	598001	ANTI-GRAFFITI COATING	SQFT	164,099
159	650012	15" REINFORCED CONCRETE PIPE	LF	1,860
160	650014	18" REINFORCED CONCRETE PIPE	LF	2,370

Item No.	Item Code	Item Description	Unit of Measure	Estimated Quantity
161	650018	24" REINFORCED CONCRETE PIPE	LF	8
162	650026	36" REINFORCED CONCRETE PIPE	LF	6
163	665712	12" SLOTTED CORRUGATED STEEL PIPE (.079" THICK)	LF	160
164	665717	18" SLOTTED CORRUGATED STEEL PIPE (.079" THICK)	LF	4,380
165	680902	6" PERFORATED PLASTIC PIPE UNDERDRAIN	LF	4,770
166	680903	6" NON-PERFORATED PLASTIC PIPE UNDERDRAIN	LF	58
167 (F)	682008	PERMEABLE MATERIAL (BLANKET)	CY	692
168	703233	GRATED LINE DRAIN	LF	2,730
169	708031	6" ALTERNATIVE PIPE RISER	LF	130
170	721430	CONCRETE (CHANNEL LINING)	CY	400
171	721810	SLOPE PAVING (CONCRETE)	CY	1,310
172	729011	ROCK SLOPE PROTECTION FABRIC (CLASS 8)	SQYD	4,790
173	730040	MINOR CONCRETE (GUTTER) (LF)	LF	520
174	731516	MINOR CONCRETE (DRIVEWAY)	CY	60
175	731519	MINOR CONCRETE (STAMPED CONCRETE)	SQFT	90,300
176	731627	MINOR CONCRETE (CURB, SIDEWALK AND CURB RAMP)	CY	150
177	733000	PRE/POST CONSTRUCTION SURVEYS	EA	62
178 (F)	750001	MISCELLANEOUS IRON AND STEEL	LB	89,726
179	800360	CHAIN LINK FENCE (TYPE CL-6)	LF	61
180	802501	4' CHAIN LINK GATE (TYPE CL-6)	EA	25

Item No.	Item Code	Item Description	Unit of Measure	Estimated Quantity
181	820107	DELINEATOR (CLASS 1)	EA	250
182	027681	INSTALL MEDIAN MILEAGE PANEL	EA	64
183	832015	MIDWEST GUARDRAIL SYSTEM (7' WOOD POST)	LF	8,370
184	832070	VEGETATION CONTROL (MINOR CONCRETE)	SQYD	5,230
185	044688	CONCRETE BARRIER (TRANSITION)	LF	133
186	839221	DOUBLE MIDWEST GUARDRAIL SYSTEM (WOOD POST)	LF	310
187 (F)	839521	CABLE RAILING	LF	512
188	839543	TRANSITION RAILING (TYPE WB-31)	EA	36
189	839576	END CAP (TYPE A)	EA	5
190	839581	END ANCHOR ASSEMBLY (TYPE SFT)	EA	32
191	839584	ALTERNATIVE IN-LINE TERMINAL SYSTEM	EA	33
192	839585	ALTERNATIVE FLARED TERMINAL SYSTEM	EA	25
193	027682	CRASH CUSHION (TYPE SCI SMART 100 GM)	EA	5
194 (F)	044689	RUSTIC ROCK BARRIER PATTERN	LF	71,288
195 (F)	044690	CONCRETE BARRIER (TYPE 60A MODIFIED)	LF	4,339
196 (F)	044691	CONCRETE BARRIER (TYPE 60A MODIFIED 1)	LF	656
197	027683	CONCRETE ANCHOR BLOCK	CY	19
198	027684	CONCRETE BARRIER (TYPE 60F MODIFIED)	LF	40
199	839700	CONCRETE BARRIER (TYPE 60F)	LF	2,860
200	839701	CONCRETE BARRIER (TYPE 60)	LF	13,500

Item No.	Item Code	Item Description	Unit of Measure	Estimated Quantity
201	839703	CONCRETE BARRIER (TYPE 60C)	LF	310
202	839704	CONCRETE BARRIER (TYPE 60D)	LF	520
203	027685	CONCRETE BARRIER (TYPE 60W)	LF	970
204	027686	CONCRETE BARRIER (TYPE 60W MODIFIED)	LF	19,300
205 (F)	027687	CONCRETE BARRIER (TYPE 736B MODIFIED)	LF	19,649
206	840504	4" THERMOPLASTIC TRAFFIC STRIPE	LF	307,000
207	840506	8" THERMOPLASTIC TRAFFIC STRIPE	LF	38,100
208	840508	8" THERMOPLASTIC TRAFFIC STRIPE (BROKEN 12-3)	LF	25,800
209	840515	THERMOPLASTIC PAVEMENT MARKING	SQFT	6,090
210	840525	4" THERMOPLASTIC TRAFFIC STRIPE (BROKEN 36-12)	LF	358,000
211	840526	4" THERMOPLASTIC TRAFFIC STRIPE (BROKEN 17-7)	LF	9,590
212	840550	8" THERMOPLASTIC TRAFFIC STRIPE (BROKEN 36-12)	LF	1,950
213	840655	PAINT TRAFFIC STRIPE (1-COAT)	LF	358,000
214	840656	PAINT TRAFFIC STRIPE (2-COAT)	LF	699,000
215	850101	PAVEMENT MARKER (NON-REFLECTIVE)	EA	29,900
216	850111	PAVEMENT MARKER (RETROREFLECTIVE)	EA	36,900
217	860090	MAINTAINING EXISTING TRAFFIC MANAGEMENT SYSTEM ELEMENTS DURING CONSTRUCTION	LS	LUMP SUM
218	860400	LIGHTING (TEMPORARY)	LS	LUMP SUM
219	027688	MICROWAVE VEHICLE DETECTION SYSTEM (MVDS)	LS	LUMP SUM
220	860889	MODIFY TRAFFIC MONITORING STATION	LS	LUMP SUM

Item No.	Item Code	Item Description	Unit of Measure	Estimated Quantity
221	861088	MODIFY RAMP METERING SYSTEM	LS	LUMP SUM
222	027689	MODIFY AUTOMATIC VEHICLE CLASSIFICATION STATION	LS	LUMP SUM
223	861504	MODIFY LIGHTING AND SIGN ILLUMINATION	LS	LUMP SUM
224	044692	TUNNELS AND CONTROL BUILDING	LS	LUMP SUM
225	999990	MOBILIZATION	LS	LUMP SUM

SPECIAL PROVISIONS

ORGANIZATION

Special provisions are under headings that correspond with the main-section headings of the *Standard Specifications*. A main-section heading is a heading shown in the table of contents of the *Standard Specifications*.

Each special provision begins with a revision clause that describes or introduces a revision to the *Standard Specifications* as revised by any revised standard specification.

Any paragraph added or deleted by a revision clause does not change the paragraph numbering of the *Standard Specifications* for any other reference to a paragraph of the *Standard Specifications*.

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2 BIDDING

Add to section 2-1.06B:

The Department makes the following supplemental project information available:

Supplemental Project Information

Means	Description
Included in the *Information Handout*	Site investigation report. Geotechnical design report for overhead sign structures, new tunnel control building, and maintenance vehicle pullout. Geotechnical design report for Pavement distress.
Available as specified in the *Standard Specifications*	Bridge as-builts

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5 CONTROL OF WORK

Add to section 5-1.09A:

The Department encourages the project team to exhaust the use of partnering in dispute resolution before engagement of an objective third party.

For certain disputes, a facilitated partnering session or facilitated dispute resolution session may be appropriate and effective in clarifying issues and resolving all or part of a dispute.

To afford the project team enough time to plan and hold the session, a maximum of 20 days may be added to the DRB referral time following the Engineer's response to a *Supplemental Potential Claim Record.*

To allow this additional referral time, the project team must document its agreement and intention in the dispute resolution plan of the partnering charter. The team may further document agreement of any associated criteria to be met for use of the additional referral time.

If the session is not held, the DRB referral time remains in effect as specified in section 5-1.43.

Add to section 5-1.20A:

During the progress of the work under this Contract, work under the following contracts may be in progress at or near the job site of this Contract:

Coincident or Adjacent Contracts

Contract no.	County–Route–Post Mile	Location	Type of work
07-288014	LA-210-R9.7/R16.1	Los Angeles and Glendale from 0.2 mile west of Big Tujunga Wash Bridge to Dunsmore Avenue undercrossing	Replace pavement with PCC and HMA and median concrete barrier
07-4Y0104	LA-210-R14.2/R22.6	Los Angeles County at various locations from La Tuna Canyon Rd undercrossing to 0.1 mile east of Arroyo Blvd. overcrossing	Place RHMA (gap graded)
07-258934	LA-210-R15.0/R36.0	Los Angeles County at various locations from 0.1 mile west of Lowell Avenue undercrossing to 0.1 mile east of Buena Vista Street undercrossing	Construct sand filters and infiltration devices
07-4T5804	LA-210-R24.2	Los Angeles County in Pasadena at westbound Mountain Street on/off ramps	Install traffic signal and curb ramps

Replace "Reserved" in section 5-1.20C with:

This project does not include work on the railroad property, but a railroad is shown on the general plan sheet within the project limits. Do not trespass on the railroad property at Route 210 from Post Mile R25.48 to R26.31.

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6 CONTROL OF MATERIALS

Add to section 6-2.03:

The Department furnishes you with:

- Model 2070 controller assembly, including controller unit, completely wired controller cabinet, Model 2070-6G universal time base module, Model 2070-6B modem, and detector sensor units
- C2 modem harness

The Department furnishes you with completely wired controller cabinets with auxiliary equipment but without controller unit at 7316 Bandini Blvd., Los Angeles CA 90040. At least 48 hours before you pick up the materials, inform the Engineer of what you will pick up and when you will pick it up.

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8 PROSECUTION AND PROGRESS

Replace "Reserved" in section 8-1.04C with:

Section 8-1.04B does not apply.

Start job site activities within 55 days after receiving notice that the Contract has been approved by the Attorney General or the attorney appointed and authorized to represent the Department.

Do not start job site activities until the Department authorizes or accepts your submittal for:

1. CPM baseline schedule
2. WPCP or SWPPP, whichever applies
3. Notification of DRA or DRB nominee and disclosure statement
4. Contingency plan for opening closures to public traffic

You may enter the job site only to measure controlling field dimensions and locate utilities.

Do not start other job site activities until all the submittals from the above list are authorized or accepted and the following information is received by the Engineer:

1. *Notice of Materials To Be Used* form.
2. Written statement from the vendor that the order for the sign panels has been received and accepted by the vendor. The statement must show the dates that the materials will be shipped.
3. Written statement from the vendor that the order for electrical material has been received and accepted by the vendor. The statement must show the dates that the materials will be shipped.
4. Written statement from the vendor that the order for structural steel has been received and accepted by the vendor. The statement must show the dates that the materials will be shipped.

You may start job site activities before the 55th day after Contract approval if you:

1. Obtain specified authorization or acceptance for each submittal before the 55th day
2. Receive authorization to start

Submit a notice 72 hours before starting job site activities. If the project has more than 1 location of work, submit a separate notice for each location.

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9 PAYMENT

Add to section 9-1.16C:

The following items are eligible for progress payment even if they are not incorporated into the work:

1. Drill and bond (dowel bar)
2. Joint seal (MR 1")
3. Joint seal (MR 1 1/2")
4. Pipe (Irrigation Systems)
5. Valves

6. Joint seal (MR 2")
7. Type B joint seal
8. Bar reinforcing steel
9. Reinforced concrete pipe
10. Slotted corrugated steel pipe
11. Perforated plastic pipe underdrain
12. Non-perforated plastic pipe underdrain
13. Rock slope protection fabric (class 8)
14. Miscellaneous iron and steel
15. Chain link fence and gate
16. Midwest guardrail system
17. Double Midwest guardrail system
18. Transition railing (Type WB-31)
19. Crash cushion (Type SCI SMART 100GM)
20. Pavement marker

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The background on construction project funding signs must be Type II retroreflective sheeting on the Authorized Material List for signing and delineation materials.

The legend must be retroreflective, except for nonreflective black letters and numerals. The colors blue and orange must comply with PR Color no. 3 and no. 6, respectively, as specified in the Federal Highway Administration's *Color Tolerance Chart*.

The legend for the type of project on construction project funding signs must read as follows:

HIGHWAY REPAIR

The legend for the types of funding on construction project funding signs must read as follows and in the following order:

FEDERAL HIGHWAY TRUST FUNDS

STATE HIGHWAY FUNDS

The Engineer will provide the year of completion for the legend on construction project funding signs. Furnish and install a sign overlay for the year of completion within 10 working days of notification.

The size of the legend on construction project funding signs must be as described. Do not add any additional information unless authorized.

12-2.03 CONSTRUCTION

Install 2 Type 2 construction project funding signs at the locations designated by the Engineer before starting major work activities visible to highway users.

When authorized, remove and dispose of construction project funding signs upon completion of the project.

12-2.04 PAYMENT

Not Used

Add to section 12-3.12C:

Start displaying the message on the portable changeable message sign 5 minutes before closing the lane.

Replace "Reserved" in section 12-3.13 with:

12-3.13A General

12-3.13A(1) Summary

Section 12-3.13 includes specifications for protecting traffic and workers with an impact attenuator vehicle during moving lane closures and when placing and removing components of stationary lane closures, ramp closures, shoulder closures, or a combination.

Impact attenuator vehicles must comply with the following test levels under National Cooperative Highway Research Program 350:

1. Test level 3 if the preconstruction posted speed limit is 50 mph or more
2. Test levels 2 or 3 if the preconstruction posted speed limit is 45 mph or less

The impact attenuator vehicle must comply with the attenuator manufacturer's instructions for:

1. Support truck, except the weight of the support truck must comply with the allowable vehicle weight limits shown on the Authorized Materials List for highway safety features and the manufacturer's instructions
2. Trailer-mounted attenuator
3. Truck-mounted attenuator

Flashing arrow signs must comply with section 12-3.03 except you may use a portable changeable message sign instead of a flashing arrow sign. If a portable changeable message sign is used as a flashing arrow sign, it must comply with section 6F.56 "Arrow Panels" of the *California MUTCD*.

12-3.13A(2) Definitions

impact attenuator vehicle: Support truck that is towing a deployed attenuator mounted to a trailer or a support truck with a deployed attenuator that is mounted to the support truck.

12-3.13A(3) Submittals

Submit a certificate of compliance for each attenuator used on the project.

12-3.13A(4) Quality Control and Assurance

Do not start impact attenuator vehicle activities until authorized.

Before using an impact attenuator vehicle, conduct a meeting with the Engineer, subcontractors, and other parties involved with traffic control to discuss the operation of the impact attenuator vehicle during moving lane closures and when placing and removing components of a stationary traffic control system.

Schedule the location, time, and date for the meeting with all participants. Furnish a meeting facility located within 5 miles of the job site or at another authorized location.

12-3.13B Materials

Impact attenuator vehicles must be on the Authorized Materials List for highway safety features. Impact attenuator vehicles must comply with Veh Code Div 12.

For the Trinity MPS-350 truck–mounted attenuator, the support truck must not have a fuel tank mounted underneath within 10'-6" of the rear of the support truck.

Each impact attenuator vehicle must have:

1. Inverted "V" chevron pattern placed across the entire rear of the attenuator composed of alternating 4-inch wide nonreflective black stripes and 4-inch wide yellow retroreflective stripes sloping at 45 degrees
2. Type II flashing arrow sign
3. Flashing or rotating amber light
4. Operable 2-way communication system for maintaining contact with workers

12-3.13C Construction

Except where prohibited, use an impact attenuator vehicle:

1. To follow behind equipment and workers who are placing and removing components of a stationary lane closure, ramp closure, shoulder closure, or any combination. Operate the flashing arrow sign in the arrow or caution mode during this activity, whichever applies. Follow at a distance that prevents intrusion into the workspace from passing traffic.
2. As a shadow vehicle in a moving lane closure.

Monitor placement and use of the attenuator vehicle on a regular basis and adjust the use of the attenuator to match changing field conditions as construction progresses.

After placing components of a stationary traffic control system you may place the impact attenuator vehicle in advance of the work area or at another authorized location to protect traffic and workers.

Secure objects, including equipment, tools, and ballast on impact attenuator vehicles to prevent loosening upon impact by an errant vehicle.

Do not use a damaged attenuator in the work. Replace any attenuator damaged from an impact during work activities.

12-3.13D Payment

Not Used

Replace section 12-3.14 with:

12-3.14 TEMPORARY TRAFFIC SCREEN

12-3.14A General

Section 12-3.14 includes specifications for constructing temporary traffic screen.

12-3.14B Materials

Temporary traffic screen panels must be new or used, CDX grade or better, plywood or weather-resistant strandboard mounted and anchored on Type K temporary railing.

Wale boards must be new or used Douglas fir, rough sawn, construction grade or better.

Pipe screen supports must be new or used schedule 40, galvanized steel pipe.

Nuts, bolts, and washers must be cadmium plated.

Screws must be black or cadmium-plated flat head, cross-slotted screws with full thread length.

12-3.14C Construction

Mount and anchor temporary traffic screen on top of Type K temporary railing.

Remove the traffic screen from the highway when the Engineer determines it is no longer required. The traffic screen that is removed becomes your property.

A lateral move of Type K temporary railing with attached temporary traffic screen is change order work if ordered and the repositioning is not shown.

12-3.14D Payment

Temporary traffic screen is measured along the line of the completed screen.

Replace "Reserved" in section 12-3.18 of the RSS for section 12-3.18:

12-3.18A General

Section 12-3.18 includes specifications for furnishing, installing, operating, and maintaining an automated work zone information system (AWIS). The AWIS must be a proven system that has been successfully deployed and operated in actual work zones or congested areas.

The AWIS must acquire traffic data throughout the work zone and display predetermined information to the motorists automatically without operator intervention after system initialization.

Real time information must be displayed to the motorists using a portable changeable message sign (PCMS). The AWIS PCMS must comply with section 12-3.12.

The AWIS must be controlled either locally or remotely, by a dedicated controller or computer.

Authorized users must be able to override motorist information messages, both locally and remotely.

Traffic sensors must not require adjustments after initial deployment.

Use the AWIS for the following construction stage:

1. 2
2. 3

The AWIS must operate one week before start and continuously for the duration of the construction stages or activities requiring its use.

Place detection devices and AWIS PCMSs at the locations shown on Traffic Handling Details Plans.

Assign an on-site system coordinator. The coordinator must be locally available to service, maintain, and relocate system components as necessary. The coordinator must be accessible 24/7 while the system is deployed. If the system fails to perform as specified, perform any necessary remedial work and replace any failed components within 24 hours of notification of system or component failure.

All messages displayed must be authorized before use, including the message content and threshold used for triggering the message.

Provide complete set-up and support for the AWIS PCMS communications.

Provide the Engineer with a user interface for the control of the AWIS PCMS communications. The user interface must be software installed at the District DTM Office compatible with a Windows environment or a Web service accessed by a Web browser. Any software you provide must be furnished on a standard compact disc.

The user interface must, at a minimum, provide the user with a list of AWIS PCMSs in the field, location information for each AWIS PCMS, and a real-time on-board display of the message in the field. Control options must, at a minimum, provide the user the ability to change on-board messages and flash rate.

12-3.18B General System Functions

The general system functions of the AWIS system must be capable of:

1. Preventing any unauthorized users or systems from gaining access to the AWIS PCMSs through an industry authentication and encryption standard level of security.
2. Providing current operational status locally (in the vicinity of the work zone) and remotely. Operational status must include current traffic data and messages, communications system, and power status.
3. Delivering notifications either by telephone, voice, or text messages to alert support staff of trouble conditions.
4. Generating trouble alerts for conditions such as (1) low roadside equipment power or voltage, (2) system communications failure, (3) low speed traffic detected, and (4) excessive delay detected.
5. Adjusting the thresholds of reduced speed and congestion-induced delay at which the system initiates a trouble alert.
6. Allowing programming of the hours during which the trouble condition alerting subsystem initiates notification to authorized users.
7. Measuring periodically and automatically the power levels of all equipment. Alert support staff, locally and remotely via a telephone message, in time to provide supplemental power before the system ceases to operate.
8. Displaying preprogrammed messages based on time of day and day of week.

12-3.18C Motorist Information Messages

The AWIS system must be capable of:

1. Displaying predetermined speed, delay, diversion, and lane closure messages to motorists when user adjustable thresholds are exceeded.
2. Updating its speed and delay advisory PCMS messages at least once per minute. Actual message updates must be consistent with traffic conditions.
3. Selecting messages for each PCMS independently, based on the traffic conditions downstream of the PCMS.
4. Recording motorist information messages in comma-separated value (CSV) file format with time and date stamps including message overrides with user ID.
5. Displaying default messages when traffic conditions, system algorithms, and user parameters do not dictate that an advisory message should be displayed.
6. Displaying separate, independent default-messages on each PCMS.
7. Analyzing traffic parameters in work zones in which there are multiple speed limits.

The following parameters related to selection and presentation of motorist information messages must be user adjustable:

1. Message update frequency
2. Minimum delay necessary to trigger a delay advisory message
3. Persistence of delay before a delay message is displayed
4. Level of delay required to trigger a diversion message
5. Change in delay needed to cause a delay advisory message update
6. Change in downstream speed at which a speed advisory message update occurs

12-3.18D System Communications

The AWIS wireless communications subsystem must:

1. Be able to operate independently of the public cellular phone system for incoming data to insure the reliability of communications
2. Communicate independent of line of sight or distance.
3. Incorporate an error detection and correction mechanism to insure the integrity of all traffic conditions data and motorist information messages
4. Configure automatically during system initialization

12-3.18E Traffic Data Acquisition

The AWIS system must collect accurate traffic data:

1. Using an accurate speed measurement technique that will allow highly-specific motorist information messages. Speed must be measured within 5 mph accuracy.
2. Use microwave sensors to perform continuous accurate traffic counts over 3 or 4 freeway lanes of traffic.
3. During reduced visibility conditions including precipitation, fog, darkness, excessive dust, and road debris.
4. That is archived in with time and date stamps. Archived speed data must be able to be aggregated into operator definable time increments (e.g., 30, 60 seconds, etc.), and accessible 24/7 to the Engineer in CSV file format.

Replace the 1st paragraph of section 12-4.02A with:

Work that interferes with traffic is limited to the hours when closures are allowed, except for work shown on the stage construction and traffic handling plans.

Delete the 2nd through 5th paragraphs of section 12-4.02A.

Replace the 6th paragraph of section 12-4.02A with:

If a minor deviation from the requirements of this section regarding hours of work is required, submit a request at least 15 days before the proposed date of closure. If no significant increase in cost is accrued to the Department and the work can be expedited and better serve the traffic, the deviation may be authorized.

Add to section 12-4.02A:

If work including installing, maintaining, and removing Category 3 traffic control devices except impact attenuator vehicles is to be performed within 6 feet of the adjacent traffic lane, close the adjacent traffic lane.

Closure of the adjacent traffic lane is not required for installing, maintaining, and removing Category 1 and 2 traffic control devices.

For grinding and grooving operations, sawcutting concrete slabs, and installing loop detectors, closure of the adjacent traffic lane is not required if an impact attenuator vehicle is used as a shadow vehicle.

The full width of the ramp traveled-way must be open for use by traffic on designated holidays.

Designated holidays are shown in the following table:

Designated Holidays

Holiday	Date observed
New Year's Day	January 1st
Washington's Birthday	3rd Monday in February
Memorial Day	Last Monday in May
Independence Day	July 4th
Labor Day	1st Monday in September
Veterans Day	November 11th
Thanksgiving Day	4th Thursday in November
Christmas Day	December 25th

If a designated holiday falls on a Sunday, the following Monday is a designated holiday. If November 11th falls on a Saturday, the preceding Friday is a designated holiday.

Special days are: Martin Luther King Jr and Columbus Day.

Concurrent stationary lane closures must be more than 5 miles apart. Closure spacing is the distance between the last cone of the upstream closure and the temporary sign (W20-1) of the downstream closure. The number of lanes open in the upstream closures must be less than or equal to the number of lanes open in the downstream closures. For multiple closures in each direction of travel, pick up downstream closures first.

Work may be performed during the hours designated as "No work allowed" on Charts no. 1 through 3 and 6 through 10 if temporary traffic screens are installed on top of the Type K temporary railings. Traffic screens are not allowed on the right side of traffic within the limits of a right horizontal curve with a radius of less than 2,000 feet as viewed in the direction of travel and on the left side of traffic within the limits of a left horizontal curve with a radius of less than 2,000 feet as viewed in the direction of travel. The Department does not pay for furnishing, installing, maintaining, or removing temporary traffic screen.

If traffic control is performed for a stationary lane closure where median shoulders are less than 8 feet, comply with the traffic handling details plan titled, "Traffic Control System for Median Shoulders Less Than 8 Feet," in addition to the specifications in sections 12-3.13 and 12-4.02.

HOV lanes may be closed any time the adjacent freeway lane is allowed to be closed as shown on Charts no. 4 and 5. Contact the following agencies at least 5 business days before closing an HOV lane:

Agency	Telephone no.
Metropolitan Transportation Authority (Metro)	(213) 922-4632

If removing and replacing concrete pavement and approach slabs, place a C43(CA) sign at the beginning and every 500 feet throughout the length of the closure during the entire curing period.

Route 2 and 210 may be closed to traffic at 1 location in each direction of travel for 2-post overhead sign structure installation and removal, striping, and loop detector installation operations. Comply with the hours and requirements shown on Charts no. 11 through 19. When installing loop detectors on Route 210 at station 1327+00 and 1357+80, close freeway lanes as shown on the traffic handling details plan titled "Traffic Control System for "Flip-Flop" Operations."

As shown on the Stage Construction and Traffic Handling plans Stage 2 phase 1A through 1D, Stage 3 Phase 1A through 1D and Stage 4 phase 1A and 1B you will be permitted to close ramps for one extended 45 days period to construct jointed plain concrete pavement on adjacent freeway lanes and all ramp work shown. Ramp locations and corresponding stage and phase of continuous closure are shown in the table below. Lincoln Ave/Howard St ramps and Mountain St ramps must not be closed continuously from September 1st through January 5th.

Ramp	Route 210 Direction	Stage 2	Stage 3	Stage 4
Pennsylvania Ave off-ramp	Eastbound	Phase 1A		
Pennsylvania Ave on-ramp	Eastbound	Phase 1A		
Pennsylvania Ave off-ramp	Westbound		Phase 1A	
Pennsylvania Ave on-ramp	Westbound		Phase 1A	
La Crescenta Ave on-ramp	Eastbound	Phase 1B		
La Crescenta Ave off-ramp	Westbound	Phase 1C		
Ocean View Blvd on-ramp	Eastbound		Phase 1A	
Ocean View Blvd off-ramp	Westbound	Phase 1B		
Ocean View Blvd on-ramp	Westbound	Phase 1C		
SB Angeles Crest Hwy on-ramp	Eastbound		Phase 1B	
NB Angeles Crest Hwy on-ramp	Eastbound			Phase 1A
Angeles Crest Hwy off-ramp	Westbound	Phase 1D		
NB Angeles Crest Hwy on-ramp	Westbound	Phase 1B		
Gould Ave on-ramp	Westbound	Phase 1D		
Foothill Blvd on-ramp	Eastbound			Phase 1B
Foothill Blvd off-ramp	Westbound		Phase 1C	
Berkshire Ave on-ramp	Eastbound	Phase 1A		
Berkshire Ave off-ramp	Westbound		Phase 1B	
Berkshire Ave on-ramp	Westbound		Phase 1C	
Arroyo Blvd off-ramp	Eastbound	Phase 1A		
Arroyo Blvd on-ramp	Eastbound		Phase 1A	
Arroyo Blvd off-ramp	Westbound		Phase 1D	
Lincoln Ave/Howard St off-ramp	Eastbound		Phase 1A	
Lincoln Ave/Howard St on-ramp	Eastbound		Phase 1B	
Lincoln Ave/Howard St off-ramp	Westbound	Phase 1C		
Lincoln Ave/Howard St on-ramp	Westbound		Phase 1D	
Mountain St off-ramp	Eastbound		Phase 1B	
Mountain St on-ramp	Eastbound		Phase 1C	
Mountain St on-ramp	Westbound	Phase 1C		

During extended ramp closures, public traffic must be detoured as shown on the Motorist Information Plans. No temporary ramp closures will be permitted if they conflict with extended ramp closure detours. You must submit a written request to the Engineer at least 30 days prior to the start of every ramp extended closure.

The eastbound Route 210 to eastbound Rte 210 and westbound Route 134 connectors may be closed from 1900 Saturday to 0500 the following Monday as shown on THD-16 when placing precast concrete panels. Place portable changeable message signs and post detour signs at locations designated on Motorist Information Plan MI-25. During extended closures, close the eastbound Route 210 on-ramps from Lincoln Blvd and Mountain St and detour ramp traffic under Chart nos. 66 and 70.

The westbound Route 210 and eastbound Route 134 to westbound Route 210 connectors may be closed from 2200 Friday to 0500 the following Monday as shown on THD-15 when placing precast concrete panels. Place portable changeable message signs and post detour signs at locations designated on Motorist Information Plan MI-26. During extended closures, close the westbound Route 210 on-ramp from Walnut St and the off-ramp to Mountain St and detour ramp traffic under Chart nos. 74.

Two extended closures for each connector are allowed and you may only work in one direction at a time. Extended closures are not permitted during Stages 2 and 3.

You must submit a written request to the Engineer at least 60 days before starting of an extended connector closure.

A Construction Management Plan is required in the extended closure request. The Construction Management Plan must include a general time-scaled logic diagram displaying the major activities, sequence of planned operations, and a contingency plan at each stage of operations to prevent late opening of extended weekend closure. Identify "early finish" and "late finish" milestones for every major activity. Include contingencies if a major activity passes the “late finish” milestones. If lanes are not open to public traffic at the end of an extended closure, approvals of Construction Management Plans for remaining extended weekend closures will be invalid.

The Engineer has 15 days to review Construction Management Plans. If the Engineer determines revisions are required, revise and resubmit Construction Management Plans within 15 days of receipt of the Engineer’s comments. The Engineer has 7 days to review revisions. Do not proceed with extended weekend closures until the Engineer issues written approval of final Construction Management Plans 7 days before the planned extended closure.

Except as otherwise specified on Charts no. 33 through 74, other ramps may be closed if the adjacent freeway lane is allowed to be closed as shown on Charts no. 4 and 5. If a ramp is closed and a ramp lane requirement chart is not included, detour traffic to the next available ramp downstream of the closed ramp in the direction of travel. For each ramp closed, post at least 12 special portable freeway detour signs, SP-2, as shown on traffic handling details plan titled "Traffic Control System for Detour Sign Installation along Designated Detour Route," along the detour route and remove at the end of each closure.

Except as otherwise specified, closure of on-ramps or off-ramps servicing 2 consecutive local street interchanges in the same direction of travel is not allowed. A request for deviation from this rule must be submitted for authorization. The Engineer may allow the deviation if traffic will be better served and the work is expedited. If 2 or more consecutive on-ramps are allowed or are specified to be closed, install special signs for entrance ramp closures, SP-4, as shown. The Department does not pay for furnishing, installing, maintaining, or removing SP-4 signs. If an off-ramp is closed, install special signs for exit ramp closures, SP-3 or SP-5, and place the sign on the right shoulder of the freeway upstream of the preceding off-ramp.

If a ramp or connector closure is allowed, post a special advance notice publicity sign, SP-1, as shown at an authorized location, at least 7 days before the ramp or connector closure. Maintain accurate information on the sign and remove or cover sign when work is not actively in progress.

From 3 hours before to 2 hours after events at the venues or special events shown in the table titled "Special Events and Venues," work that encroaches onto the freeway or connector traveled way or the following ramps will not be allowed:

1. Angeles Crest Hwy ramps
2. Gould Ave ramps
3. Foothill Blvd ramps
4. Berkshire Ave ramps
5. Arroyo Blvd ramps
6. Lincoln Ave/Howard St ramps
7. Mountain St ramps
8. Walnut St ramps
9. Del Mar Blvd/Colorado Blvd ramps

Special events and venues are as follows:

Special Events and Venues

Venue/Special event	Affected routes	Route limits
Rose Bowl Events	210	Route 2 to Route 605
Rose Parade	210	Route 134 to Rosemead Blvd

Do not perform work on city streets that interferes with traffic between 0600 and 0900 or between 1500 and 1900 hours.

Personal vehicles of the Contractor's employees must not be parked within the right-of-way.

If work vehicles or equipment are parked within 6 feet of a traffic lane, close the shoulder area as shown.

Add to the RSS for section 12-4.03B:

For each 10-minute interval or fraction thereof past the time specified to open the closure, the Department deducts the amount for liquidated damages per interval shown in the table below. Liquidated damages are limited to 5 percent of the total bid per occurrence. Liquidated damages are not assessed if the Engineer orders the closure to remain in place beyond the scheduled pickup time.

Type of facility	Route	Direction or Segment	Period	Liquidated damages/interval ($)
Mainline	2	NB from Mountain St to Rte 210	1st half hour and beyond	$1,000 / 10 minutes
Mainline	210	EB from Lowell Ave to Rte 2	1st half hour 2nd half hour 2nd hour and beyond	$3,000 / 10 minutes $7,100 / 10 minutes $9,500 / 10 minutes
Mainline	210	EB from Rte 2 to Angeles Crest Hwy	1st half hour 2nd half hour 2nd hour and beyond	$1,500 / 10 minutes $2,200 / 10 minutes $2,900 / 10 minutes
Mainline	210	EB from Angeles Crest Hwy to Mountain	1st half hour 2nd half hour 2nd hour and beyond	$2,100 / 10 minutes $3,200 / 10 minutes $4,300 / 10 minutes
Mainline	210	EB from 210/134 Sep to Los Robles	1st half hour 2nd half hour 2nd hour and beyond	$3,000 / 10 minutes $6,200 / 10 minutes $8,200 / 10 minutes
Mainline	210	WB from Los Robles to 210/134 Sep	1st half hour 2nd half hour 2nd hour and beyond	$3,000 / 10 minutes $6,800 / 10 minutes $9,100 / 10 minutes
Mainline	210	WB from 210/134 Sep to Lowell Ave	1st half hour and beyond	$1,000 / 10 minutes $1,300 / 10 minutes $1,700 / 10 minutes
Connector	210	East 210 to South 2	1st half hour 2nd half hour 2nd hour and beyond	$3,100 / 10 minutes $4,600 / 10 minutes $6,100 / 10 minutes
Connector	210	North 2 to East 210 West 210 to 110 West 210 to South 2 East 210 to Ramps East 210 to West 134 East 134 to West 210 East 134 to West 210 & South 710 North 710 to East 210 North 710 to West 134	1st half hour and beyond	$1,000 / 10 minutes
Connector	210	North 2 to West 210	1st half hour 2nd half hour 2nd hour and beyond	$1,300 / 10 minutes $1,400 / 10 minutes $1,800 / 10 minutes
Connector	210	East 210 to East 210	1st half hour 2nd half hour 2nd hour and beyond	$1,500 / 10 minutes $2,300 / 10 minutes $3,200 / 10 minutes
Connector	210	West 210 to West 210	1st half hour 2nd half hour 2nd hour and beyond	$1,300 / 10 minutes $1,800 / 10 minutes $2,400 / 10 minutes

Replace "Sunday" at each occurrence in the 1st paragraph of the RSS for section 12-4.03B with:

Friday

Add to the RSS for section 12-4.03C:

Submit a contingency plan for each of the following activities:

1. Complete freeway closure
2. Rapid set concrete work
3. Asphalt or concrete grinding
4. Bridge work
5. Striping
6. Asphalt or concrete pavement sealing
7. Precast jointed concrete pavement lane replacement

Discuss the contingency plan with the Engineer at least 5 business days before starting the activity.

Replace "Reserved" in section 12-4.04 with:

Freeway or Connector Lane Closure Restriction for Designated Holidays and Special Days

Thu	Fri	Sat	Sun	Mon	Tues	Wed	Thu	Fri	Sat	Sun
x	**H** xx	xx	xx							
x	xx	**H** xx	xx							
	x	xx	xx	**H** xx						
	x			**SD** xx						
				x	**H** xx					
					x	**H** xx				
						x	**H** xx	xx		xx

Legend:

	Refer to Charts no. 1 through 32.
x	The full width of the traveled way must be open for use by traffic by 0600.
xx	The full width of the traveled way must be open for use by traffic.
H	Designated holiday
SD	Special day

Replace "Reserved" in section 12-4.05B with:

Chart no. 1
Freeway Lane Requirements and Hours of Work

County: LA | Route/Direction: 210 / East

Closure limits: Lowell Ave off-ramp to South 2 off-connector

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	1	1	1	1	1	2	N	N	N	N	S	S	S	S	S	N	N	N	N	3	2	2	2	1	
Fri	1	1	1	1	1	2	N	N	N	N	S	S	S	S	S	N	N	N	N	3	3	2	2	1	
Sat	1	1	1	1	1	1	2	3	S	S	S	S	S	S	S	S	S	S	3	3	2	2	2	2	
Sun	1	1	1	1	1	1	1	2	2	3	3	3	3	3	3	3	3	3	3	3	2	2	2	1	

Legend:

1	Provide at least 1 through freeway lane open in direction of travel
2	Provide at least 2 adjacent through freeway lanes open in direction of travel
3	Provide at least 3 adjacent through freeway lanes open in direction of travel
S	Shoulder closure allowed
N	No work allowed

REMARKS: The number of through traffic lanes is 4.
Traffic lanes outside of the through traffic lanes delineated with a double line of pavement markers as shown on "Pavement Markers and Traffic Lines Typical Details (Detail 37 series)," may be closed at the same time the adjacent through traffic lane connector or ramp is allowed to be closed as shown on Chart no. 20 & 39.
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.

Chart no. 2
Freeway Lane Requirements and Hours of Work

County: LA | Route/Direction: 210 / East

Closure limits: South 2 off-connector to Angeles Crest Hwy OC

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	1	1	1	1	1	2	N	N	N	3	3	3	3	3	3	N	N	N	N	3	2	2	2	1	
Fri	1	1	1	1	1	2	N	N	N	3	3	3	3	3	3	N	N	N	N	3	2	2	2	1	
Sat	1	1	1	1	1	1	2	2	3	3	3	3	3	3	3	3	3	3	3	2	2	2	2	1	
Sun	1	1	1	1	1	1	1	1	2	2	2	2	2	3	3	3	3	3	2	2	2	2	1	1	

Legend:

1	Provide at least 1 through freeway lane open in direction of travel
2	Provide at least 2 adjacent through freeway lanes open in direction of travel
3	Provide at least 3 adjacent through freeway lanes open in direction of travel
N	No work allowed

REMARKS: The number of through traffic lanes is 4.
Traffic lanes outside of the through traffic lanes delineated with a double line of pavement markers as shown on "Pavement Markers and Traffic Lines Typical Details (Detail 37 series)," may be closed at the same time the adjacent through traffic lane connector or ramp is allowed to be closed as shown on Chart no. 21 & 49.
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.

Chart no. 3
Freeway Lane Requirements and Hours of Work

County: LA | Route/Direction: 210 / East

Closure limits: Angeles Crest Hwy OC to Mountain St UC

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	1	1	1	1	1	2	N	N	N	N	S	S	S	S	N	N	N	N	N	3	2	2	2	1	
Fri	1	1	1	1	1	2	N	N	N	N	S	S	S	S	N	N	N	N	N	3	2	2	2	2	
Sat	1	1	1	1	1	1	2	2	3	3	3	S	S	S	S	S	S	3	3	2	2	2	2	2	
Sun	1	1	1	1	1	1	1	2	2	2	3	3	3	3	3	2	2	2	2	2	2	2	1	1	

Legend:

1	Provide at least 1 through freeway lane open in direction of travel
2	Provide at least 2 adjacent through freeway lanes open in direction of travel
3	Provide at least 3 adjacent through freeway lanes open in direction of travel
S	Shoulder closure allowed
N	No work allowed

REMARKS: The number of through traffic lanes is 4.
Traffic lanes outside of the through traffic lanes delineated with a double line of pavement markers as shown on "Pavement Markers and Traffic Lines Typical Details (Detail 37 series)," may be closed at the same time the adjacent through traffic lane ramp is allowed to be closed as shown on Chart no. 53, 57, 61, 62, 65 & 69.
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.

Chart no. 4
Freeway Lane Requirements and Hours of Work

County: LA | Route/Direction: 210 / East

Closure limits: Route 210/134 Sep to Los Robles Ave OC

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	2	1	1	1	1	2	N	N	N	N	N	N	N	N	N	N	N	N	N	N	4	4	3	2	
Fri	2	1	1	1	1	2	N	N	N	N	N	N	N	N	N	N	N	N	N	N	S	4	3	3	
Sat	2	2	1	1	1	1	2	4	S	N	N	N	N	N	N	N	N	N	N	S	4	3	3	3	
Sun	2	2	2	1	1	1	2	2	3	4	S	N	N	N	N	N	N	N	S	4	4	3	3	2	

Legend:

1	Provide at least 1 through freeway lane open in direction of travel
2	Provide at least 2 adjacent through freeway lanes open in direction of travel
3	Provide at least 3 adjacent through freeway lanes open in direction of travel
4	Provide at least 4 adjacent through freeway lanes open in direction of travel
S	Shoulder closure allowed
N	No work allowed

REMARKS: The number of through traffic lanes is 4, 5, 6 and does not include HOV lane.
Traffic lanes outside of the through traffic lanes delineated with a double line of pavement markers as shown on "Pavement Markers and Traffic Lines Typical Details (Detail 37 series)," may be closed at the same time the adjacent through traffic lane connector or ramp is allowed to be closed.

Chart no. 5
Freeway Lane Requirements and Hours of Work

County: LA | Route/Direction: 210 / West

Closure limits: Los Robles Ave OC to Route 210/134 Sep

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	2	1	1	2	2	S	N	N	N	N	N	N	N	N	N	N	N	N	N	N	4	4	3	2	
Fri	2	1	1	2	2	S	N	N	N	N	N	N	N	N	N	N	N	N	N	N	4	4	4	3	
Sat	2	2	1	1	1	2	3	4	S	S	N	N	N	N	N	N	N	N	N	S	4	4	4	3	
Sun	2	2	1	1	1	1	2	2	3	4	S	N	N	N	N	N	N	N	N	S	4	4	3	2	

Legend:

1	Provide at least 1 through freeway lane open in direction of travel
2	Provide at least 2 adjacent through freeway lanes open in direction of travel
3	Provide at least 3 adjacent through freeway lanes open in direction of travel
4	Provide at least 4 adjacent through freeway lanes open in direction of travel
S	Shoulder closure allowed
N	No work allowed

REMARKS: The number of through traffic lanes is 4, 5 and does not include HOV lane.
Traffic lanes outside of the through traffic lanes delineated with a double line of pavement markers as shown on "Pavement Markers and Traffic Lines Typical Details (Detail 37 series)," may be closed at the same time the adjacent through traffic lane connector or ramp is allowed to be closed-.

Chart no. 6
Freeway Lane Requirements and Hours of Work

County: LA | Route/Direction: 210 / West

Closure limits: Walnut St OC to West 210 on-connector

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	1	1	1	1	1	1	N	N	N	1	1	1	1	1	1	N	N	N	N	1	1	1	1	1	
Fri	1	1	1	1	1	1	N	N	N	1	1	1	1	1	1	N	N	N	N	1	1	1	1	1	
Sat	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	
Sun	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	

Legend:

1	Provide at least 1 through freeway lane open in direction of travel
N	No work allowed

REMARKS: The number of through traffic lanes is 3.
Traffic lanes outside of the through traffic lanes delineated with a double line of pavement markers as shown on "Pavement Markers and Traffic Lines Typical Details (Detail 37 series)," may be closed at the same time the adjacent through traffic lane connector or ramp is allowed to be closed.
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.

Chart no. 7
Freeway Lane Requirements and Hours of Work

County: LA | Route/Direction: 210 / West

Closure limits: West 210 on-connector to Mountain St UC

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	1	1	1	1	1	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	2	
Fri	1	1	1	1	1	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	2	
Sat	1	1	1	1	1	1	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	
Sun	1	1	1	1	1	1	1	2	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	1	

Legend:

1	Provide at least 1 through freeway lane open in direction of travel
2	Provide at least 2 adjacent through freeway lanes open in direction of travel
N	No work allowed

REMARKS: The number of through traffic lanes is 5.
Traffic lanes outside of the through traffic lanes delineated with a double line of pavement markers as shown on "Pavement Markers and Traffic Lines Typical Details (Detail 37 series)," may be closed at the same time the adjacent through traffic lane connector or ramp is allowed to be closed as shown on Chart no. 27, 31 & 71.

Chart no. 8
Freeway Lane Requirements and Hours of Work

County: LA | Route/Direction: 210 / West

Closure limits: Mountain St UC to Angeles Crest Hwy OC

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	1	1	1	1	1	2	N	N	N	N	N	N	N	N	N	N	N	N	N	3	3	3	2	2	
Fri	1	1	1	1	1	2	N	N	N	N	N	N	N	N	N	N	N	N	N	3	3	3	2	2	
Sat	1	1	1	1	1	1	2	2	2	3	3	N	N	N	N	N	N	N	N	3	3	3	2	2	
Sun	1	1	1	1	1	1	1	2	2	2	2	2	3	N	N	N	N	N	3	3	3	2	2	1	

Legend:

1	Provide at least 1 through freeway lane open in direction of travel
2	Provide at least 2 adjacent through freeway lanes open in direction of travel
3	Provide at least 3 adjacent through freeway lanes open in direction of travel
N	No work allowed

REMARKS: The number of through traffic lanes is 4.
Traffic lanes outside of the through traffic lanes delineated with a double line of pavement markers as shown on "Pavement Markers and Traffic Lines Typical Details (Detail 37 series)," may be closed at the same time the adjacent through traffic lane connector or ramp is allowed to be closed as shown on Chart no. 51, 54, 56, 59, 63 & 67.

Chart no. 9
Freeway Lane Requirements and Hours of Work

County: LA | Route/Direction: 210 / West

Closure limits: Angeles Crest Hwy OC to North 2 on-connector

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	1	1	1	1	1	2	N	N	N	3	3	3	3	3	3	N	N	N	N	3	2	2	2	1	
Fri	1	1	1	1	1	2	N	N	N	3	3	3	3	3	N	N	N	N	N	3	2	2	2	2	
Sat	1	1	1	1	1	1	1	2	2	3	3	3	3	S	S	S	S	3	3	3	3	2	2	2	
Sun	2	1	1	1	1	1	1	2	2	2	3	3	3	S	S	S	S	S	3	3	3	2	2	1	

Legend:

1	Provide at least 1 through freeway lane open in direction of travel
2	Provide at least 2 adjacent through freeway lanes open in direction of travel
3	Provide at least 3 adjacent through freeway lanes open in direction of travel
S	Shoulder closure allowed
N	No work allowed

REMARKS: The number of through traffic lanes is 4.
Traffic lanes outside of the through traffic lanes delineated with a double line of pavement markers as shown on "Pavement Markers and Traffic Lines Typical Details (Detail 37 series)," may be closed at the same time the adjacent through traffic lane connector is allowed to be closed as shown on Chart no. 23.
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.

Chart no. 10
Freeway Lane Requirements and Hours of Work

County: LA | Route/Direction: 210 / West

Closure limits: North 2 on-connector to Lowell Ave off-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	1	1	1	1	1	2	N	N	N	3	3	3	3	3	4	N	N	N	N	4	3	3	2	2	
Fri	1	1	1	1	1	2	N	N	N	3	3	3	3	4	4	N	N	N	N	4	3	3	2	2	
Sat	2	1	1	1	1	1	2	2	2	3	3	3	4	4	4	4	4	4	4	3	3	3	2	2	
Sun	2	1	1	1	1	1	1	1	2	2	3	3	3	3	3	3	4	4	4	3	3	2	2	1	

Legend:

1	Provide at least 1 through freeway lane open in direction of travel
2	Provide at least 2 adjacent through freeway lanes open in direction of travel
3	Provide at least 3 adjacent through freeway lanes open in direction of travel
4	Provide at least 4 adjacent through freeway lanes open in direction of travel
N	No work allowed

REMARKS: The number of through traffic lanes is 5, 6.
Traffic lanes outside of the through traffic lanes delineated with a double line of pavement markers as shown on "Pavement Markers and Traffic Lines Typical Details (Detail 37 series)," may be closed at the same time the adjacent through traffic lane ramp is allowed to be closed as shown on Chart no. 38 & 44. The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.

Replace "Reserved" in section 12-4.05C with:

Chart no. 11
Complete Freeway Closure Hours

County: LA | Route/Direction: 2 / North

Closure limits: Mountain St/Camino San Rafael to Route 210

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C																				
Fri	C	C	C	C	C																				
Sat	C	C	C	C	C	C																			
Sun	C	C	C	C	C	C	C																		

Legend:

C	Freeway may be closed completely
	No complete freeway closure is allowed

REMARKS:
Detour all traffic to exit at Mountain St off-ramp; west on Mountain St; north on Verdugo Rd; north on Montrose Ave; north on Ocean View Blvd to the on-ramp to Route 210.
Place a portable changeable message sign on the right shoulder of northbound Route 2 in advance of the closure with the message: "FREEWAY / CLOSED / AHEAD".
Post at least 28 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.

Chart no. 12
Complete Freeway Closure Hours

County: LA	Route/Direction: 210 / East	

Closure limits: Lowell Ave to Route 2

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C																				
Fri	C	C	C	C	C																				
Sat	C	C	C	C	C	C																			
Sun	C	C	C	C	C	C	C																		

Legend:

C	Freeway may be closed completely
	No complete freeway closure is allowed

REMARKS:
Detour traffic to exit at Lowell Ave off-ramp; north on Lowell Ave; east on Honolulu Ave; north on Pennsylvania Ave; east on Foothill Blvd to the on-ramp to eastbound Route 210. Place a portable changeable message sign on the right shoulder of eastbound Route 210 in advance of the closure with the message: "FREEWAY / CLOSED – LOWELL / TO / RTE 2".
Close all ramps as necessary within the closure and detour traffic to the next available ramp downstream of the closed ramps in the direction of travel.
Post at least 28 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.

Chart no. 13
Complete Freeway Closure Hours

County: LA	Route/Direction: 210 / East	

Closure limits: Route 2 to Berkshire Place

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C																				
Fri	C	C	C	C	C																				
Sat	C	C	C	C	C	C																			
Sun	C	C	C	C	C	C	C																		

Legend:

C	Freeway may be closed completely
	No complete freeway closure is allowed

REMARKS:
Detour traffic to exit at Ocean View Blvd off-ramp; north on Ocean View Blvd; east on Foothill Blvd; south on Oak Grove Dr; west on Berkshire Pl to the on-ramp to eastbound Route 210. Place a portable changeable message sign on the right shoulder of eastbound Route 210 in advance of the Ocean View Blvd off-ramp with the message: "E210 / CLOSED – RTE 2 / TO / BERKSHIRE".
Close all ramps/connector as necessary within the closure and detour traffic to the next available ramp downstream of the closed ramps/connector in the direction of travel.
Post at least 28 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.

Chart no. 14
Complete Freeway Closure Hours

County: LA | Route/Direction: 210 / East

Closure limits: Berkshire Place to Mountain St

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C																					
Fri	C	C	C	C	C																				
Sat	C	C	C	C	C	C																			
Sun	C	C	C	C	C	C	C																		

Legend:

C	Freeway may be closed completely
	No complete freeway closure is allowed

REMARKS:
Detour traffic to exit at Berkshire Ave off-ramp; east on Berkshire Pl; east on Oak Grove Dr/Woodbury Rd; south on Fair Oaks Ave; west on Mountain St to the on-ramp to eastbound Route 210. Place a portable changeable message sign on the right shoulder of eastbound Route 210 in advance of the closure with the message: "FREEWAY / CLOSED – BERKSHIRE / TO / MOUNTAIN".
Close all ramps as necessary within the closure and detour traffic to the next available ramp downstream of the closed ramps in the direction of travel.
Post at least 28 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.

Chart no. 15
Complete Freeway Closure Hours

County: LA	Route/Direction: 210 / East	

Closure limits: Mountain St to the 210/134 Separation

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C																					
Fri	C	C	C	C	C																				
Sat	C	C	C	C	C	C																			
Sun	C	C	C	C	C	C	C																		

Legend:

C	Freeway may be closed completely
	No complete freeway closure is allowed

REMARKS:
Detour traffic to exit at Mountain St off-ramp; east on Mountain St; south on Fair Oaks Ave; east on Corson St to the on-ramp to eastbound Route 210. Place a portable changeable message sign on the right shoulder of eastbound Route 210 in advance of the closure with the message: "FREEWAY / CLOSED – MOUNTAIN / TO / RTE 134".
Close all ramps as necessary within the closure and detour traffic to the next available ramp downstream of the closed ramps in the direction of travel.
Post at least 28 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.

Chart no. 16
Complete Freeway Closure Hours

County: LA | Route/Direction: 210 / West

Closure limits: Walnut St OC to Mountain St

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C																				
Fri	C	C	C	C	C																				
Sat	C	C	C	C	C	C																			
Sun	C	C	C	C	C	C	C																		

Legend:

C	Freeway may be closed completely
	No complete freeway closure is allowed

REMARKS:
(EB) California Blvd: detour traffic to continue eastbound on California Blvd; north on Fair Oaks Ave; west on Mountain St to the on-ramp to westbound Route 210. Place a portable changeable message sign on the right shoulder of eastbound California Blvd in advance of the California/Pasadena intersection with the message: "WEST 210 / CLOSED - AT / MOUNTAIN".
(WB) California Blvd: detour traffic to continue westbound on California Blvd; north on Orange Grove Blvd; north on Fair Oaks Ave; west on Mountain St to the on-ramp to westbound Route 210. Place a portable changeable message sign on the right shoulder of westbound California Blvd in advance of the California/Pasadena intersection with the message: "WEST 210 / CLOSED - AT / MOUNTAIN".
(NB) Pasadena Ave: detour traffic onto westbound Route 134; northbound Route 2 to westbound 210. Place a portable changeable message sign at the gore on-ramp area with the message: "WEST 210 / CLOSED - USE / W 134 / TO N2".
Close all ramps/connectors as necessary within the closure and detour traffic to the next available ramp downstream of the closed ramp/connector in the direction of travel.
Post at least 28 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.

Chart no. 17
Complete Freeway Closure Hours

County: LA | Route/Direction: 210 / West

Closure limits: Mountain St to Berkshire Pl

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C																				
Fri	C	C	C	C	C																				
Sat	C	C	C	C	C	C																			
Sun	C	C	C	C	C	C	C																		

Legend:

C	Freeway may be closed completely
	No complete freeway closure is allowed

REMARKS:
Detour traffic to exit at Mountain St off-ramp; east on Mountain St; north on Fair Oaks Ave; west on Woodbury Rd/Oak Grove Dr; west on Berkshire Pl to the on-ramp to westbound Route 210. Place a portable changeable message sign on the right shoulder of westbound Route 210 in advance of the closure with the message: "FREEWAY / CLOSED – MOUNTAIN / TO / BERKSHIRE".
Close all ramps as necessary within the closure and detour traffic to the next available ramp downstream of the closed ramps in the direction of travel.
Post at least 12 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.

Chart no. 18
Complete Freeway Closure Hours

County: LA | Route/Direction: 210 / West

Closure limits: Berkshire Pl to Route 2

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C																				
Fri	C	C	C	C	C																				
Sat	C	C	C	C	C	C																			
Sun		C	C	C	C	C	C																		

Legend:

C	Freeway may be closed completely
	No complete freeway closure is allowed

REMARKS:
Detour traffic to exit at Berkshire Ave off-ramp; east on Berkshire Pl; north on Oak Grove Dr; west on Foothill Blvd; south on Ocean View Blvd to the on-ramp to westbound Route 210. Place a portable changeable message sign on the right shoulder of westbound Route 210 in advance of the closure with the message: "FREEWAY / CLOSED – BERKSHIRE / TO / RTE 2".
Close all ramps as necessary within the closure and detour traffic to the next available ramp downstream of the closed ramps in the direction of travel.
Post at least 12 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.

Chart no. 19
Complete Freeway Closure Hours

County: LA | Route/Direction: 210 / West

Closure limits: Route 2 to Lowell Ave

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C																				
Fri	C	C	C	C	C																				
Sat		C	C	C	C	C																			
Sun		C	C	C	C	C	C																		

Legend:

C	Freeway may be closed completely
	No complete freeway closure is allowed

REMARKS:
Detour traffic to exit at Angeles Crest Hwy off-ramp; south on Angeles Crest Hwy; west on Foothill Blvd; south on Lowell Ave; west on Honolulu Ave to the on-ramp to westbound Route 210. Place a portable changeable message sign on the right shoulder of westbound Route 210 in advance of the Angeles Crest Hwy off-ramp with the message: "FREEWAY / CLOSED – RTE 2 / TO / LOWELL".
Close all ramps/connector as necessary within the closure and detour traffic to the next available ramp downstream of the closed ramps/connector in the direction of travel.
Post at least 12 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.

Replace "Reserved" in section 12-4.05D with:

Chart no. 20
Connector Lane Requirements and Hours of Work

County: LA | Route/Direction: 210 / East

Closure limits: East 210 to South 2

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	1	N	N	N	N	2	2	2	2	2	2	2	2	2	1	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	N	2	2	2	2	2	2	2	2	2	2	C	C	C	C	
Sat	C	C	C	C	C	C	C	2	2	2	2	2	2	2	2	2	2	2	2	2	1	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	2	2	2	2	2	2	2	2	2	2	C	C	C	C	C	

Legend:

C	Connector may be closed completely
1	Provide at least 1 through connector lane open in direction of travel
2	Provide at least 2 through connector lanes open in direction of travel
N	No work allowed

REMARKS:
When the connector is completely closed detour traffic to continue eastbound on Route 210 and exit at Angeles Crest Hwy off-ramp; north on Angeles Crest Hwy to the on-ramp to westbound Route 210.
Place a portable changeable message sign on the right shoulder of eastbound Route 210 in advance of the Ocean View Blvd off-ramp with the message: "SOUTH 2 / EXIT / CLOSED - USE / ANGELES / CREST".
Post at least 6 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.

Chart no. 21
Complete Connector Closure Hours

County: LA | Route/Direction: 2 / North

Closure limits: North 2 to East 210

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	C	C	C	C	C	C	N	N	N	N	C	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	C	C	C	C	C	C	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C	Connector may be closed completely
N	No work allowed

REMARKS:
Detour traffic to continue northbound on Route 2 and exit at Foothill Blvd off-ramp; east on Foothill Blvd; north on Angeles Crest Hwy to the on-ramp to eastbound Route 210. Place a portable changeable message sign on the right shoulder of northbound Route 2 in advance of the connector with the message: "EAST 210 / EXIT / CLOSED - DETOUR / USE / FOOTHILL".
Post at least 8 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.

Chart no. 22
Connector Lane Requirements and Hours of Work

County: LA | Route/Direction: 2 / North

Closure limits: North 2 to West 210

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	C	2	2	2	2	2	2	2	N	N	N	N	N	N	2	2	1	C	
Fri	C	C	C	C	C	C	C	2	2	2	2	2	2	2	N	N	N	N	N	N	N	N	N	C	
Sat	C	C	C	C	C	C	C	C	C	C	1	2	2	2	2	2	2	2	2	2	2	1	1	C	
Sun	C	C	C	C	C	C	C	C	C	C	1	1	2	2	2	2	2	2	2	2	1	1	C	C	

Legend:

C	Connector may be closed completely
1	Provide at least 1 through connector lane open in direction of travel
2	Provide at least 2 through connector lanes open in direction of travel
N	No work allowed

REMARKS:
Detour traffic to continue northbound on Route 2 and exit at Foothill Blvd off-ramp; north on Foothill Blvd; south on Ocean View Blvd to the on-ramp to westbound Route 210. Place a portable changeable message sign on the median shoulder of northbound Route 2 in advance of the connector with the message: "WEST 210 / EXIT / CLOSED - DETOUR / USE / FOOTHILL".
Post at least 8 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.

Chart no. 23
Complete Connector Closure Hours

County: LA | Route/Direction: 210 / West

Closure limits: West 210 to South 2

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	C	C	C	C	C	C	N	N	N	N	C	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	C	C	C	C	C	C	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C	Connector may be closed completely
N	No work allowed

REMARKS:
Detour traffic to exit at Angeles Crest Hwy off-ramp; south on Angeles Crest Hwy; west on Foothill Blvd to the on-ramp to southbound Route 2. Place a portable changeable message sign on the right shoulder of westbound Route 210 in advance of the Angeles Crest Hwy with the message: "SOUTH 2 / EXIT / CLOSED - USE / ANGELES / CREST".
Post at least 8 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.

Chart no. 24
Connector Lane Requirements and Hours of Work

County: LA | Route/Direction: 210 / East

Closure limits: East 210 to East 210 (Tunnel)

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	1	C	
Fri	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	1	
Sat	C	C	C	C	C	1	1	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	1	
Sun	C	C	C	C	C	1	1	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	1	C	

Legend:

C	Connector may be closed completely
1	Provide at least 1 through connector lane open in direction of travel
N	No work allowed

REMARKS:
When the connector is completely closed detour traffic to exit at Colorado Blvd off-ramp; east on Walnut St; east on Corson St to the on-ramp to eastbound Route 210. Place a portable changeable message sign on the right shoulder of eastbound Route 210 in advance of the closure with the message: "EAST 210 / EXIT / CLOSED - DETOUR / USE / COLORADO".
Post at least 12 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.

Chart no. 25
Complete Connector Closure Hours

County: LA | Route/Direction: 210 / East

Closure limits: East 210 to West 134

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	C	C	C	C	C	C	N	N	N	N	C	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	C	C	C	C	C	C	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C	Connector may be closed completely
N	No work allowed

REMARKS:
Detour traffic to exit at Del Mar Blvd off-ramp; west on Del Mar Blvd; north on Orange Grove Ave; west on Colorado Blvd to the on-ramp to westbound Route 134. Place a portable changeable message sign on the right shoulder of eastbound Route 210 in advance of the closure with the message: "WEST 134 / EXIT / CLOSED - DETOUR / USE / DEL MAR".
Post at least 12 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.

Chart no. 26
Connector Lane Requirements and Hours of Work

County: LA | Route/Direction: 210 / East

Closure limits: East 210 to Colorado Blvd/Del Mar Blvd/California Blvd off ramps

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	N	S	S	S	S	N	N	N	N	N	1	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	N	S	S	S	S	N	N	N	N	N	1	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	1	1	1	S	S	S	S	S	S	1	1	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	1	1	1	1	1	1	1	1	C	C	C	C	C	C	

Legend:

C	Connector may be closed completely
1	Provide at least 1 connector lane open in direction of travel
N	No work allowed

REMARKS:
When the connector is completely closed detour traffic to continue eastbound Route 210 exit at Lake Ave off-ramp; south on Lake Ave; west on Walnut St. Place a portable changeable message sign on the left shoulder of eastbound Route 210 in advance of the closure with the message: " EXIT TO / PASADENA / CLOSED - USE E210 / EXIT / LAKE AVE".
Post at least 12 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.

Chart no. 27
Complete Connector Closure Hours

County: LA | Route/Direction: 134 / East

Closure limits: East 134 to West 210

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	C	C	C	C	C	C	N	N	N	N	C	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	C	C	C	C	C	C	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	N	N	N	N	N	N	N	C	C	C	C	C	

Legend:

C	Connector may be closed completely
N	No work allowed

REMARKS:

Detour (Alternative 1)

Detour traffic onto eastbound Route 210 and exit at Fair Oaks Ave off-ramp; south on Fair Oaks Ave; west on Walnut St to the on-ramp to westbound Route 210. Place a portable changeable message sign on the right shoulder of eastbound Route 134 in advance of the closure with the message: "WEST 210 / EXIT / CLOSED - USE / FAIR / OAKS".

Post at least 8 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.

Detour (Alternative 2)

Detour traffic onto eastbound Route 210 and exit at Fair Oaks Ave off-ramp; north on Fair Oaks Ave; west on Mountain St to the on-ramp to westbound Route 210. Place a portable changeable message sign on the right shoulder of eastbound Route 134 in advance of the closure with the message: "WEST 210 / EXIT / CLOSED - USE / FAIR / OAKS".

Post at least 12 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.

Chart no. 28
Complete Connector Closure Hours

County: LA	Route/Direction: 134 / East	

Closure limits: East 134 to West 210 & South 710

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	N	N	N	C	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	C	C	N	N	N	N	N	N	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	N	N	N	N	N	N	C	C	C	C	C	C	

Legend:

C	Connector may be closed completely
N	No work allowed

REMARKS:
Detour traffic to continue eastbound Route 210 and exit at Lake Ave off-ramp; north on Lake Ave to the on-ramp to westbound Route 210. Place a portable changeable message sign on right shoulder of eastbound Route 134 in advance of Colorado St off-ramp with the message: "210 & 710 / EXITS / CLOSED - DETOUR / USE / LAKE AVE".
Post at least 12 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.

Chart no. 29
Complete Connector Closure Hours

County: LA | Route/Direction: 710 / North

Closure limits: North 710 to East 210

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	N	N	N	C	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C	Connector may be closed completely
N	No work allowed

REMARKS:
Detour California Blvd/ Pasadena Ave traffic onto westbound Route 210 and exit at Mountain St off-ramp; west on Mountain St to the on-ramp to eastbound Route 210. Place the portable changeable message sign inside the connector closure with the message: "EAST 210 / EXIT / CLOSED - DETOUR / W210 TO / MOUNTAIN".
Detour Del Mar Blvd/Pasadena Ave traffic onto westbound Route 134 and exit at San Rafael Ave off-ramp; south on San Rafael Ave to the on-ramp to eastbound Route 134. Place a portable changeable message sign inside the connector closure with the message: "EAST 210 / EXIT / CLOSED - USE W134 / TO SAN / RAFAEL".
Post at least 12 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.

Chart no. 30
Complete Connector Closure Hours

County: LA	Route/Direction: 710 / North	

Closure limits: North 710 to West 134

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	N	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	N	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C	Connector may be closed completely
N	No work allowed

REMARKS:
Close connector from Del Mar Blvd/Pasadena Ave on-ramp. Detour traffic continue north on Pasadena Ave; east on Corson St; north on Fair Oaks Ave the on-ramp to westbound Route 134.
Post at least 16 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.

Chart no. 31
Connector Lane Requirements and Hours of Work

County: LA | Route/Direction: 210 / West

Closure limits: West 210 to West 210

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	1	C	
Fri	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	C	
Sat	C	C	C	C	C	C	1	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	
Sun	C	C	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	N	N	N	N	1	C	

Legend:

C	Connector may be closed completely
1	Provide at least 1 through connector lane open in direction of travel
N	No work allowed

REMARKS:
When the connector is completely closed detour traffic to exit at Fair Oaks Ave off-ramp; north on Fair Oaks Ave; west on Mountain St to the on-ramp to westbound Route 210. Place a portable changeable message sign on the right shoulder of westbound Route 210 in advance of the Fair Oaks off-ramp with the message: "WEST 210 / EXIT / CLOSED - USE / FAIR / OAKS".
Post at least 8 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.

Chart no. 32
Connector Lane Requirements and Hours of Work

County: LA | Route/Direction: 210 / West

Closure limits: West 210 to 110/Del Mar Blvd/California Blvd (Tunnel)

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	1	1	C	C	C	C	C	C	C	C	C	C	C	C	C	C
Fri	C	C	C	C	C	C	N	N	N	1	1	C	C	C	C	C	C	C	C	C	C	C	C	C	C
Sat	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C

Legend:

C	Connector may be closed completely
1	Provide at least 1 through connector lane open in direction of travel
N	No work allowed

REMARKS:
When the connector is completely closed detour traffic onto westbound Route 134 and exit at Orange Grove Blvd off-ramp; south to desired destination. Place a portable changeable message sign on the right shoulder of westbound Route 210 in advance of the closure with the message: "S 110 / EXIT / CLOSED - USE / ORANGE / GROVE".
Post at least 6 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.

Replace "Reserved" in section 12-4.05E with:

Chart no. 33
Complete Ramp Closure Hours

County: LA | Route/Direction: 2 / North

Closure limits: Verdugo Blvd off-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	N	N	N	N	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	N	N	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	C	C	C	C	C	C	

Legend:

C	Ramp may be closed completely
N	No work allowed

REMARKS:

Chart no. 34
Complete Ramp Closure Hours

County: LA | Route/Direction: 2 / South

Closure limits: Verdugo Blvd on-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	N	N	N	N	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	N	N	N	N	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	N	N	N	N	N	N	C	C	C	C	C	C	C	C	C	

Legend:

C Ramp may be closed completely

N No work allowed

REMARKS:
When the ramp is closed, traffic shall be detoured as follows:
(WB) Verdugo Blvd: detour traffic to continue westbound Verdugo Blvd; south on Verdugo Rd; east on Mountain St to the on-ramp to southbound Route 2. Post at least 24 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
(EB) Verdugo blvd: detour traffic to continue eastbound Verdugo Blvd; west on foothill Blvd to the on-ramp to southbound Route 2. Post at least 12 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.

Chart no. 35
Complete Ramp Closure Hours

County: LA | Route/Direction: 2 / North

Closure limits: Foothill Blvd off-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	N	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C Ramp may be closed completely

N No work allowed

REMARKS:
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.

Chart no. 36
Complete Ramp Closure Hours

County: LA | Route/Direction: 2 / South

Closure limits: Foothill Blvd on-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	N	N	N	N	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	N	N	N	N	C	C	C	C	C	C	C	

Legend:

C Ramp may be closed completely

N No work allowed

REMARKS:
When the ramp is closed, traffic shall be detoured as follows:
(WB) Foothill Blvd: detour traffic to continue westbound Foothill Blvd; south on Ocean View Blvd; east on Montrose Ave; east on Verdugo Blvd to the on-ramp to southbound Route 2. Post at least 14 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
(EB) Foothill Blvd: detour traffic to continue eastbound Foothill Blvd; west on Verdugo Blvd to the on-ramp to southbound Route 2. Post at least 12 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.

Chart no. 37
Complete Ramp Closure Hours

County: LA | Route/Direction: 210 / East

Closure limits: Lowell Ave / Honolulu Ave on-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	C	C	
Fri	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	C	C	
Sat	C	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	C	
Sun	C	C	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	N	N	N	N	C	C	

Legend:

C Ramp may be closed completely

N No work allowed

REMARKS:
When the ramp is closed, traffic shall be detoured as follows:
(SB) Lowell Ave/ Honolulu Ave: detour traffic to south Honolulu Ave; north on Pennsylvania Ave to the on-ramp to eastbound Route 210. Post at least 12 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
(NB) Honolulu Ave: detour traffic to continue northbound Honolulu Ave; west on La Tuna Canyon Rd to the on-ramp to eastbound Route 210. Post at least 10 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Chart no. 38
Complete Ramp Closure Hours

County: LA | Route/Direction: 210 / West

Closure limits: Lowell Ave / Tujunga off-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	C	
Fri	C	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	
Sat	C	C	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	
Sun	C	C	C	C	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	N	N	N	C	

Legend:

C Ramp may be closed completely

N No work allowed

REMARKS:
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Chart no. 39
Complete Ramp Closure Hours

County: LA | Route/Direction: 210 / East

Closure limits: Pennsylvania Ave / La Crescenta off-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	C	C	C	C	C	C	N	N	N	N	C	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	C	C	C	C	C	C	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C Ramp may be closed completely

N No work allowed

REMARKS:
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Chart no. 40
Complete Ramp Closure Hours

County: LA | Route/Direction: 210 / East

Closure limits: Pennsylvania Ave on-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	N	N	N	C	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	N	N	N	N	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	N	N	N	N	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C Ramp may be closed completely

N No work allowed

REMARKS:
When the ramp is closed, traffic shall be detoured as follows:
(NB) Pennsylvania Ave: detour traffic to continue northbound Pennsylvania Ave; east on Foothill Blvd; south on La Crescenta Ave to the on-ramp to eastbound Route 210. Post at least 4 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
(SB) Pennsylvania Ave: detour traffic to continue southbound Pennsylvania Ave; east on Montrose Ave; north on La Crescenta Ave to the on-ramp to eastbound Route 210. Post at least 10 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Chart no. 41
Complete Ramp Closure Hours

County: LA | Route/Direction: 210 / West

Closure limits: Pennsylvania Ave off-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	C	C	
Fri	C	C	C	C	C	C	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C	Ramp may be closed completely
N	No work allowed

REMARKS:
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Chart no. 42
Complete Ramp Closure Hours

County: LA | Route/Direction: 210 / West

Closure limits: Pennsylvania Ave on-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	C	C	C	C	C	C	N	N	N	N	C	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	C	C	C	C	C	C	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C	Ramp may be closed completely
N	No work allowed

REMARKS:
When the ramp is closed, traffic shall be detoured as follows:
(NB) Pennsylvania Ave: detour traffic to continue northbound Pennsylvania Ave; west on Foothill Blvd; south on Lowell Ave; west on Honolulu Ave to the on-ramp to westbound Route 210. Post at least 14 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
(SB) Pennsylvania Ave: detour traffic to continue southbound Pennsylvania Ave; west on Honolulu Ave to the on-ramp to westbound Route 210. Post at least 6 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Chart no. 43
Complete Ramp Closure Hours

County: LA | Route/Direction: 210 / East

Closure limits: La Crescenta Ave on-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	N	N	N	N	C	C	C	C	C	C	C	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	N	N	N	N	C	C	C	C	C	C	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	N	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	C	C	C	C	C	

Legend:

C	Ramp may be closed completely
N	No work allowed

REMARKS:
When the ramp is closed, traffic shall be detoured as follows:
(NB) La Crescenta Ave: detour traffic to continue northbound La Crescenta Ave; east on Foothill Blvd; south on Ocean View Blvd to the on-ramp to eastbound Route 210. Post at least 6 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
(SB) La Crescenta Ave: detour traffic to continue southbound La Crescenta Ave; east on Montrose Ave; north on Ocean View Blvd to the on-ramp to eastbound Route 210. Post at least 10 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Chart no. 44
Complete Ramp Closure Hours

County: LA | Route/Direction: 210 / West

Closure limits: La Crescenta Ave off-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	C	C	
Fri	C	C	C	C	C	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	N	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	N	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	N	N	N	N	N	N	N	C	C	C	C	C	

Legend:

C	Ramp may be closed completely
N	No work allowed

REMARKS:
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Chart no. 45
Complete Ramp Closure Hours

County: LA | Route/Direction: 210 / East

Closure limits: Ocean View Blvd / Montrose Ave off-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	C	C	C	C	C	C	N	N	N	N	C	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	C	C	C	C	C	C	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C	Ramp may be closed completely
N	No work allowed

REMARKS:
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Chart no. 46
Complete Ramp Closure Hours

County: LA	Route/Direction: 210 / East	

Closure limits: Ocean View Blvd on-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	C	C	C	C	C	C	N	N	N	N	C	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	C	C	C	C	C	C	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C	Ramp may be closed completely
N	No work allowed

REMARKS:
When the ramp is closed, traffic shall be detoured as follows:
(NB) Ocean View Blvd: detour traffic to continue northbound Ocean View Blvd; east on Foothill Blvd; north on Angeles Crest Hwy to the on-ramp to eastbound Route 210. Post at least 10 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
(SB) Ocean View Blvd: detour traffic to continue southbound Ocean View Blvd; east on Montrose Ave; east on Verdugo Blvd; east on Foothill Blvd; north on Angeles Crest Hwy to the on-ramp to eastbound Route 210. Post at least 18 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Chart no. 47
Complete Ramp Closure Hours

County: LA | Route/Direction: 210 / West

Closure limits: Ocean View Blvd / Montrose Ave off-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	C	C	C	C	C	C	N	N	N	N	C	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C Ramp may be closed completely

N No work allowed

REMARKS:
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Chart no. 48
Complete Ramp Closure Hours

County: LA | Route/Direction: 210 / West

Closure limits: Ocean View Blvd on-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	C	C	C	C	C	C	N	N	N	N	C	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	C	C	C	C	C	C	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C Ramp may be closed completely

N No work allowed

REMARKS:
When the ramp is closed, traffic shall be detoured as follows:
(NB) Ocean View Blvd: detour traffic to continue northbound Ocean View Blvd; west on Foothill Blvd; south on Pennsylvania Ave to the on-ramp to westbound Route 210. Post at least 10 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
(SB) Ocean View Blvd: detour traffic to continue southbound Ocean View Blvd; east on Montrose Ave; north on Pennsylvania Ave to the on-ramp to westbound Route 210. Post at least 6 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Chart no. 49
Complete Ramp Closure Hours

County: LA | Route/Direction: 210 / East

Closure limits: Angeles Crest Hwy / La Canada Flintridge off-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	C	C	C	N	N	N	N	N	N	N	N	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	C	C	C	N	N	N	N	N	N	N	N	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C	Ramp may be closed completely
N	No work allowed

REMARKS:
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Chart no. 50
Complete Ramp Closure Hours

County: LA | Route/Direction: 210 / East

Closure limits: Angeles Crest Hwy on ramps

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	C	C	C	C	C	C	N	N	N	N	C	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	C	C	C	C	C	C	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C	Ramp may be closed completely
N	No work allowed

REMARKS:
When the ramp is closed, traffic shall be detoured as follows:
(NB) Angeles Crest Hwy: detour traffic to the on-ramp to westbound Route 210; onto westbound Route 210 and exit at Ocean View Blvd; south on Ocean View Blvd to the on-ramp to eastbound Route 210. Post at least 10 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
(SB) Angeles Crest Hwy: detour traffic to continue southbound Angeles Crest Hwy; east on Foothill Blvd to the on-ramp to eastbound Route 210. Post at least 6 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Chart no. 51
Complete Ramp Closure Hours

County: LA | Route/Direction: 210 / West

Closure limits: Angeles Crest Hwy / La Canada Flintridge off-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	N	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C	Ramp may be closed completely
N	No work allowed

REMARKS:
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Chart no. 52
Complete Ramp Closure Hours

County: LA | Route/Direction: 210 / West

Closure limits: Angeles Crest Hwy on ramps

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	C	C	C	C	C	C	N	N	N	N	C	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	C	C	C	C	C	C	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C	Ramp may be closed completely
N	No work allowed

REMARKS:
When the ramp is closed, traffic shall be detoured as follows:
(WB) Foothill Blvd: detour traffic to continue westbound Foothill Blvd; west on Foothill Blvd; south on Ocean View Blvd to the on-ramp to westbound Route 210. Post at least 8 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
(NB) Angeles Crest Hwy: detour traffic to the on-ramp to eastbound Route 210; onto eastbound Route 210 and exit at Gould Ave; north on Gould Ave to the on-ramp to westbound Route 210. Post at least 8 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
(SB) Angeles Crest Hwy: detour traffic to continue southbound Angeles Crest Hwy; west on Foothill Blvd; south on Ocean View Blvd to the on-ramp to westbound Route 210. Post at least 8 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Chart no. 53
Complete Ramp Closure Hours

County: LA | Route/Direction: 210 / East

Closure limits: Gould Ave off-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C	Ramp may be closed completely
N	No work allowed

REMARKS:
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Chart no. 54
Complete Ramp Closure Hours

County: LA	Route/Direction: 210 / West	

Closure limits: Gould Ave on-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C	Ramp may be closed completely
N	No work allowed

REMARKS:
When the ramp is closed, traffic shall be detoured as follows:
(EB) Foothill Blvd: detour traffic to continue eastbound Foothill Blvd to the on-ramp to eastbound Route 210, onto eastbound Route 210 and exit at Berkshire Ave; east on Berkshire Pl to the on-ramp to westbound Route 210. Post at least 6 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
(WB) Foothill Blvd: detour traffic to continue westbound Foothill Blvd; north on Angeles Crest Hwy to the on-ramp to westbound Route 210. Post at least 4 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
(SB) Gould Ave: detour traffic to continue southbound Gould Ave; west on Foothill Blvd; north on Angeles Crest Hwy to the on-ramp to westbound Route 210. Post at least 4 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Chart no. 55
Complete Ramp Closure Hours

County: LA | Route/Direction: 210 / East

Closure limits: Foothill Blvd on-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	C	C	C	N	N	N	N	N	N	N	C	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	C	C	C	N	N	N	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	C	N	N	N	N	N	N	C	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C	Ramp may be closed completely
N	No work allowed

REMARKS:
When the ramp is closed, traffic shall be detoured as follows:
(WB) Foothill Blvd: detour traffic to continue westbound Foothill Blvd; north on Angeles Crest Hwy to the on-ramp to westbound Route 210. Post at least 8 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
(EB) Foothill Blvd: detour traffic to continue eastbound foothill Blvd; south on Oak Grove Dr; west on Berkshire Pl to the on-ramp to eastbound Route 210. Post at least 6 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Chart no. 56
Complete Ramp Closure Hours

County: LA | Route/Direction: 210 / West

Closure limits: Foothill Blvd off-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	N	N	N	C	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	C	N	N	N	N	C	C	C	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C	Ramp may be closed completely
N	No work allowed

REMARKS:
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Chart no. 57
Complete Ramp Closure Hours

County: LA | Route/Direction: 210 / East

Closure limits: Berkshire Ave off-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C	Ramp may be closed completely
N	No work allowed

REMARKS:
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Chart no. 58
Complete Ramp Closure Hours

County: LA	Route/Direction: 210 / East	

Closure limits: Berkshire Ave on-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C	Ramp may be closed completely
N	No work allowed

REMARKS:
When the ramp is closed, traffic shall be detoured as follows:
(EB) Berkshire Pl: detour traffic to continue eastbound Berkshire Pl; south on Oak Grove Dr; south on Windsor Ave to the on-ramp to eastbound Route 210. Post at least 8 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
(NB) Oak Grove Dr: detour traffic to continue northbound Oak Grove Dr; west on Foothill Blvd to the on-ramp to eastbound Route 210. Post at least 10 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
(SB) Oak Grove Dr: detour traffic to continue southbound Oak Grove Dr; south on Windsor Ave to the on-ramp to eastbound Route 210. Post at least 8 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Chart no. 59
Complete Ramp Closure Hours

County: LA | Route/Direction: 210 / West

Closure limits: Berkshire Ave off-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C	Ramp may be closed completely
N	No work allowed

REMARKS:
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Chart no. 60
Complete Ramp Closure Hours

County: LA | Route/Direction: 210 / West

Closure limits: Berkshire Ave on-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C	Ramp may be closed completely
N	No work allowed

REMARKS:
When the ramp is closed, traffic shall be detoured as follows:
(EB) Berkshire Pl and (WB) Oak Grove Dr: detour traffic to continue westbound Oak Grove Dr; west on Foothill Blvd; north on Gould Ave to the on-ramp to westbound Route 210. Post at least 6 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
(EB) Oak Grove Dr: detour traffic to continue eastbound Oak Grove Dr; south on Windsor Ave/Arroyo Blvd to the on-ramp to westbound Route 210. Post at least 8 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Chart no. 61
Complete Ramp Closure Hours

County: LA | Route/Direction: 210 / East

Closure limits: Arroyo Blvd off-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	N	N	N	N	C	C	C	C	C	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C Ramp may be closed completely

N No work allowed

REMARKS:
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Chart no. 62
Complete Ramp Closure Hours

County: LA | Route/Direction: 210 / East

Closure limits: Arroyo Blvd on-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C Ramp may be closed completely

N No work allowed

REMARKS:
When the ramp is closed, traffic shall be detoured as follows:
Woodbury Rd: detour traffic to continue westbound Oak Grove Dr; west on Berkshire Pl to the on-ramp to eastbound Route 210. Post at least 12 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
Oak Grove Dr, Windsor Ave, Arroyo Blvd: detour traffic east on Woodbury Rd; south on Lincoln Ave; west on Howard St to the on-ramp to eastbound Route 210. Post at least 12 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Chart no. 63
Complete Ramp Closure Hours

County: LA | Route/Direction: 210 / West

Closure limits: Arroyo Blvd off-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C	Ramp may be closed completely
N	No work allowed

REMARKS:
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Chart no. 64
Complete Ramp Closure Hours

County: LA | Route/Direction: 210 / West

Closure limits: Arroyo Blvd on-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C	Ramp may be closed completely
N	No work allowed

REMARKS:
When the ramp is closed, traffic shall be detoured as follows:
(WB) Woodbury Rd, Windsor Ave, Arroyo Blvd: detour traffic west on Oak Grove Dr; west on Berkshire Pl to the on-ramp to westbound Route 210. Post at least 14 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
(EB) Oak Grove Dr: detour traffic eastbound on Woodbury Rd; south on Lincoln Ave; west on Howard St to the on-ramp to westbound Route 210. Post at least 12 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Chart no. 65
Complete Ramp Closure Hours

County: LA | Route/Direction: 210 / East

Closure limits: Lincoln Ave off-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C	Ramp may be closed completely
N	No work allowed

REMARKS:
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Chart no. 66
Complete Ramp Closure Hours

County: LA | Route/Direction: 210 / East

Closure limits: Lincoln Ave on-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	N	N	N	N	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	N	N	N	N	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	N	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	N	N	N	N	N	N	N	N	C	C	C	C	C	C	

Legend:

C	Ramp may be closed completely
N	No work allowed

REMARKS:
When the ramp is closed, traffic shall be detoured as follows:
(WB) NB Lincoln Ave and WB Howard St: detour traffic to continue northbound Lincoln Ave; west on Woodbury Rd; south Arroyo Blvd to the on-ramp to eastbound Route 210. Post at least 12 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
(EB) SB Lincoln Ave and EB Howard St: detour traffic to continue southbound Lincoln Ave; south on Forest Ave; east on Mountain St to the on-ramp to eastbound Route 210. Post at least 16 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Detour for extended weekend closure:
(WB) NB Lincoln Ave: detour traffic to continue northbound Lincoln Ave; east on Woodbury Rd; south Fair Oaks Ave; east on Corson St to the on-ramp to eastbound Route 210. Post at least 12 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
(EB) SB Lincoln Ave and Howard St: detour traffic to continue southbound Lincoln Ave; east on Washington Blvd; south Fair Oaks Ave; east on Corson St to the on-ramp to eastbound Route 210. Post at least 16 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.

Chart no. 67
Complete Ramp Closure Hours

County: LA | Route/Direction: 210 / West

Closure limits: Lincoln Ave off ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	N	N	N	N	N	N	C	C	C	
Sat	C	C	C	C	C	C	C	C	N	N	N	N	N	N	N	N	N	C	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	N	N	N	C	C	C	C	C	C	C	C	C	

Legend:

C Ramp may be closed completely

N No work allowed

REMARKS:
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Chart no. 68
Complete Ramp Closure Hours

County: LA | Route/Direction: 210 / West

Closure limits: Lincoln Ave on-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C Ramp may be closed completely

N No work allowed

When the ramp is closed, traffic shall be detoured as follows:
(NB) Lincoln Ave: detour traffic to continue northbound Lincoln Ave; west on Woodbury Rd; south on Windsor Ave to the on-ramp to westbound Route 210. Post at least 16 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
(SB) Lincoln Ave: detour traffic to continue southbound Lincoln Ave; east on Washington Blvd; north on Fair Oak Ave, west on Woodbury Rd; south on Windsor Ave to the on-ramp to westbound Route 210. Post at least 20 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Chart no. 69
Complete Ramp Closure Hours

County: LA	Route/Direction: 210 / East	

Closure limits: Mountain St off-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C	Ramp may be closed completely
N	No work allowed

REMARKS:
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Chart no. 70
Complete Ramp Closure Hours

County: LA | Route/Direction: 210 / East

Closure limits: Mountain St on-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C	Ramp may be closed completely
N	No work allowed

REMARKS:
When the ramp is closed, traffic shall be detoured as follows:
(WB) Mountain St: detour traffic to continue westbound Mountain St; north on Forest Ave; north on Lincoln Ave; west on Howard St to the on-ramp to eastbound Route 210. Post at least 8 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
(EB) Mountain St: detour traffic to continue eastbound Mountain St; south on Fair Oaks Ave; east on Corson St to the on-ramp to eastbound Route 210. Post at least 8 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Detour for extended weekend closure:
(WB) Mountain St: detour traffic to continue westbound Mountain St; north on Forest Ave; north on Lincoln Ave; east on Washington Blvd; south on Fair Oaks Ave; east on Corson St to the on-ramp to eastbound Route 210. Post at least 12 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
(EB) Mountain St: detour traffic to continue eastbound Mountain St; south on Fair Oaks Ave; east on Corson St to the on-ramp to eastbound Route 210. Post at least 8 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.

Chart no. 71
Complete Ramp Closure Hours

County: LA | Route/Direction: 210 / West

Closure limits: Mountain St off-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C	Ramp may be closed completely
N	No work allowed

REMARKS:
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Chart no. 72
Complete Ramp Closure Hours

County: LA | Route/Direction: 210 / West

Closure limits: Mountain St on-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C	Ramp may be closed completely
N	No work allowed

REMARKS:
When the ramp is closed, traffic shall be detoured as follows:
(WB) Mountain St: detour traffic to continue westbound Mountain St; north on Forest Ave; north on Lincoln Ave to the on-ramp to westbound Route 210. Post at least 14 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
(EB) Mountain St: detour traffic to continue eastbound Mountain St; north on Fair Oaks Ave; west on Washington Blvd; north on Lincoln Ave to the on-ramp to westbound Route 210. Post at least 24 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Chart no. 73
Complete Ramp Closure Hours

County: LA | Route/Direction: 210 / East

Closure limits: Colorado Blvd off-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C	Ramp may be closed completely
N	No work allowed

REMARKS:
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Chart no. 74
Complete Ramp Closure Hours

County: LA | Route/Direction: 210 / West

Closure limits: Walnut St on-ramp

Hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon-Thu	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	N	C	C	C	C	
Fri	C	C	C	C	C	C	N	N	N	C	C	C	C	C	N	N	N	N	N	C	C	C	C	C	
Sat	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	
Sun	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	

Legend:

C	Ramp may be closed completely
N	No work allowed

REMARKS:
When the ramp is closed, traffic shall be detoured as follows:
(WB) Walnut St: detour traffic to continue westbound Walnut St; north on Orange Grove Blvd; north on Fair Oaks; west on Mountain St to the on-ramp to westbound Route 210. Post at least 4 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
(EB) Walnut St: detour traffic to continue eastbound Walnut St; north on Fair Oaks Ave; west on Mountain St to the on-ramp to westbound Route 210. Post at least 4 special portable freeway detour signs, SP-2, along the detour route and remove signs at the end of each closure.
The full width of the traveled way must be open for use by traffic when construction activities are not actively in progress.
No ramp closures will be permitted if they conflict with the continuous ramp closure detours.

Replace section 12-5 with:

12-5 TRAFFIC CONTROL SYSTEM FOR LANE CLOSURE

12-5.01 GENERAL

Section 12-5 includes specifications for closing traffic lanes, ramps, or a combination, with stationary and moving lane closures on multilane highways and 2-lane, 2-way highways. The traffic control system for a lane closure or a ramp closure must comply with the details shown.

Traffic control system includes signs.

12-5.02 MATERIALS

Vehicles equipped with attenuators must comply with section 12-3.13 of the special provisions.

12-5.03 CONSTRUCTION

12-5.03A General

During traffic striping and pavement marker placement using bituminous adhesive, control traffic with a stationary or a moving lane closure. During other activities, control traffic with stationary lane closures.

Whenever components of the traffic control system are displaced or cease to operate or function as specified from any cause, immediately repair the components to the original condition or replace the components and restore the components to the original location.

12-5.03B Stationary Lane Closures

For a stationary lane closure, ramp closure, or a combination, made only for the work period, remove the components of the traffic control system from the traveled way and shoulder, except for portable delineators placed along open trenches or excavation adjacent to the traveled way at the end of each work period. You may store the components at selected central locations designated by the Engineer within the limits of the highway.

Each vehicle used to place, maintain, and remove components of a traffic control system on a multilane highway must be equipped with a Type II flashing arrow sign that must be in operation whenever the vehicle is being used for placing, maintaining, or removing the components. Vehicles equipped with a Type II flashing arrow sign not involved in placing, maintaining, or removing the components if operated within a stationary-type lane closure must display only the caution display mode. The sign must be controllable by the operator of the vehicle while the vehicle is in motion. If a flashing arrow sign is required for a lane closure, the flashing arrow sign must be operational before the lane closure is in place.

For multilane freeway or expressway lane closures, do not place the 2L tangent section shown along lane lines between the lane closure tapers.

For multilane freeways and expressways, do not place the traffic cones shown to be placed transversely across closed traffic lanes and shoulders.

12-5.03C Moving Lane Closures

A changeable message sign used in a moving lane closure must comply with section 12-3.12 except the sign must be truck-mounted. The full operational height to the bottom of the sign may be less than 7 feet above the ground but must be as high as practicable.

A flashing arrow sign used in a moving lane closure must be truck-mounted. Operate the flashing arrow sign in the caution display mode whenever it is being used on a 2-lane, 2-way highway.

12-5.04 PAYMENT

Traffic control system for lane closure is paid for as traffic control system.

The requirements in section 4-1.05 for payment adjustment do not apply to traffic control system. Adjustments in compensation for traffic control system will be made for an increase or decrease in traffic control work if ordered and will be made on the basis of the cost of the necessary increased or decreased traffic control. The adjustment will be made on a force account basis for increased work and estimated on the same basis in the case of decreased work.

A traffic control system required by change order work is paid for as a part of the change order work.

12-5.04A Incentives and Disincentives

Incentive payments and disincentive deductions apply to the completion of the work specified in the Incentive / Disincentive table.

Comply with "Maintaining Traffic" and "Closure Requirements and Conditions".

Incentive payments and disincentive deductions are independent of liquidated damages and damages specified in "Closure Requirements and Conditions".

Complete the work specified and open all the lanes to public traffic within the time specified in the Incentive / Disincentive table starting on the day specified. If you complete the work and open all the lanes to public traffic within the specified time, you will receive the incentive shown for each day less than the time specified. If you do not complete the work and open all the lanes to public traffic within the specified time, the Department will deduct the disincentive shown for each day needed to complete the work and opening all the lanes to public traffic.

Location	Days	Incentive amount	Disincentive amount
EB Route 210 Pennsylvania Ave off-ramp	15	$1,000 per day $15,000 Maximum	$1,000 per day
EB Route 210 Pennsylvania Ave on-ramp	15	$1,000 per day $15,000 Maximum	$1,000 per day
WB Route 210 Pennsylvania Ave off-ramp	15	$1,000 per day $15,000 Maximum	$1,000 per day
WB Route 210 Pennsylvania Ave on-ramp	15	$1,000 per day $15,000 Maximum	$1,000 per day
EB Route 210 La Crescenta Ave on-ramp	15	$1,500 per day $22,500 Maximum	$1,500 per day
WB Route 210 La Crescenta Ave off-ramp	15	$2,000 per day $30,000 Maximum	$2,000 per day
EB Route 210 Ocean View Blvd on-ramp	15	$1,500 per day $22,500 Maximum	$1,500 per day
WB Route 210 Ocean View Blvd off-ramp	15	$2,000 per day $30,000 Maximum	$2,000 per day
WB Route 210 Ocean View Blvd on-ramp	15	$1,500 per day $22,500 Maximum	$1,500 per day
EB Route 210 (SB) Los Angeles Crest Hwy on-ramp	15	$500 per day $7,500 Maximum	$500 per day
EB Route 210 (NB) Los Angeles Crest Hwy on-ramp	15	$1000 per day $15,000 Maximum	$1000 per day
WB Route 210 Los Angeles Crest Hwy off-ramp	15	$1,000 per day $15,000 Maximum	$1,000 per day
WB Route 210 (NB) Los Angeles Crest Hwy on-ramp	15	$500 per day $7,500 Maximum	$500 per day
WB Route 210 Gould Ave off-ramp	15	$1,000 per day $15,000 Maximum	$1,000 per day
EB Route 210 Foothill Blvd on-ramp	15	$1,000 per day $15,000 Maximum	$1,000 per day
WB Route 210 Foothill Blvd off-ramp	15	$2,000 per day $30,000 Maximum	$2,000 per day
EB Route 210 Berkshire Ave on-ramp	15	$500 per day $7,500 Maximum	$500 per day
WB Route 210 Berkshire Ave off-ramp	15	$500 per day $7,500 Maximum	$500 per day

WB Route 210 Berkshire Ave on-ramp	15	$1,000 per day $15,000 Maximum	$1,000 per day
EB Route 210 Arroyo Blvd Ave off-ramp	15	$1,500 per day $22,500 Maximum	$1,500 per day
EB Route 210 Arroyo Blvd Ave on-ramp	15	$1,000 per day $15,000 Maximum	$1,000 per day
WB Route 210 Arroyo Blvd Ave off-ramp	15	$1,000 per day $15,000 Maximum	$1,000 per day
EB Route 210 Lincoln Ave off-ramp	15	$500 per day $7,500 Maximum	$500 per day
EB Route 210 Lincoln Ave on-ramp	15	$1,000 per day $15,000 Maximum	$1,000 per day
WB Route 210 Lincoln Ave off-ramp	15	$2,000 per day $30,000 Maximum	$2,000 per day
WB Route 210 Lincoln Ave on-ramp	15	$500 per day $7,500 Maximum	$500 per day
EB Route 210 Mountain St off-ramp	15	$1,000 per day $15,000 Maximum	$1,000 per day
EB Route 210 Mountain St on-ramp	15	$1,000 per day $15,000 Maximum	$1,000 per day
WB Route 210 Mountain St on-ramp	15	$1,000 per day $15,000 Maximum	$1,000 per day

Total incentive payment will not exceed $487,500.

Total disincentive deduction will not exceed $487,500.

The time limit specified for the completion of the work is considered insufficient to permit completion of the work by working a normal number of hours per day or week on a single shift basis. Should you fail to maintain the progress of the work in conformance with "Progress Schedule (Critical Path Method)" , additional shifts will be required to the extent necessary to ensure that the progress conforms to the above mentioned schedule and that the work will be completed within the time limit specified.

Actions required by the Engineer to perform normal inspection and testing duties will not be considered as contributing to any delay in awarding incentives or to any delay that will require charging disincentives.

Department does not pay for costs incurred by compliance with this section.

Replace section 12-8 with:

12-8 TEMPORARY PAVEMENT DELINEATION

12-8.01 GENERAL

Section 12-8 includes specifications for placing, applying, maintaining, and removing temporary pavement delineation.

Temporary signing for no-passing zones must comply with section 12-3.06.

Temporary painted traffic stripes and painted pavement markings used for temporary delineation must comply with section 84-3.

12-8.02 MATERIALS

12-8.02A General

Not Used

12-8.02B Temporary Lane Line and Centerline Delineation

Temporary pavement markers must be the same color as the lane line or centerline markers being replaced. Temporary pavement markers must be temporary pavement markers on the Authorized Material List for short-term day/night use, 14 days or less, or long-term day/night use, 180 days or less. Place temporary pavement markers under the manufacturer's instructions.

12-8.02C Temporary Edge Line Delineation

On multilane roadways, freeways, and expressways open to traffic where edge lines are obliterated and temporary pavement delineation to replace those edge lines is not shown, provide temporary pavement delineation for:

1. Right edge lines consisting of (1) a solid 4-inch wide traffic stripe tape of the same color as the stripe being replaced, (2) traffic cones, or (3) portable delineators or channelizers placed longitudinally at intervals not exceeding 100 feet
2. Left edge lines consisting of (1) solid 4-inch wide traffic stripe tape of the same color as the stripe being replaced, (2) traffic cones, (3) portable delineators or channelizers placed longitudinally at intervals not exceeding 100 feet, or (4) temporary pavement markers placed longitudinally at intervals not exceeding 6 feet

12-8.02D Temporary Traffic Stripe Tape

Temporary traffic stripe tape must be one of the types of temporary, removable striping tape on the Authorized Material List.

12-8.02E Temporary Traffic Stripe Paint

Not Used

12-8.02F Temporary Pavement Marking Tape

Temporary pavement marking tape must be one of the types of temporary, removable pavement marking tape on the Authorized Material List and must be applied and removed as specified for applying and removing temporary, removable traffic stripe tape.

12-8.02G Temporary Pavement Marking Paint

You may use one of the types of temporary removable pavement marking tape or permanent pavement marking tape on the Authorized Material List instead of temporary pavement marking paint.

12- 8.02H Temporary Pavement Markers

Temporary pavement markers must be one of the temporary pavement markers on the Authorized Material List for long term day/night use, 180 days or less.

12-8.03 CONSTRUCTION

12-8.03A General

Wherever work activities obliterate pavement delineation, place temporary or permanent pavement delineation before opening the traveled way to traffic. Place lane line and centerline pavement delineation for traveled ways open to traffic. On multilane roadways, freeways and expressways, place edge line delineation for traveled ways open to traffic.

Establish the alignment for the temporary pavement delineation including required lines or markers. Surfaces to receive an application of paint or removable traffic tape must be dry and free of dirt and loose material. Do not apply temporary pavement delineation over existing pavement delineation or other temporary pavement delineation. Maintain temporary pavement delineation until it is superseded or you replace it with a new pattern of temporary pavement delineation or permanent pavement delineation.

When the Engineer determines the temporary pavement delineation is no longer required for the direction of traffic, remove the temporary pavement markers, underlying adhesive, and removable traffic tape from the final layer of surfacing and from the existing pavement to remain in place. Remove temporary pavement delineation that conflicts with any subsequent or new traffic pattern for the area.

12-8.03B Temporary Lane line and Centerline Delineation

Whenever lane lines or centerlines are obliterated and temporary pavement delineation to replace the lines is not shown, the minimum lane line and centerline delineation must consist of temporary pavement markers placed longitudinally at intervals not exceeding 24 feet. For temporary pavement markers on the Authorized Material List for long-term day/night use, 180 days or less, cement the markers to the surfacing with the adhesive recommended by the manufacturer except do not use epoxy adhesive to place the pavement markers in areas where removal of the markers will be required.

For temporary lane line or centerline delineation consisting entirely of temporary pavement markers on the Authorized Material List for short-term day/night use, 14 days or less, place the markers longitudinally at intervals not exceeding 24 feet. Do not use the markers for more than 14 days on lanes opened to traffic. Place the permanent pavement delineation before the end of the 14 days. If the permanent pavement delineation is not placed within the 14 days, replace the temporary pavement markers with additional temporary pavement delineation equivalent to the pattern specified or shown for the permanent pavement delineation for the area. The Department does not pay for the additional temporary pavement delineation.

Where no-passing centerline pavement delineation is obliterated, install the following temporary no-passing zone signs before opening lanes to traffic. Install a W20-1, "Road Work Ahead," sign from 1,000 feet to 2,000 feet in advance of a no-passing zone. Install a R4-1, "Do Not Pass," sign at the beginning of a no-passing zone and at 2,000-foot intervals within the no-passing zone. For continuous zones longer than 2 miles, install a W7-3a or W71(CA), "Next ___ Miles," sign beneath the W20-1 sign. Install a R4-2, "Pass With Care," sign at the end of the no-passing zone. The Engineer determines the exact location of temporary no-passing zone signs. Maintain the temporary no-passing zone signs in place until you place the permanent no-passing centerline pavement delineation. Remove the temporary no-passing zone signs when the Engineer determines they are no longer required for the direction of traffic.

12-8.03C Temporary Edge Line Delineation

You may apply temporary painted traffic stripe where removal of a 4-inch wide traffic stripe is not required.

The Engineer determines the lateral offset for traffic cones, portable delineators, and channelizers used for temporary edge line delineation. If traffic cones or portable delineators are used for temporary pavement delineation for edge lines, maintain the cones or delineators during hours of the day when the cones or delineators are being used for temporary edge line delineation.

Channelizers used for temporary edge line delineation must be an orange surface-mounted type. Cement channelizer bases to the pavement under section 85 for cementing pavement markers to pavement except do not use epoxy adhesive to place channelizers on the top layer of the pavement. Channelizers must be one of the 36-inch, surface-mounted types on the Authorized Material List.

Remove the temporary edge line delineation when the Engineer determines it is no longer required for the direction of traffic.

12-8.03D Temporary Traffic Stripe Tape

Apply temporary traffic stripe tape under the manufacturer's instructions. Slowly roll the tape with a rubber-tired vehicle or roller to ensure complete contact with the pavement surface. Apply the tape straight on a tangent alignment and on a true arc on a curved alignment. Do not apply the tape when the air or pavement temperature is less than 50 degrees F unless the installation procedures are authorized beforehand.

The temporary traffic stripe tape must be complete in place at the location shown before opening the traveled way to traffic.

12-8.03E Temporary Traffic Stripe Paint

Apply 1 or 2 coats of temporary traffic stripe paint for new or existing pavement.

The painted temporary traffic stripe must be complete in place at the location shown before opening the traveled way to traffic. Removal of painted temporary traffic stripe is not required.

12-8.03F Temporary Pavement Marking Tape

Apply temporary pavement marking tape at the locations shown. The tape must be complete in place at the location shown before opening the traveled way to traffic.

12-8.03G Temporary Pavement Marking Paint

Apply and maintain temporary pavement markings consisting of painted pavement markings at the locations shown. The painted temporary pavement marking must be complete in place at the location shown before opening the traveled way to traffic. Removal of painted temporary pavement marking is not required.

Apply 1 or 2 coats of temporary pavement marking paint for new or existing pavement.

12- 8.03H Temporary Pavement Markers

Place temporary pavement markers under the manufacturer's instructions. Cement the markers to the surfacing with the manufacturer's recommended adhesive, except do not use epoxy adhesive in areas where removal of the pavement markers is required.

You may use retroreflective pavement markers specified in section 85 instead of temporary pavement markers for long term day/night use, 180 days or less, except to simulate patterns of broken traffic stripe. Retroreflective pavement markers used for temporary pavement markers must comply with section 85, except the waiting period before placing pavement markers on new HMA surfacing as specified in section 85-1.03 does not apply. Do not use epoxy adhesive to place pavement markers in areas where removal of the pavement markers is required.

Temporary pavement markers must be complete in place before opening the traveled way to traffic.

12-8.04 PAYMENT

Not Used

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13 WATER POLLUTION CONTROL

Add to section 13-3.01A:

The project is risk level 2.

^^

14 ENVIRONMENTAL STEWARDSHIP

Replace section 14-11.03 with:

14-11.03 MATERIAL CONTAINING HAZARDOUS WASTE CONCENTRATIONS OF AERIALLY DEPOSITED LEAD

14-11.03A General

14-11.03A(1) Summary

Section 14-11.03 includes specifications for hazardous waste management while excavating, stockpiling, transporting, placing, and disposing of material containing hazardous waste concentrations of aerially deposited lead (ADL).

ADL is present within the project limits.

14-11.03A(2) Definitions

Type Y-1: Material that contains ADL in average concentrations (using the 90 percent Upper Confidence Limit) of 1.5 mg/L or less extractable lead (based on a modified waste extraction test using deionized water as the extractant) and 1,411 mg/kg or less total lead. This material is a California hazardous waste that may be reused as permitted under the variance of the DTSC provided that the lead contaminated soil is placed a minimum of 5 feet above the maximum historic water table elevation and covered with at least 1 foot of non-hazardous soil.

Type Y-2: Material that contains ADL in average concentrations (using the 90 percent Upper Confidence Limit) that exceed either 1.5 mg/L extractable lead (based on a modified waste extraction test using deionized water as the extractant) or 1,411 mg/kg total lead but are less than 150 mg/L extractable lead (based on a modified waste extraction test using deionized water as the extractant) and less than 3,397 mg/kg of total lead. This material is a California hazardous waste that may be reused as permitted under the variance of DTSC provided that the lead contaminated soil is placed a minimum of 5 feet above the maximum historic water table elevation and protected from infiltration by a pavement structure which will be maintained by the Department.

Type Z-2: Material that contains ADL in average concentrations (using the 95 percent Upper Confidence Limit) greater than or equal to 1,000 mg/kg total lead, greater than or equal to 5.0 mg/L soluble lead (as tested using the California Waste Extraction Test), and the material is surplus; or material that contains ADL in average concentrations greater than 150 mg/L extractable lead (based on a modified waste extraction test using deionized water as the extractant) or greater than 3,397 mg/kg total lead. This material is a Department-generated California hazardous waste and must be transported to and disposed of at a California Class I disposal site.

Type Z-3: Material that contains ADL in average concentrations (using the 95 percent Upper Confidence Limit) greater than 5.0 mg/L soluble lead, (as tested using the Toxicity Characteristic Leaching Procedure). This material is a Department-generated federal hazardous waste and must be transported to and disposed of at a California Class I disposal site.

14-11.03A(3) Site Conditions

ADL concentration data and sample locations maps are included in the *Information Handout*.

Type Z-2 material exists as shown.

14-11.03A(4) Submittals

14-11.03A(4)(a) Lead Compliance Plan

Submit a lead compliance plan under section 7-1.02K(6)(j)(ii).

14-11.03A(4)(b) Excavation and Transportation Plan

Within 15 days after approval of the Contract, submit 3 copies of an excavation and transportation plan. Allow 15 days for review. If revisions are required, as determined by the Engineer, submit the revised plan within 7 days of receipt of the Engineer's comments. For the revision, allow 7 days for the review. Minor changes to or clarifications of the initial submittal may be made and attached as amendments to the excavation and transportation plan. In order to allow construction to proceed, the Engineer may conditionally approve the plan while minor revisions or amendments are being completed.

Prepare the written, project specific excavation and transportation plan establishing the procedures you will use to comply with requirements for excavating, stockpiling, transporting, and placing or disposing of material containing ADL. The plan must comply with the regulations of the DTSC and Cal/OSHA and the requirements of the variance. The sampling and analysis portions of the excavation and transportation plan must meet the requirements for the design and development of the sampling plan, statistical analysis, and reporting of test results contained in US EPA, SW 846, "Test Methods for Evaluating Solid Waste," Volume II: Field Manual Physical/Chemical, Chapter Nine, Section 9.1. The plan must include the following elements:

1. Excavation schedule by location and date
2. Dust control measures
3. Air monitoring. Include the following information:
   - 3.1. Location and type of equipment
   - 3.2. Sampling frequency
   - 3.3. Name and address of the accredited laboratory where the analysis was performed
4. Transportation equipment and routes
5. Method for preventing spills and tracking material onto public roads
6. Truck waiting and staging areas
7. Site for disposal of hazardous waste
8. Spill Contingency Plan for material containing ADL

14-11.03A(4)(c) Burial Location Report

14-11.03A(4)(d) Bill of Lading

Copies of the bills of lading must be submitted as an informational submittal upon placement of Type Y-1 or Y-2 material in its final location.

14-11.03A(5) Quality Control and Assurance

Excavation, reuse, and disposal of material with ADL must comply with rules and regulations of the following agencies:

1. US DOT
2. US EPA
3. California Environmental Protection Agency
4. CDPH
5. DTSC
6. Cal/OSHA
7. California Department of Resources Recycling and Recovery
8. RWQCB, Region 4,Los Angeles
9. California Air Resources Board
10. South Coast Air Quality Management District

Transport and dispose of material containing hazardous levels of lead under federal and state laws and regulations and county and municipal ordinances and regulations. Laws and regulations that govern this work include:

1. Health & Safety Code, Division 20, Chp 6.5 (California Hazardous Waste Control Act)
2. 22 CA Code of Regs, Div. 4.5 (Environmental Health Standards for the Management of Hazardous Waste)
3. 8 CA Code of Regs

14-11.03B Materials

Not Used

14-11.03C Construction

14-11.03C(1) General

Not Used

14-11.03C(2) Material Management

Transport excavated Type Z-2 material using:

1. Hazardous waste manifest
2. Hazardous waste transporter with a current DTSC registration certificate and CA Highway Patrol (CHP) Biennial Inspection of Terminals (BIT) Program compliance documentation.

14-11.03C(3) Dust Control

Excavation, transportation, placement, and handling of material containing ADL must result in no visible dust migration. A water truck or tank must be on the job site at all times while clearing and grubbing or performing earthwork operations in work areas containing ADL. Apply water to prevent visible dust.

14-11.03C(4) Surveying Type Y-1 or Y-2 Material Burial Locations

Not Used

14-11.03C(5) Material Transportation

Before traveling on public roads, remove loose and extraneous material from surfaces outside the cargo areas of the transporting vehicles and cover the cargo with tarpaulins or other cover, as outlined in the approved excavation and transportation plan. You are responsible for costs due to spillage of material containing lead during transport. Transportation routes for Type Y-1 or Y-2 material must only include the highway.

14-11.03C(6) Disposal

Not Used

14-11.03D Payment

Payment for a lead compliance plan is not included in the payment for environmental stewardship work.

The Department does not pay for stockpiling of material containing ADL, unless the stockpiling is ordered. The Department does not pay for sampling and analysis unless it is ordered. The Department does not pay for additional sampling and analysis required by the receiving landfill.

Replace section 14-11.04 with:

14-11.04 MINIMAL DISTURBANCE OF MATERIAL CONTAINING HAZARDOUS WASTE CONCENTRATIONS OF AERIALLY DEPOSITED LEAD

14-11.04A General

14-11.04A(1) Summary

Section 14-11.04 includes specifications for minimal disturbance of material containing hazardous waste concentrations of Aerially Deposited Lead (ADL).

Compliance with 22 CA Code of Regs is not required where there is minimal disturbance of hazardous waste concentrations of ADL.

14-11.04A(2) Project Conditions

Hazardous waste concentrations of ADL are typically found within the top 2 feet of material in unpaved areas of the highway.

Levels found in the area of minimal disturbance range from less than 820 to 1.8 mg/kg total as analyzed by US EPA Method 6010 or US EPA Method 7000 series.

Minimal disturbance of hazardous waste concentrations of ADL will occur at the following locations:

1. Midwest guardrail system.
2. Construction area signs installation.
3. Cast-in-drilled-hole concrete pile for sign foundation.

14-11.04A(3) Quality Control and Assurance

Handling material containing aerially deposited lead must comply with rules and regulations of the following agencies:

1. Cal/OSHA
2. RWQCB, Region 4—Los Angeles

14-11.04A(4) Lead Compliance Plan

Submit a lead compliance plan under section 7-1.02K(6)(j)(ii).

14-11.04B Materials

Not Used

14-11.04C Construction

14-11.04C(1) General

Not Used

14-11.04C(2) Soil Handling

Handling of material containing ADL must result in no visible dust migration. Use dust control measures. A means of controlling dust must be available at all times when handling material in work areas containing ADL at hazardous waste concentrations.

Separate material from vegetation. The resulting soil must remain on the job site.

Surplus material from the areas containing hazardous waste concentrations of ADL must remain in the area of disturbance. Do not dispose of surplus material outside the highway.

14-11.04D Payment

Payment for a lead compliance plan is not included in the payment for environmental stewardship work.

Replace section 14-11.07 with:

14-11.07 REMOVE YELLOW TRAFFIC STRIPE AND PAVEMENT MARKING WITH HAZARDOUS WASTE RESIDUE

14-11.07A General

14-11.07A(1) Summary

Section 14-11.07 includes specifications for removing existing yellow thermoplastic and yellow painted traffic stripe and pavement marking. The residue from the removal of this material is a Department-generated hazardous waste.

Residue from removal of yellow thermoplastic and yellow painted traffic stripe and pavement marking contains lead chromate. The average lead concentration is at least 1,000 mg/kg total lead or 5 mg/l soluble lead. When applied to the roadway, the yellow thermoplastic and yellow painted traffic stripe and pavement marking contained as much as 2.6 percent lead. Residue produced from the removal of this yellow thermoplastic and yellow painted traffic stripe and pavement marking contains heavy metals in concentrations that exceed thresholds established by the Health & Safety Code and 22 CA Code of Regs. For bidding purposes, assume the residue is not regulated under the Federal Resource Conservation and Recovery Act (RCRA), 42 USC § 6901 et seq.

Work associated with disposal of hazardous waste residue regulated under RCRA as determined by test results is change order work.

Yellow thermoplastic and yellow paint may produce toxic fumes when heated.

14-11.07A(2) Submittals

14-11.07A(2)(a) General

Reserved

14-11.07A(2)(b) Lead Compliance Plan

Submit a lead compliance plan under section 7-1.02K(6)(j)(ii).

14-11.07A(2)(c) Work Plan

Submit a work plan for the removal, containment, storage, and disposal of yellow thermoplastic and yellow painted traffic stripe and pavement marking. The work plan must include:

1. Objective of the operation
2. Removal equipment
3. Procedures for removal and collection of yellow thermoplastic and yellow painted traffic stripe and pavement marking residue, including dust
4. Type of hazardous waste storage containers
5. Container storage location and how it will be secured
6. Hazardous waste sampling protocol and QA/QC requirements and procedures
7. Qualifications of sampling personnel
8. Analytical lab that will perform the analyses
9. DTSC registration certificate and CA Highway Patrol (CHP) Biennial Inspection of Terminals (BIT) Program compliance documentation of the hazardous waste hauler that will transport the hazardous waste
10. Disposal site that will accept the hazardous waste residue

The Engineer will review the work plan within 5 business days of receipt.

Do not perform work that generates hazardous waste residue until the work plan has been authorized.

Correct any rejected work plan and resubmit a corrected work plan within 5 business days of notification by the Engineer. A new review period of 5 business days will begin from date of resubmittal.

14-11.07A(2)(d) Analytical Test Results

Submit analytical test results of the residue from removal of yellow thermoplastic and yellow painted traffic stripe and pavement marking, including chain of custody documentation, for review and acceptance before:

1. Requesting the Engineer's signature on the waste profile requested by the disposal facility
2. Requesting the Engineer obtain an US EPA Generator Identification Number for disposal
3. Removing the residue from the site

14-11.07A(2)(e) U.S. Environmental Protection Agency Identification Number Request

Submit a request for the US EPA Generator Identification Number when the Engineer accepts analytical test results documenting that residue from removal of yellow thermoplastic and yellow painted traffic stripe and pavement marking is a hazardous waste.

14-11.07A(2)(f) Disposal Documentation

Submit documentation of proper disposal from the receiving landfill within 5 business days of residue transport from the project.

14-11.07B Materials

Not Used

14-11.07C Construction

Where grinding or other authorized methods are used to remove yellow thermoplastic and yellow painted traffic stripe and pavement marking that will produce a hazardous waste residue, immediately contain and collect the removed residue, including dust. Use a HEPA filter-equipped vacuum attachment operated concurrently with the removal operations or other equally effective approved methods for collection of the residue.

Make necessary arrangements to test the yellow thermoplastic and yellow paint hazardous waste residue as required by the disposal facility and these special provisions. Testing must include:

1. Total lead by US EPA Method 6010B
2. Total chromium by US EPA Method 6010B
3. Soluble lead by California Waste Extraction Test (CA WET)
4. Soluble chromium by CA WET
5. Soluble lead by Toxicity Characteristic Leaching Procedure (TCLP)
6. Soluble chromium by TCLP

From the first 220 gal of hazardous waste or portion thereof if less than 220 gal of hazardous waste are produced, a minimum of 4 randomly selected samples must be taken and analyzed individually. Samples must not be composited. From each additional 880 gal of hazardous waste or portion thereof if less than 880 gal are produced, a minimum of 1 additional random sample must be taken and analyzed. Use chain of custody procedures consistent with chapter 9 of US EPA Test Methods for Evaluating Solid Waste, Physical/Chemical Methods (SW-846) while transporting samples from the project to the laboratory. Each sample must be homogenized before analysis by the laboratory performing the analyses. A sample aliquot sufficient to cover the amount necessary for the total and the soluble analyses must then be taken. This aliquot must be homogenized a 2nd time and the total and soluble analyses run on this aliquot. The homogenization process must not include grinding of the samples. Submit the name and location of the disposal facility that will be accepting the hazardous waste and the analytical laboratory along with the testing requirements not less than 5 business days before the start of removal of yellow thermoplastic and yellow painted traffic stripe and pavement marking. The analytical laboratory must be certified by the California Department of Public Health (CDPH) Environmental Laboratory Accreditation Program (ELAP) for all analyses to be performed.

After the Engineer accepts the analytical test results, dispose of yellow thermoplastic and yellow paint hazardous waste residue at a Class 1 disposal facility located in California under the requirements of the disposal facility operator within 30 days after accumulating 220 pounds of residue and dust.

If less than 220 pounds of hazardous waste residue and dust is generated in total, dispose of it within 30 days after the start of accumulation of the residue and dust.

The Engineer will sign all manifests as the generator within 2 business days of receiving and accepting the analytical test results and receiving your request for the US EPA Generator Identification Number. Use a transporter with a current DTSC registration certificate and that is in compliance with the CHP BIT Program when transporting hazardous waste.

14-11.07D Payment

Payment for a lead compliance plan is not included in the payment for environmental stewardship work.

If analytical test results demonstrate that the residue is a non-hazardous waste and the Engineer agrees, dispose of the residue at an appropriately permitted CA Class II or CA Class III facility. The Department does not adjust payment for this disposal.

Replace "Reserved" in section 14-11.08 with:

14-11.08A General

Section 14-11.08 includes specifications relating to the disturbance of existing paint systems.

The existing paint system on bridge number 53-2341, 53-2256, and 53-2268 and inside tunnels 1,2 and 3 are expected to contain lead. Any work that disturbs the existing paint system potentially exposes workers to health hazards and produces:

1. Debris containing heavy metal in amounts that exceed the thresholds established in 8 CA Code of Regs and 22 CA Code of Regs. This debris is a Department-generated hazardous waste.
2. Toxic fumes when heated.

The Department is the hazardous waste generator if the Engineer accepts waste-characterization test results demonstrating that the debris is a hazardous waste.

Contain all debris produced when the existing paint system is removed. If containment measures are inadequate to contain and collect debris produced when the existing paint system is removed, stop the work and do not perform additional work until:

1. Revised debris containment and collection plan has been authorized
2. Released material has been collected and contained

Handle, store, transport, and dispose of debris produced when the existing paint system is removed under applicable federal, state, and local hazardous waste laws.

14-11.08B Submittals

14-11.08B(1) General

Not Used

14-11.08B(2) Debris Containment and Collection Plan

Submit a debris containment and collection plan. The plan must:

1. Identify materials, equipment, and methods to be used when the existing paint system is removed
2. Include shop drawings of:
   2.1. Containment systems complying with section 59-2.03B(3)
   2.2. Components that provide ventilation, air movement, and visibility for worker safety
3. Include the name and location of the analytical laboratory that will perform the analyses
4. Identify the hazardous waste transporter that will haul the debris and provide documentation of
   4.1 Current DTSC registration
   4.2 Compliance with the CA Highway Patrol Biennial Inspection of Terminals Program
5. Include the name and location of the disposal facility that will accept the hazardous waste

Allow 20 days for review.

If required, submit a revised debris containment and collection plan.

14-11.08B(3) Lead Compliance Plan

Submit a lead compliance plan under section 7-1.02K(6)(j)(ii).

14-11.08B(4) Air Monitoring Reports

Air monitoring reports, including test results for samples taken after corrective action, must be prepared by the CIH and submitted:

1. Verbally within 48 hours after sampling
2. As an informational submittal within 5 days after sampling

Air monitoring reports must include:

1. Date and location of sample collection, sample number, contract number, full name of the structure, and District-County-Route-Post mile
2. Name and address of the certified laboratory that performed the analyses
3. Chain of custody documentation
4. List of emission control measures in place when air samples were taken
5. Air sample results compared to the appropriate permissible exposure limit (PEL)
6. Corrective action recommended by the CIH to ensure exposure to airborne metals outside containment systems and work areas is within specified limits
7. Signature of the CIH who reviewed the data and made recommendations

14-11.08B(5) Soil Sampling Results for Debris Containment Verification

Not Used

14-11.08B(6) Waste-Characterization Test Results

Submit waste-characterization test results for the debris and chain of custody documentation before:

1. Requesting the Engineer's signature on the disposal facility's waste profile document
2. Requesting a generator's EPA Identification Number
3. Removing the debris from the site

14-11.08B(7) Request for U.S. Environmental Protection Agency Identification Number

Submit a request for the generator's EPA Identification Number when the Engineer accepts waste-characterization test results documenting that the debris is a hazardous waste.

14-11.08B(8) Disposal Documentation

Submit documentation from the receiving landfill or recycling facility confirming proper disposal within 5 business days of transporting debris from the project.

14-11.08C Safety and Health Provisions

14-11.08C(1) General

Comply with 8 CA Code of Regs, including § 1532.1.

14-11.08C(2) Protective Work Clothing and Washing Facilities

Supply clean protective work clothing for 5 Department personnel:

1. Whenever there is possible exposure to heavy metals or silica dust
2. During application of paint undercoats

Replace protective work clothing as needed.

Protective work clothing and washing facilities must be inspected and authorized for use by Department personnel before starting any activity with the potential for lead exposure.

Protective work clothing remains your property upon completion of the Contract.

14-11.08D Work Area Monitoring

14-11.08D(1) General

Monitor the ambient air in and around the work area to verify the effectiveness of the containment system. Work area monitoring includes:

1. Collecting, analyzing, and reporting air test results
2. Recommending corrective action when specified air concentrations are exceeded

Collect air samples at locations designated by the Engineer.

14-11.08D(2) Air Monitoring

Air monitoring must be performed under the direction of a CIH.

Collect and analyze air samples to detect lead under the National Institute of Occupational Safety and Health (NIOSH) Method 7082 using a detection limit of at least 0.05 $\mu g/m^3$. Collect and analyze air samples to detect other metals under NIOSH Method 7300 using a detection limit of at least 1 percent of the appropriate PEL specified by Cal/OSHA. You may use alternative methods of sampling and analysis with equivalent detection limits.

Concentrations of airborne metals outside containment systems and work areas must not exceed any of the following:

1. Average of 1.5 $\mu g/m^3$ of air per day and 0.15 $\mu g/m^3$ per day on a rolling 90-day basis. Calculate average daily concentrations based on monitoring to date and projections based on monitoring trends for the next 90 days or to the end of work subject to the lead compliance plan if less than the specified averaging period.
2. 10 percent of the action level specified for lead by 8 CA Code of Regs §1532.1.
3. 10 percent of the appropriate PELs specified for other metals by Cal/OSHA.

Collect air samples daily during work activities that disturb the existing paint system. Air samples must be analyzed within 48 hours by a facility accredited by the Environmental Lead Laboratory Accreditation Program of the American Industrial Hygiene Association. If concentrations of airborne metals exceed allowable levels, modify the containment system or work activities to prevent further release of metals. If the CIH recommends corrective action, collect and analyze additional samples after implementing the corrective action unless directed otherwise.

14-11.08D(3) Soil Sampling for Debris Containment

Not Used

14-11.08E Debris Management

14-11.08E(1) Debris Storage

Debris produced when the existing paint system is removed must not be temporarily stored on the ground. Before the end of each work shift, remove accumulated debris from the containment system. Store the debris as a hazardous waste.

14-11.08E(2) Debris Waste Characterization

Perform waste characterization testing on the debris as required by the disposal facility including:

1. Total lead by US EPA Method 6010B
2. Soluble lead by California Waste Extraction Test (CA WET)
3. Soluble lead by Toxicity Characteristic Leaching Procedure (TCLP)

From the first 220 gal of hazardous waste or portion thereof, if less than 220 gal of hazardous waste are produced, a minimum of 4 randomly selected samples must be taken and analyzed individually. Samples must not be composited. From each additional 880 gal of hazardous waste or portion thereof, if less than 880 gal are produced, a minimum of 1 additional random sample must be taken and analyzed.

Use chain of custody procedures consistent with chapter 9 of US EPA Test Methods for Evaluating Solid Waste, Physical/Chemical Methods (SW-846) while transporting samples from the job site to the analytical laboratory. The laboratory must be certified by the CDPH's Environmental Laboratory Accreditation Program (ELAP) for all analyses to be performed.

Before performing the analyses, the laboratory must homogenize each sample. The homogenization process must not include grinding of the samples. A sample aliquot must be:

1. Obtained in an amount large enough for all analyses to be performed
2. Homogenized a 2nd time
3. Used for the total and soluble analyses after the 2nd homogenization

14-11.08E(3) Debris Transport and Disposal

14-11.08E(3)(a) General

For bidding purposes, assume the debris is a hazardous waste.

14-11.08E(3)(b) Hazardous Waste Debris

After the Engineer accepts the waste-characterization test results, dispose of the debris:

1. Within 30 days after accumulating 220 lb of debris
2. At an appropriately permitted Class I facility located in California

Make all arrangements with the operator of the disposal facility.

If less than 220 lb of hazardous waste is generated in total, dispose of it within 90 days after the start of accumulation of the debris.

Use a hazardous waste manifest and a transporter using vehicles with current DTSC registration certificate when transporting hazardous waste. The Engineer provides the generator's EPA Identification Number and signs all manifests as the hazardous waste generator within 2 business days of accepting the waste-characterization test results and receiving your request for the generator's EPA Identification Number.

14-11.08E(3)(c) Nonhazardous Waste Debris

If waste characterization test results demonstrate that the debris is a nonhazardous waste and the Engineer accepts the results, dispose of the debris at an appropriately permitted CA Class II or CA Class III facility or recycle it. Make all arrangements with the operator of the disposal facility and comply with the facility's requirements.

You may dispose of nonhazardous debris at a facility equipped to recycle the debris if:

1. Copper slag abrasive blended by the supplier with a calcium silicate compound is used for blast cleaning.
2. You make all arrangements with the recycling facility's operator and perform any facility-required testing of the debris.

The Department does not adjust payment for disposal of nonhazardous debris at a recycling facility.

Replace section 14-11.09 with:

14-11.09 TREATED WOOD WASTE

14-11.09A General

14-11.09A(1) Summary

Section 14-11.09 includes specifications for handling, storing, transporting, and disposing of treated wood waste (TWW).

Wood removed from guard railing and roadside signs is TWW. Manage TWW under 22 CA Code of Regs, Div. 4.5, Chp. 34.

14-11.09A(2) Submittals

For disposal of TWW, submit as an informational submittal a copy of each completed shipping record and weight receipt within 5 business days.

14-11.09B Materials

Not Used

14-11.09C Construction

14-11.09C(1) General

Not Used

14-11.09C(2) Training

Provide training to personnel who handle TWW or may come in contact with TWW. Training must include:

1. Applicable requirements of 8 CA Code of Regs
2. Procedures for identifying and segregating TWW
3. Safe handling practices
4. Requirements of 22 CA Code of Regs, Div. 4.5, Chp. 34
5. Proper disposal methods

Maintain records of personnel training for 3 years.

14-11.09C(3) Storage

Store TWW before disposal using the following methods:

1. Elevate on blocks above a foreseeable run-on elevation and protect from precipitation for no more than 90 days.
2. Place on a containment surface or pad protected from run-on and precipitation for no more than 180 days.
3. Place in water-resistant containers designed for shipping or solid waste collection for no more than 1 year.
4. Place in a storage building as defined in 22 CA Code of Regs, Div. 4.5, Chp. 34, § 67386.6(a)(2)(C).

Prevent unauthorized access to TWW using a secured enclosure such as a locked chain-link-fenced area or a lockable shipping container located within the job site.

Resize and segregate TWW at a location where debris from the operation including sawdust and chips can be contained. Collect and manage the debris as TWW.

Provide water-resistant labels that comply with 22 CA Code of Regs, Div. 4.5, Chp. 34, §67386.5, to clearly mark and identify TWW and accumulation areas. Labels must include:

1. Caltrans, District number, Construction, Construction Contract number
2. District office address
3. Engineer's name, address, and telephone number
4. Contractor's contact name, address and telephone number
5. Date placed in storage

14-11.09C(4) Transporting and Disposal

Before transporting TWW, obtain an agreement from the receiving facility that the TWW will be accepted. Protect shipments of TWW from loss and exposure to precipitation. For projects with 10,000 lb or more of TWW, request a generator's EPA Identification Number at least 5 business days before the 1st shipment. Each shipment must be accompanied by a shipping record such as a bill of lading or invoice that includes:

1. Caltrans with district number
2. Construction Contract number
3. District office address
4. Engineer's name, address, and telephone number
5. Contractor's contact name and telephone number
6. Receiving facility name and address
7. Waste description: Treated Wood Waste with preservative type if known or unknown/mixture
8. Project location
9. Estimated quantity of shipment by weight or volume
10. Date of transport
11. Date of receipt by the receiving TWW facility
12. Weight of shipment as measured by the receiving TWW facility
13. Generator's EPA Identification Number for projects with 10,000 lb or more of TWW

The shipping record must be at least a 4-part carbon or carbonless 8-1/2-by-11-inch form to allow retention of copies by the Engineer, transporter, and disposal facility.

Dispose of TWW at an approved TWW facility. A list of currently approved TWW facilities is available at:

http://www.dtsc.ca.gov/HazardousWaste/upload/lanfillapr11pdated1.pdf

Dispose of TWW within:

1. 90 days of generation if stored on blocks
2. 180 days of generation if stored on a containment surface or pad
3. 1 year of generation if stored in a water-resistant container or within 90 days after the container is full, whichever is shorter
4. 1 year of generation if storing in a storage building as defined in 22 CA Code of Regs, Div. 4.5, Chp. 34, § 67386.6(a)(2)(C)

14-11.09D Payment

Not Used

Replace section 14-11.10 with:

14-11.10 DISPOSAL OF ELECTRICAL EQUIPMENT REQUIRING SPECIAL HANDLING

14-11.10A General

Section 14-11.10 includes specifications for disposal of electrical equipment containing hazardous materials.

14-11.10B Waste Management

14-11.10B(1) Universal Waste

Management of universal waste must comply with 22 CA Code Regs § 66261.9.

Transport fluorescent tubes, bulbs, and lamps, and mercury lamps to an appropriately permitted facility.

Vehicle sensor nodes (VSNs) contain lithium thionyl chloride (LTC) batteries. Thionyl chloride is designated as an extremely hazardous waste under 22 CA Code of Regs, Div. 4.5, Chp 11, Art. 5, App.10. Each VSN includes one integral LTC battery.

Package removed VSNs containing undamaged LTC batteries in sealed shipping containers and transport to a recycling or disposal facility. Notify the receiving facility 48 hours before delivery. Affix a label on packages of intact VSN that states the package contains lithium thionyl chloride batteries.Shipments of VSN batteries to receiving facility for recycling or disposal that are separated from a VSN must comply with the requirement of 49 CFR 173.185. All batteries must be packaged in such a manner that prevents short circuits under conditions normally encountered during transportation.

1. Cover terminal ends to prevent them from touching one another
2. Place batteries in sealed plastic bags and pack with loose fill, such as vermiculite, to prevent accidental contact.

The outer packaging must comply with 49 CFR 173.24 and 173.24a. Transport LTC batteries to an approved hazardous waste recycling/disposal facility. A partial list of facilities may be viewed at:

http://www.dtsc.ca.gov/HazardousWaste/upload/LIST_HWM_Commercial_Facilities.pdf

or

http://www.calrecycle.ca.gov/Electronics/Collection/RecyclerList.aspx

14-11.10B(2) Fluorescent light ballasts containing PCBs

Disposal of fluorescent light ballasts that contain PCBs must comply with 22 CA Code of Regs § 67426.1 et seq. Ballasts must be packaged and transported by a hauler with a current DTSC registration certificate and documentation of compliance with the CA Highway Patrol Biennial Inspection of Terminals Program to a DTSC permitted hazardous waste disposal facility.

14-11.10C Damaged Electrical Equipment

Damaged electrical equipment is a Department generated hazardous waste and must be transported to hazardous waste disposal facility under hazardous waste manifest.

PCB and thionyl chloride are designated as extremely hazardous wastes.

You are the generator of and responsible for cleanup, management, disposal and associated costs of hazardous waste generated as a result of mishandling electrical equipment.

14-11.10D Payment

Management of Department generated hazardous waste is change order work.

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15 EXISTING FACILITIES

Replace section 15-1.03B with:

15-1.03B Residue Containing Lead from Paint and Thermoplastic

Residue from grinding or cold planing contains lead from paint and thermoplastic. The average lead concentrations are less than 1,000 mg/kg total lead and 5 mg/L soluble lead. This residue:

1. Is a nonhazardous waste
2. Does not contain heavy metals in concentrations that exceed thresholds established by the Health and Safety Code and 22 CA Code of Regs
3. Is not regulated under the Federal Resource Conservation and Recovery Act (RCRA), 42 USC § 6901 et seq.

Submit a lead compliance plan under section 7-1.02K(6)(j)(ii).

Payment for a lead compliance plan is not included in the payment for existing facilities work.

Payment for handling, removal, and disposal of grinding or cold planing residue that is a nonhazardous waste is included in the payment for the type of removal work involved.

Replace section 15-2.02B(3) with:

15-2.02B(3) Cold Planing Asphalt Concrete Pavement

15-2.02B(3)(a) General

Schedule cold planing activities so that not more than 24 hours elapses between the time the pavement is cold planed and the HMA is placed.

15-2.02B(3)(b) Materials

Use the same quality of HMA for temporary tapers that is used for the HMA overlay or comply with the specifications for minor HMA in section 39.

15-2.02B(3)(c) Construction

15-2.02B(3)(c)(i) General

Do not use a heating device to soften the pavement.

The cold planing machine must be:

1. Equipped with a cutter head width that matches the planing width. If the cutter head width is wider than the cold plane area shown, submit to the Engineer a request for using a wider cutter head. Do not cold plane unless the Engineer approves your request.
2. Equipped with automatic controls for the longitudinal grade and transverse slope of the cutter head and:
   - 2.1. If a ski device is used, it must be at least 30 feet long, rigid, and a 1-piece unit. The entire length must be used in activating the sensor.
   - 2.2. If referencing from existing pavement, the cold planing machine must be controlled by a self-contained grade reference system. The system must be used at or near the centerline of the roadway. On the adjacent pass with the cold planing machine, a joint-matching shoe may be used.
3. Equipped to effectively control dust generated by the planing operation
4. Operated so that no fumes or smoke is produced.

Replace broken, missing, or worn machine teeth.

15-2.02B(3)(c)(ii) Grade Control and Surface Smoothness

Furnish, install, and maintain grade and transverse slope references.

The depth, length, width, and shape of the cut must be as shown or as ordered. The final cut must result in a neat and uniform surface. Do not damage the remaining surface.

The completed surface of the planed asphalt concrete pavement must not vary more than 0.02 foot when measured with a 12-foot straightedge parallel with the centerline. With the straightedge at right angles to the centerline, the transverse slope of the planed surface must not vary more than 0.03 foot.

Where lanes are open to traffic, the drop-off of between adjacent lanes must not be more than 0.15 foot.

15-2.02B(3)(c)(iii) Temporary HMA Tapers

If a drop-off between the existing pavement and the planed area at transverse joints cannot be avoided before opening to traffic, construct a temporary HMA taper. The HMA temporary taper must be:

1. Placed to the level of the existing pavement and tapered on a slope of 30:1 (horizontal:vertical) or flatter to the level of the planed area
2. Compacted by any method that will produce a smooth riding surface

Completely remove temporary tapers before placing permanent surfacing.

15-2.02B(3)(c)(iv) Remove Planed Material

Remove cold planed material concurrent with planing activities so that removal does not lag more than 50 feet behind the planer.

15-2.02B(3)(d) Payment

Payment for removal of pavement markers, thermoplastic traffic stripe, painted traffic stripe, and pavement marking within the area of cold planing is included in the payment for cold plane asphalt concrete pavement of the types shown in the Bid Item List.

Replace section 15-2.02C(2) with:

15-2.02C(2) Remove Traffic Stripes and Pavement Markings Containing Lead

Residue from removing traffic stripes and pavement markings contains lead from the paint or thermoplastic. The average lead concentrations are less than 1,000 mg/kg total lead and 5 mg/L soluble lead. This residue:

1. Is a nonhazardous waste
2. Does not contain heavy metals in concentrations that exceed thresholds established by the Health and Safety Code and 22 CA Code of Regs
3. Is not regulated under the Federal Resource Conservation and Recovery Act (RCRA), 42 USC § 6901 et seq.

Submit a lead compliance plan under section 7-1.02K(6)(j)(ii).

Payment for a lead compliance plan is not included in the payment for existing facilities work.

Payment for handling, removal, and disposal of pavement residue that is a nonhazardous waste is included in the payment for the type of removal work involved.

Replace section 15-2.02I with:

15-2.02I Remove Sign Structures

Removing overhead sign structures includes removal of:

1. Frames, braces, supports, and brackets
2. Portions of foundations
3. Sign panels
4. Mounting hardware for light fixtures
5. Walkways, safety railing, gutter
6. Electrical equipment for sign lighting
7. Hardware
8. Posts
9. Portions of foundations

Concrete foundations may be abandoned in place except that the top portion, including anchor bolts, reinforcing steel, and conduits, must be removed to a depth of not less than 3 feet below the adjacent finished grade. The resulting holes must be backfilled and compacted with material that is equivalent to the surrounding material.

Remove signs' conduit and wiring to the nearest pull box. Remove fuses within spliced connections in the pull box.

Replace section 15-2.03A(2)(b) with:

15-2.03A(2)(b) Department Salvage Location

A minimum of 2 business days before hauling salvaged material to the Department salvage storage location, notify:

1. Engineer
2. District Recycle coordinator at telephone number (213) 620-2069, for irrigation facility.
   District Recycle coordinator at telephone number (626) 794-6545 or (818) 422-6459 for guard rail

For irrigation facility, the Department salvage storage location is:

2133 Riverside Drive
Los Angeles, CA 90039
For guard rail, the Department salvage storage location is:

Altadena Maintenance Field Office
2122 N. Windsor Ave., Altadena, CA 91001

Replace section 15-2.03C with:

15-2.03C Salvage Irrigation Facilities

With each delivery to the salvage location, include a list of the quantity and size of each item.

Delete the 4th paragraph of section 15-3.01 in the RSS for section 15-3.01.

Add to section 15-4.01C(1):

Remove the following portions of bridges:

Bridge no.	Description of work
53-2282 (LOCATION A)	Concrete barriers, Overhangs, Guard rail anchor block
53-2308 (LOCATION B)	Concrete barriers, Guard rail anchor block
53-2310 (LOCATION C)	Concrete barriers, Overhangs, Guard rail anchor block, Bridge approach guard railing (BAGR)
53-2219 (LOCATION D)	Concrete barriers., Guard rail anchor block
53-2191G (LOCATION E)	Concrete barriers, Guard rail anchor block
53-2199 (LOCATION F)	Concrete barriers, Guard rail anchor block
53-0734 (LOCATION G)	Concrete barriers, Guard rail anchor blocks
53-2202 (LOCATION H)	Concrete barriers, Guard rail anchor blocks
53-0579 (LOCATION I)	Concrete barriers, Guard rail anchor blocks
53-2173 (LOCATION J)	Concrete barriers, Guard rail anchor block
53-2154 (LOCATION K)	Concrete barriers
53-2155 (LOCATION L)	Concrete barriers, Guard rail anchor block
53-2157 (LOCATION M)	Concrete barriers, Guard rail anchor block, Bridge approach guard railing
53-2158 (LOCATION N)	Concrete barriers, Guard rail anchor block
53-2159 (LOCATION O)	Concrete barriers, Guard rail anchor block, Bridge approach guard railings
53-2160 (LOCATION P)	Concrete barriers, Guard rail anchor blocks

Add to section 15-5.01C(1):

Remove any unsound concrete under section 15-5.01C(5) and place any rapid setting concrete patches under section 15-5.02. This is change order work.

Replace item 1 in the list in the 1st paragraph of section 15-5.06A(2) with:

1. Schedule of overlay work for each bridge

Replace the 13th paragraph of section 15-5.06C(1) with:

The approximate rate of application of methacrylate resin is 90 sq ft/gal.

Replace the 21st paragraph of section 15-5.06C(1) with:

Completed polyester-concrete deck surfaces must have a uniform surface texture with a coefficient of friction of at least 0.35 when tested under California Test 342 and a surface smoothness complying with section 51-1.01D(4)(b).

Add to section 15-5.06C(1):

Do not place polyester concrete overlay until at least 28 days after placing new portland cement concrete.

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Add to section 20-1.02C of the RSS for section 20:

Select herbicides from the following table:

Herbicides

Herbicide name	Herbicide type					
	Preemergent (granular)	Preemergent (non-granular)	Post-emergent	Selective	Non-selective	Systemic
Aminopyralid				X		
Chlorsulfuron				X		
Clopyralid MEA					X	
Diquat dibromide					X	
Dithiopyr		X				
Fluazifop-P-Butyl				X		
Flumioxazin				X		
Glyphosate			X			X
Imazapyr					X	
Isoxaben		X				
Oryzalin		X				
Oxadiazon	X	X				X
Oxyfluorfen (odorless)		X	X			
Pendimethalin	X	X	X			
Prodiamine		X				
Rimsulfuron				X		
Sethoxydim			X	X		
Sulfentrazone					X	
Sulfometuron-methyl					X	
Sulfosulfuron					X	
Triclopyr						X

Delete the 5th paragraph of section 20-1.03A of the RSS for section 20.

Add to section 20-1.03C(3) of the RSS for section 20:

Control weeds within the highway including medians and surfaced areas such as new and existing pavement, curbs, and sidewalks.

Replace the 2nd paragraph in section 20-1.03C(4) of the RSS for section 20 with:

Dispose of mowed material from initial and subsequent mowing during roadside clearing.

Replace the 1st paragraph in section 20-2.11B(2) of the RSS for section 20 with:

Ball valve must be PVC or chlorinated PVC and must comply with the requirements shown in the following table:

Property	Requirements
Nonshock working pressure for 3/4 to 4 inch valves, min	235 psi
Nonshock working pressure for 6 inch valves, min	150 psi
Seats	PTFE
O-ring seals	EPDM or fluoroelastomer

Add to section 20-4.01A of the RSS for section 20:

The plant establishment period must be Type 2.

Add to section 20-4.03C of the RSS for section 20:

Apply slow-release fertilizer to the plants during the 1st week of April and October of each year.

^^^

3. Without wrinkles

Do not use a polyester-type geotextile wrap.

Overlap adjacent edges of biaxial geogrid rolls and geotextile wrap at least 2 feet. Overlap the ends of the rolls at least 2 feet in the direction you spread AB covering the biaxial geogrid and geotextile wrap.

You may fold or cut biaxial geogrid and geotextile wrap to conform to curves. If cut, overlap at least 2 feet. You may hold material in place with mechanical ties, staples, pins, or small piles of AB.

Do not:

1. Stockpile material on biaxial geogrid and geotextile wrap
2. Place more biaxial geogrid and geotextile wrap than can be covered in 72 hours
3. Operate equipment or vehicles directly on the geotextile wrap
4. Compact with a sheepsfoot or other non-smooth roller
5. Turn vehicles on material placed directly over geotextile

You may operate vehicles and equipment on biaxial geogrid if one of the following conditions is met:

1. Vehicles and equipment are:
   1.1. Equipped with rubber tires
   1.2. Operated under 10 mph
   1.3. Operated in a manner to avoid sudden braking and sharp turns
2. At least 0.35 ft of AB has been placed, spread, and compacted on the biaxial geogrid
3. At least 0.5 ft of AB has been placed, spread, and compacted on the geotextile wrap

Compact AB with either (1) a smooth-wheeled roller or (2) a rubber-tired roller. Do not use vibratory devices during compaction.

Repair or replace damaged biaxial geogrid and geotextile wrap. Repair biaxial geogrid and geotextile by placing a new piece of material with at least 3 feet of overlap from the edges of the damaged area.

The 1st through 3rd paragraphs of section 26-1.03 do not apply to ABCS.

Store, proportion, mix, transport, and deposit the cement, water, and aggregate in compliance with recognized standards of good practice that result in thoroughly and uniformly mixed ABSC suitable for the intended use. Recognized standards of good practice are outlined in various industry publications, such as those issued by ACI, AASHTO, or the Department.

Use no more than 50 pounds of cement per cubic yard of ABCS. Use sufficient water to produce a stiff workable mixture that can be placed and finished in trenches and narrow excavations, without segregation.

You may spread ABCS in one or more layers, to prevent floating and shifting of pipes and culverts. Remove foreign material that falls into trenches.

Place ABCS no higher than the bottom of the base or pavement layers to be constructed.

Whenever ABCS is placed within the travelled way or covered by paving or embankment materials, the ABCS much achieve a maximum indentation diameter of 2 inches, determined under ASTM D 6024, before covering and opening to traffic.

Add to section 26-1.03C:

The 1st paragraph of section 26-1.03D does not apply to ABCS.

Use high-frequency internal vibrators within 15 minutes of depositing ABCS on the subgrade to uniformly consolidate the ABCS across the trench or excavation width including adjacent forms. Do not use the vibrators to shift the mass of ABCS.

Add to section 26-1.04:

ABCS is paid for as Class 3 AB.

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28 CONCRETE BASES

Replace "Reserved" in section 28-4 of the RSS for section 28 with:

28-4.01 GENERAL

28-4.01A Summary

Section 28-4 includes specifications for constructing lean concrete base rapid setting (LCBRS).

28-4.01B Definitions

final set time: Elapsed time after initial contact of cement and water required for the mortar sieved from the concrete to develop a penetration resistance of 4,000 psi under ASTM C 403.

opening age: Age when the LCBRS achieves the specified strength for opening to traffic including construction traffic.

28-4.01C Submittals

28-4.01C(1) General

At least 25 days before field qualification, submit the name of your proposed testing laboratory.

Submit quality control test results within 24 hours of test completion.

28-4.01C(2) Mix Design

Determine the mix proportions for LCBRS and submit mix designs.

At least 10 days before placing LCBRS, submit a mix design for LCBRS and include:

1. Opening age
2. Proposed aggregate gradation
3. Mix design, including
   - 3.1. Proportions
   - 3.2. Types and amounts of chemical admixtures
4. Maximum time allowed between batching and placing
5. Range of ambient temperatures over which the mix design is applicable
6. Final-set-time
7. Aggregate qualification test results if required

Submit 1 mix design for each ambient temperature variation anticipated during LCBRS placement. Each mix design must have a maximum ambient temperature range of 18 degrees F.

Submit compressive strength development data for each mix design. You may use strength development data from laboratory-prepared samples. The testing ages for strength development data must include 1 hour before opening age, opening age, 1 hour after opening age, 24 hours, and 7 days.

28-4.01C(3) Field Qualification

Submit field qualification data and test reports including:

1. Mixing date
2. Mixing equipment and procedures used
3. Batch volume in cubic yards, minimum 5 cu yd
4. Type and source of ingredients used
5. Age and strength at time of cylinder testing

Field qualification test reports must be certified with a signature by an official in responsible charge of the laboratory performing the tests.

28-4.01D Quality Control and Assurance

28-4.01D(1) General

Stop LCBRS activities and immediately notify the Engineer if:

1. Any quality control or acceptance test result does not comply with the specifications
2. Visual inspection shows noncompliant LCBRS

If LCBRS activities are stopped, before resuming activities:

1. Inform the Engineer of the adjustments you will make
2. Remedy or replace the noncompliant LCBRS until it complies with specifications
3. Field qualify the LCBRS demonstrating ability to comply with the specifications
4. Obtain authorization

For compressive strength testing, prepare 6 cylinders under California Test 540. Test specimens must be 6 by 12 inches. As an alternative to rodding, a vibrator may be used under California Test 524. Test cylinders under California Test 521 and perform 3 tests with each test consisting of 2 cylinders. The test result is the average from the 2 cylinders.

28-4.01D(2) Field Qualification

Proposed mix proportions must be field qualified before you place LCBRS. The technician performing the field test must hold current American Concrete Institute (ACI) certification as a Concrete Field Testing Technician-Grade I.

Field qualification must comply with the following:

1. Test for compressive strength at opening age and 7 days of age
2. At opening age, the compressive strength for each test must be at least 180 psi and the average strength for the 3 tests must be at least 200 psi
3. At 7 days age, the compressive strength for each test must be at least 600 psi and the average strength for the 3 tests must be at least 725 psi

28-4.01D(3) Quality Control Testing

Perform sampling under California Test 125.

Testing laboratories and testing equipment must comply with the Department's Independent Assurance Program.

Perform quality control sampling, testing, and inspection throughout LCBRS production and placement. For LCBRS, your quality control testing and results must comply with the following table:

Quality Control Requirements

Quality characteristic	Test method	Minimum testing frequency	Requirement
Sand equivalent (min)	ASTM D 2419	1 per 500 cu yd, minimum 1 per day of production	71[a]
Aggregate gradation	ASTM C 136		Comply with section 28-2.02C
Air content (max, percent)[b]	ASTM C 231	1 per 4 hours of placement work, plus one in the last hour of placement work	4
Penetration[c] (inches)	ASTM C 360		0–2-1/2 nominal 3 maximum
Slump[c] (inches)	ASTM C 143		0–5 nominal 6 maximum
Compressive strength (min, psi at 7 days)	California Test 521		725
Compressive strength (min, psi at opening age)	California Test 521		200

[a] If aggregate is qualified under section 28-4.02D, subparagraph 2, the minimum is 18.
[b] If no single test in the first 5 air content tests exceeds 1-1/2 percent, no further air content tests are required.
[c] Test either penetration or slump

28-4.01D(4) Acceptance Criteria

LCBRS acceptance is based on compliance with the requirement for the quality characteristic shown in the following table:

LCBRS Acceptance Criteria Testing

Quality characteristic	Test method	Requirement
Compressive strength (min, psi at 7 days)	California Test 521[a]	725

[a]Cylinders made under California Test 540

28-4.02 MATERIALS

28-4.02A General

Not Used

28-4.02B Cement

Cement must comply with the requirements for RSC.

28-4.02C Chemical Admixtures

Chemical admixtures must comply with chemical admixtures for concrete except you may use Type E chemical admixture. You may submit a request to use citric acid or borax. Your request must include a request from the cement manufacturer and a test sample.

28-4.02D Aggregates

Aggregate must comply with either of the following:

1. Section 90-1.02C except aggregate grading must comply with the aggregate grading table in section 28-2.02C
2. Section 28-2.02C and the following:
   2.1. Qualify the aggregate for each proposed aggregate source and gradation
   2.2. Qualification tests include (1) sand equivalent and (2) average 7-day compressive strength under ASTM C 39 on 3 specimens manufactured under ASTM C 192. The cement content for this test must be 300 lb/cu yd, and the 7-day compressive strength must be at least 610 psi. Cement must be Type II portland cement under section 90-1.02B(2) without SCM.

28-4.03 CONSTRUCTION

28-4.03A General

Construct LCBRS under section 28-2.03 except (1) section 28-2.03A does not apply and (2) the 4th through 6th paragraphs of section 28-2.03D do not apply.

Do not open the LCBRS to traffic before opening age.

Subsequent paving operations may begin only after final set time of LCBRS and it must have a compressive strength of at least 450 psi under California Test 521 before:

1. Placing HMA
2. Placing base
3. Operating equipment on the LCBRS

28-4.03B Proportioning, Mixing, and Transporting

For batches 1 cu yd or more, comply with one of the following methods:

1. Batch the ingredients at a central batch plant and charge them into a mixer truck for transportation to the pour site.
2. Batch the ingredients except the cement at a central batch plant and charge them into a mixer truck for transportation to a cement silo and weigh system, which must proportion cement for charging into the mixer truck.
3. Batch ingredients except the cement at a central batch plant and charge them into a mixer truck for transportation to a location where preweighed containerized cement is added to the mixer truck. The cement preweighing operation must utilize a platform scale. The platform scale must have a maximum capacity of 2.75 tons with a maximum graduation size of 1 lb. Preweigh cement into a fabric container. The minimum amount of cement to be proportioned into any single container must be 1/2 of the total amount required for the load of LCBRS being produced.
4. Proportion cement, water, and aggregate volumetrically under ASTM C 685 or section 90-3.02B.

28-4.03C Spreading, Compacting and Shaping

You may use metal or wood side forms. Wood side forms must be at least 1-1/2 inches thick.

After you deposit the LCBRS on the subgrade, consolidate it with high-frequency internal vibrators. Consolidate adjacent to forms and across the full pavement width. Place LCBRS as nearly as possible to its final position.

Spread and shape LCBRS with powered finishing machines supplemented by hand finishing.

After you place LCBRS, do not add water to the surface to facilitate finishing. Use surface finishing additives as recommended by the manufacturer of the cement after their use is authorized.

28-4.04 PAYMENT

Lean concrete base rapid setting is measured from the dimensions shown.

If volumetric proportioning is used and calibration is performed more than 100 miles from the project limits, the Department deducts $1,000 for each calibration session.

Replace section 28-4.02D with:

28-4.02D Aggregates

Aggregate must comply with either of the following:

1. Section 90-1.02C except aggregate grading must comply with the aggregate grading in the following table
2. Section 28-2.02 and the following:
   - 2.1. Section 28-2.01B does not apply
   - 2.2. Perform California Test 548 except part I

2.3 Comply with the aggregate grading in the following table:

Aggregate Grading

Sieve sizes	Percentage passing	
	3/8 inch maximum	No. 4 maximum
1/2"	100	--
3/8"	83-100	100
No. 4	65-82	62-85
No. 8	45-64	45-67
No. 16	27-48	29-50
No. 30	12-30	16-36
No. 50	6-17	5-20
No. 100	0-7	0-7
No. 200	0-3	0-3

Choose either the 3/8 inch maximum grading or the no. 4 maximum grading.

Replace "Reserved" in section 28-5 of the RSS for section 28 with:

28-5.01 GENERAL

28-5.01A Summary

Section 28-5 includes specifications for constructing concrete base.

28-5.01B Quality Control and Assurance

28-5.01B(1) General

For each lot of concrete for concrete base:

1. Quantity must not exceed 1,000 cu yd
2. The Department determines the modulus of rupture of test beams aged 28 days under California Test 523
3. The Department calculates the modulus of rupture by averaging the individual test results of 2 beams aged for 28 days

28-5.01B(2) Acceptance Criteria

Concrete base acceptance is based on modulus of rupture

Acceptance for modulus of rupture is on a lot basis. The modulus of rupture determined under California Test 523 for each lot must be at least 570 psi at 28 days.

The Department provides molds and machines for modulus of rupture acceptance testing. Provide material and labor the Engineer may require.

28-5.02 MATERIALS

28-5.02A General

Not Used

28-5.02B Portland Cement Concrete

Concrete must be PCC.

For the combined aggregate grading, the difference between the percent passing the 3/8-inch sieve and the percent passing the no. 8 sieve must be at least 16 percent of the total aggregate.

The specifications for reduction in operating range and contract compliance for cleanness value and sand equivalent for coarse aggregate in section 90-1.02C(2), and fine aggregate in section 90-1.02C(3) do not apply to concrete base.

Concrete base must contain not less than 550 lb of cementitious material per cubic yard.

28-5.02C Curing Seal

Asphaltic emulsion must be Grade RS1 or SS1.

28-5.03 CONSTRUCTION

28-5.03A General

Aggregate and bulk cementitious material must be proportioned by weight by means of automatic proportioning devices of authorized types.

28-5.03B Subgrade Preparation

Immediately before placing concrete base, the subgrade to receive the base must comply with the specified compaction and elevation tolerances and the following:

1. Free of loose and extraneous material
2. Uniformly moist, but free of standing or flowing water

28-5.03C Placing, Spreading, and Shaping

You may place concrete base in 2-lane monolithic segments. A longitudinal joint is not required in the center of this concrete base.

Place concrete base under section 40-1.03H(1) except the 4th paragraph does not apply.

The concrete base finished surface must not vary more than 1/4 inch from the bottom of a 12-foot long straightedge placed parallel with the center line.

Broom the concrete base surface to produce a uniform, rough surface. Do not broom the surface until the concrete base has set sufficiently.

Begin curing seal work as soon as free water leaves the concrete base surface and the surface is finished, but not later than 3 hours after placing.

Cure concrete base by applying asphaltic emulsion under section 94. Apply curing seal at a rate from 0.15 to 0.25 gal per sq yd. The Engineer determines the exact application rate. If you damage applied curing seal, immediately cover the damaged area with additional curing seal. Leave the curing seal on the concrete base for when surfacing is placed.

Construct transverse contraction joints at 30-foot intervals. Cut a groove in the concrete base with a power driven concrete saw. Grooves for longitudinal and transverse contraction joints must be the minimum width possible for the type of saw used. Immediately wash slurry from the joint with water. Water pressure must be less than 100 psi. Apply curing seal on cleaned contraction joints.

Saw transverse contraction joints before volunteer cracking occurs and after the concrete base is hard enough to saw without spalling, raveling, or tearing.

Concrete base must be within 0.05 foot of the grade established be the Engineer. Remove and replace hardened concrete base with a surface higher than 0.05-foot above the grade established by the Engineer with concrete base, or if approved, high areas may be ground until the surface of concrete base conforms to the tolerances specified. Grinding equipment must be equipped with diamond carborundum blades. Clean the ground area of concrete base that is to be covered with pavement of all foreign material and grinding residue as soon as any free water has left the surface and apply curing seal.

Where the surface of concrete base is lower than 0.05 foot from the grade established by the Engineer, remove and replace it with concrete base or, if authorized, fill low areas with HMA that complies with the specifications for the lowest layer of pavement. Do not fill low areas concurrently with the paving operation.

If you do not place HMA within 24 hours of applying curing seal, apply more asphaltic emulsion at a residual binder rate from 0.03 to 0.05 gal/sq yd immediately before placing HMA.

28-5.04 PAYMENT

Concrete base is measured from the dimensions shown.

Curing seal is measured under section 94. The amount of curing seal used is determined from the gauge on the curing equipment.

Replace "Reserved" in section 28-6 of the RSS for section 28 with:

28-6 ROLLER COMPACTED CONCRETE BASE

28-6.01 GENERAL

28-6.01A Summary

Section 28-6 Roller Compacted Concrete Base includes specifications for placing and curing RCC base.

28-6.01B Definitions

high density paver: a paver equipped with a high density screed capable of placing the RCC material to a minimum of 88 percent relative density based on wet density.

28-6.01C Submittals

28-6.01C(1) General

Reserved

28-6.01C(2) Mix Design

At least 10 days before starting field qualification, submit the proposed RCC base mix proportions, the corresponding mix identifications, and laboratory test reports including the compressive strength for each trial mixture at 10, 21, 28, and 42 days. If the mix proportions qualify for 56 days to attain the strength under 90-1.01D(5)a, submit the strength test results for 56 days.

28-6.01C(3) Quality Control Plan

Submit a RCC base QC plan. Allow 30 days for the Department's review. You or the Engineer may request a QC plan meeting.

The QC plan must comply with section 40-1.01D(4) except do not include bar reinforcement, dowel bars, tie bars, joint seals, and corrective action to be taken if penetration or air content measurements are outside action or suspension limits.

28-6.01C(4) Field Qualification

Submit field qualification data and test reports including:

1. Mixing date
2. Mixing equipment and procedures used
3. Volume produced in cubic yards
4. Type and source of ingredients used
5. Age and strength at time of testing

Field qualification test reports must be certified with a signature by the official in responsible charge of the laboratory performing the tests.

28-6.01D Quality Control and Assurance

28-6.01D(1) General

For QC testing use personnel qualified under the Department's Independent Assurance Program who are employed by an authorized laboratory.

Stop RCC base activities and immediately notify the Engineer whenever:

1. Any quality control or acceptance test result does not comply with the specifications
2. Visual inspection shows noncompliant RCC base

If RCC base activities are stopped, before resuming activities:

1. Inform the Engineer of the adjustments you will make
2. Remedy or replace the noncompliant RCC base
3. Obtain authorization

28-6.01D(2) Mix Design

Class A RCC base must contain at least 340 pounds of cementitious material per cubic yard.

Class B RCC base must contain at least 255 pounds of cementitious material per cubic yard.

Your laboratory must determine the cementitious materials content to ensure compliance with the specifications.

For Class A RCC base, the compressive strength of cylinders made under ASTM C 1435 and tested under California Test 521 used to determine the mixture proportions must be at least 2,800 psi at 28 days age and at least 3,600 psi at 42 days age. If the mix proportions qualify the mix to allow 56 days to attain the compressive strength under 90-1.01D(5)a, the compressive strength must be at least 2,400 psi at 28 days and 3,600 psi at 56 days.

For Class B RCC base, the compressive strength of cylinders made under ASTM C 1435 and tested under California Test 521 used to determine the mixture proportions must be at least 1,500 psi at 28 days age and at least 2,100 psi at 42 days age. If the mix proportions qualify the mix to allow 56 days to attain the compressive strength under 90-1.01D(5)a, the compressive strength must be at least 1,250 psi at 28 days and 2,100 psi at 56 days.

If changing an aggregate supply source or the mix proportions, produce a trial batch and field-qualify the new RCC base. The Engineer does not adjust contract time for performing sampling, testing, and qualifying new mix proportions or changing an aggregate supply source.

28-6.01D(3) Field Qualification

Proposed proportions must be field qualified before you place the RCC base represented by those proportions. Use an American Concrete Institute (ACI) certified "Concrete Laboratory Technician, Grade I" to perform field qualification tests and calculations.

Notify the Engineer at least 5 days before field qualification. Perform field qualification within the job site or a location authorized by the Engineer.

For field qualification fabric five test cylinders for each test age. Comply with the following:

1. Test cylinders at 10, 21, 28 days and the maximum age allowed.
2. For Class A RCC base each specimen must either:
    - 2.1 Achieve 2400 psi at 28 days or less
    - 2.2 Achieve 3600 psi at the maximum age allowed or less.
3. For Class B RCC base each specimen must either:
    - 3.1 Achieve 1250 psi at 28 days or less
    - 3.2 Achieve 2100 psi at the maximum age allowed or less.

28-6.01D(4) Quality Control Testing

Perform sampling under California Test 125.

Perform quality control sampling, testing, and inspection throughout RCC base production and placement. Notify the Engineer at least 2 business days before any sampling and testing. Notify the Engineer of testing results within 15 minutes of testing completion. Record inspection, sampling, and testing on the forms accepted with the QC plan and submit them within 48 hours of completion of each day of production. Perform testing and sampling for the quality characteristics shown in the following table:

Quality Control Testing

Test	Frequency	Test method	Requirement
Cleanness value	2 per day	CT 227	Section 90-1
Sand equivalent	2 per day	CT 217	
Aggregate gradation	2 per day	CT 202	
Relative compaction (%, min)	4 per day	CT 231	98

28-6.01D(5) Acceptance Criteria

RCC base is accepted based on the Department's testing for the quality characteristics shown in the following table:

Concrete Base Acceptance Criteria

Quality characteristic	Requirement		Test
Compressive strength at 28 days (psi, min) [a, b]	Class A	Class B	ASTM C 1435; CT 521
	2,800	1,500	
Compressive strength at max age allowed (psi, min) [a, b]	3,600	2,100	
Air content [c, d]	Section 90-1		CT 504
Relative Compaction (%, min) [a]	98		CT 231

[a] A single test represents no more than 1,000 cu yd
[b] The Engineer calculates compressive strength by averaging the individual test results of 2 cylinders. The difference in the individual test results of two cylinders the same age must not exceed 10 percent of the average.
[c] If air entrainment is specified or optionally used, air content tests must be performed.
[d] A single test represents no more than one day's paving.

Other base quality characteristics are considered in determining final acceptance. Acceptance of strength, thickness, placement, smoothness, and air content, does not constitute final base acceptance.

28-6.02 MATERIALS

RCC base must comply with section 90-8.

Choose a maximum aggregate size that is less than one third of the lift thickness you are placing.

Aggregate for Class A RCC base may contain crushed portland cement concrete.

Aggregate for Class B RCC base may contain crushed portland cement concrete and crushed asphalt concrete coarse aggregate. The maximum amount of crushed asphalt concrete coarse aggregate is 25 percent of the mass of the total aggregate.

Crushed asphalt concrete coarse aggregate must be:

1. 100 percent passing the 5/8 inch sieve and no more than 20 percent passing the No. 4 sieve

2. No more than 10 percent of the particles may be an agglomeration of smaller mineral particles.

28-6.03 CONSTRUCTION

28-6.03A General

Reserved

28-6.03B Subgrade

Immediately before placing RCC base, the subgrade must:

1. Comply with the specified compaction and elevation tolerance for the material involved
2. Be free from loose or extraneous material
3. Be uniformly moist

Areas of subgrade lower than the grade established by the Engineer must be filled with RCC base. RCC base used to fill low areas of subgrade is not included in the quantity of RCC base for payment.

28-6.03C Construction Joints

Construct transverse construction joints by placing and compacting the concrete beyond the joint location a sufficient distance so the base is full thickness at the construction joint location. Saw cut the RCC base at the construction joint location full depth and remove excess concrete.

Construct longitudinal construction joints by sawing the older concrete full depth and removing the edge. If the older concrete is existing or new LCB, JPCP or CRCP cut as close to the edge as possible to create a vertical edge. If the older concrete is RCC, cut 1.2 times the concrete thickness from the edge.

28-6.03D Placing RCC Base

Place RCC base using equipment specifically designed or adapted for placing RCC. Compact RCC base with steel tired rollers, rubber tired rollers or a combination of both. The minimum compacted thickness of a lift is 0.33 ft. The maximum thickness of a lift is 0.50 ft unless a high density paving machine is used and then the maximum is 0.77 ft compacted thickness.

When RCC base is placed in multiple lifts, place the subsequent lift on compacted RCC base that was placed less than 45 minutes before. Keep the surfaces of the prior lift damp until it is covered.

Place RCC base adjacent to previously placed RCC base within 60 minutes of mixing the previously placed concrete or construct a construction joint. Keep the previously placed RCC base surface damp until it is covered or cured.

Unless shown or specified, construct RCC base in minimum widths of 12 feet separated by construction joints. For RCC base constructed monolithically in widths greater than 60 times the thickness, construct longitudinal contraction joints no less than 12 feet apart and no more than 60 times the RCC base thickness apart.

Longitudinal contraction joints must be constructed with a power driven saw with a diamond blade.

For RCC base to be paved with concrete pavement, construct longitudinal construction and contraction joints in RCC base to provide at least 1 foot horizontal clearance from planned longitudinal construction and contraction joints in the concrete pavement.

Do not mix or place RCC base when the atmospheric temperature is below 35 degrees F.

28-6.03E Finishing

Finishing must produce a smooth and true-to-grade finish.

For RCC base to be paved with concrete pavement produce a finish free from cracks or fissures. If allowed, you may apply cement slurry to fill cracks and fissures and remove all slurry from the surface. If you use geotextile bond breaker filling cracks and fissures less than 1/4 inch wide is optional.

Complete finishing activities during daylight unless lighting is authorized.

For RCC base to be paved with HMA, the finished surface must not vary more than 0.05 foot from the grade established by the Engineer.

For RCC base to be paved with concrete pavement, the finished surface must not be above the grade, or more than 0.05 foot below the grade established by the Engineer.

28-6.03F Curing

Cure the RCC base exposed surfaces under section 90-1.03B< Curing Concrete > using the curing compound method or waterproof membrane method. If using the curing compound method use curing compound 1, 2 or 3. If the base is to be paved with HMA do not use curing compound 3.

Apply curing compound with mechanical sprayers in two applications. Complete the first application at a nominal rate of 150 sq ft/gal. Complete the second application at a nominal rate of 300 sq ft/gal with the nozzles offset 50 percent of the nozzle spacing form the first application, except if curing compound 3 is used the second application must be at a rate of 150 sq ft/gal. Reapply curing compound to saw cuts and disturbed areas.

If the RCC base will be used for traffic before paving, allow 3 days of curing before allowing traffic on RCC base.

28-6.03G Surfaces Not Within Tolerance

Where RCC base will be paved with concrete pavement, grind the surface with either a diamond or carborundum blade to within tolerance. After grinding RCC base to be paved with concrete pavement and after all free water has left the surface, clean foreign material and grinding residue from the surface. Apply curing compound to the ground area at a rate of approximately 150 ft sq/gal.

Where the surface of RCC base is lower than 0.05 foot from the grade established by the Engineer, fill low areas according to the pavement material as follows:

1. For HMA pavement, fill low areas with HMA that complies with the specifications for the lowest layer of pavement. Do not fill low areas concurrently with the paving operation.
2. For concrete pavement, fill low areas with pavement concrete concurrent with the paving operation.

28-6.04 PAYMENT

The volume for the payment of roller compacted concrete base is calculated based on the dimensions shown.

Replace "Reserved" in section 28-8 of the RSS for section 28 with:

28-8 ALTERNATE TREATED BASE

28-8.01 General

28-8.01A(1) Summary

Section 28-5 includes specifications for placing treated base using any of the following:

1. LCB under section 28-2
2. Roller compacted concrete base under section 28-6
3. Lean concrete base rapid setting under section 28-4

28-8.01A(2) Definitions

Not Used

28-8.01A(3) Submittals

Not Used

28-8.01A(4) Quality Control and Assurance

Not Used

28-8.02 Materials

Not Used

28-8.03 Construction

28-8.03A General

Whenever placing alternate concrete base beneath concrete pavement, place base bond breaker between the base and pavement. Base bond breaker must comply with section 36-2.

28-8.03B HMA Type A

Not Used

28-8.04 Payment

Alternate treated base is measured by the cubic yard from the dimensions shown. Increased quantities of aggregate base or aggregate subbase resulting from the alternate treated base selected are not paid for.

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30–36 RESERVED

Replace "Reserved" in the RSS for section 36-2 with:

36-2.01 GENERAL

36-2.01A Summary

Section 36-2 includes specifications for placing base bond breaker.

36-2.01B Definitions

Not Used

36-2.01C Submittals

Submit a certificate of compliance for each lot of base bond breaker material delivered.

36-2.01D Quality Control and Assurance

Not Used

36-2.02 MATERIALS

Base bond breaker must comply with the specifications shown in the following table:

Base bond breaker no.	Specification
1	PG asphalt binder, Grade PG 64-10
2	Curing compound no. 3
3	White opaque polyethylene film under ASTM C 171 except the minimum thickness must be 6 mils
4	White curing paper under ASTM C 171
5	Geosynthetic bond breaker

Use base bond breaker no. 1, 2, or 5 on concrete base and LCB.

Use base bond breaker no. 3, 4, or 5 on lean concrete base rapid setting, CTPB, and rapid strength concrete base.

36-2.03 CONSTRUCTION

36-2.03A General

Before placing base bond breaker, remove foreign and loose materials from the base.

Do not place base bond breaker until the base has cured. Allow base bond breaker nos. 1 and 2 to cure before paving.

Within 72 hours of placing base bond breaker, pave over the base bond breaker.

36-2.03B Placing Base Bond Breaker

Place base bond breaker no. 1 in one application at a uniform rate from 0.09 to 0.15 gal/sq yd over the entire base surface area. Allow at least 4 hours for curing.

Comply with section 90-1.03B(3)(c) for mixing base bond breaker no. 2. Place base bond breaker no. 2 in one or more applications to achieve a coverage rate of at least 0.12 gal/sq yd over the entire base surface area. Allow at least 4 hours for curing.

Place base bond breakers no. 3 and 4 without wrinkles. Overlap adjacent sheets a minimum of 6 inches and in the same direction as the concrete pour. Tape or bond the sheets together as needed to prevent sheets from folding or wrinkling. Secure the bond breaker sufficiently so that it remains in place during concrete pavement placement. Ensure that no concrete gets under the bond breaker.

Place base bond breaker no. 5 without wrinkles. Overlap adjacent sheets a minimum of 8 inches in the same direction as the concrete pour. Overlap no more than 3 layers at any location. Secure base bond breaker to the base with pins or nails punched through galvanized washers or discs 2 to 2.75-inches in diameter. Place fasteners less than 3 ft apart along edges and 6 ft apart elsewhere. If bond breaker moves or wrinkles while placing concrete pavement, increase the frequency of fasteners. Ensure that no concrete gets under the bond breaker.

36-2.04 PAYMENT

The Department does not pay for overlaps of base bond breaker material.

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The HFST QCP must include:

1. Schedule for the trial HFST work and the production HFST work
2. Description of equipment for placing HFST
3. Description of equipment for measuring, mixing, placing, and finishing HFST
4. Method for protecting areas not to receive HFST
5. Cure time estimates for HFST
6. Storage and handling of HFST components
7. Disposal of excess HFST and containers
8. Contingency plan for possible failure during the HFST application
9. Name of the certified independent testing laboratory

Have 1 quart of resin binder and calcined bauxite aggregate topping tested at a certified independent testing laboratory and then furnish the verifications to the Engineer that the materials meet all requirements specified.

Submit the following two working days before use:

1. An MSDS for each shipment of HFST
2. Certified independent testing laboratory test results for the resin binder and calcined bauxite aggregate topping

Do not begin a trial HFST until authorized in writing.

37-6.01D Quality Control and Assurance

Complete a trial of HFST at an approved location on asphalt concrete pavement before starting HFST production work.

The trial HFST must:

1. Be at least 12 feet wide by 20 feet long
2. Be constructed using the same equipment as the production work
3. Replicate field conditions, including ambient and surface temperatures, anticipated for the production work
4. Demonstrate surface preparation requirements
5. Remove pavement markers and delineation within the area to receive HFST, for the lane and length involved, prior to placing resin binder
6. Document the settings on the applicator equipment, initial quantities of resin and aggregate topping, and unused quantities of resin and aggregate topping remaining in the applicator equipment after applying the HFST
7. Determine the initial set time for the resin binder
8. Test the coefficient of friction using ASTM E1911 on:
   8.1. The existing surface
   8.2. The trial HFST

Do not begin production HFST until authorized in writing after successful completion of the trial HFST.

Take a 1-quart sample of the resin binder from the trial HFST and once during production work. Test in accordance with ASTM D638 by a certified independent testing laboratory. Provide the test results within 5 days after taking the sample.

If the trial HFST coefficient of friction is below 0.75, you must correct or replace the trial HFST until the coefficient of friction is greater than or equal to 0.75.

37-6.02 Materials

37-6.02A General

Reserved

37-6.02B Polymer Resin Binder

A two-part exothermic polymer resin binder which holds the aggregate topping firmly in place, and meets the requirements shown in the following table:

Polymer Resin Binder Requirements

Property	Requirement	Test Method
Ultimate Tensile Strength	2650 psi min.	ASTM D638
Elongation at break point	30% min.	ASTM D638
Compressive Strength	1600 psi min.	ASTM D695
Water Absorption	1.0 % max.	ASTM D570
Shore D Hardness, min. 77ºF	65-75	ASTM D2240
Viscosity	1000-3000 Pa	ASTM D2393
Flexural Yield Strength, min psi	2000	ASTM D790
Cure Rate	3 hrs. max.	ASTM D1640, 0.2" thickness
Mixing Ratio	As recommended by the manufacturer	N/A

37-6.02C Epoxy Resin Binder

A two-part exothermic epoxy resin binder which holds the aggregate topping firmly in place, and meets the requirements shown in the following table:

Epoxy Resin Binder Requirements

Property	Requirement	Test Method
Ultimate Tensile Strength	2650 psi min	ASTM D638
Elongation at break point	30% min	ASTM D638
Compressive Strength	1600 psi min	ASTM D695
Water Absorption	1.0% max	ASTM D570
Shore D Hardness, min 77ºF	65-75	ASTM D2240
Gel Time, minutes	15-45 min	ASTM C881
Flexural Yield Strength, min psi	2000	ASTM D790
Cure Rate	3 hrs. max	ASTM D1640, 0.2" thickness
Mixing Ratio	As recommended by the manufacturer	N/A

37-6.02D Aggregate Topping

A blend of calcined bauxite aggregate. The aggregate topping is to be clean, dry, and free from deleterious matter. The aggregate topping must meet the requirements shown in the following table:

Aggregate Topping Requirements

Property	Test Method	Requirement
Aggregate Grading	CTM 202	No. 6 Percentage Passing 95% min No.16 Percentage Passing 5% max
Aggregate Abrasion Value Loss at 100 rev.	CTM 211	10% max.
Aggregate Acid Insolubility	ASTM D3042	Greater than 90%
Aggregate Magnesium Soundness	ASTM C88	30% max

37-6.03 Pre-construction

37-6.03A General

Reserved

37-6.03 Pre-construction Conference

Schedule a pre-construction conference with the Engineer at a mutually agreed time and place. Make the arrangements for the conference facility. Be prepared to discuss the trial HFST requirements.

Attendance at the pre-construction conference is mandatory for:

1. HFST supplier
2. HFST Foreman
3. Project Superintendent

37-6.04 Construction

37-6.04A General

Attendance during construction activities is mandatory for:

1. HFST supplier
2. HFST Foreman
3. Project Superintendent

Surfaces must be clean, dry, and free of all dust, oil, debris and any other material that might interfere with the bond between the resin binder material and existing surfaces.

Remove pavement markers and delineation to a maximum depth of 0.01 foot within the area to receive HFST before placing resin binder.

Perform street sweeping before placing pavement markers and delineation.

Pre-treat cracks greater than 0.25 inch width with mixed resin binder. Place HFST after mixed resin binder has gelled.

The HFST must conform to the following:

1. Apply HFST to asphalt pavement surfaces that are greater than 30 days old
2. Apply resin binder to a clean dry surface above 55°F and when anticipated weather conditions allow proper application of the surface treatment
3. Apply HFST under the resin binder manufacturer's recommendations
4. The minimum spread rate for resin binder is 0.32 gal/sq yd
5. The minimum spread rate of retained aggregate is 15 lb/sq yd.
6. HSFT must be allowed to cure for the minimum duration as recommended by the supplier and during that time the application area must be closed to all vehicle and Contractor equipment traffic
7. The surface texture of the HFST must be uniform and have a coefficient of friction not less than 0.75 as tested by ASTM E1911.
8. Provide a correction plan for HFST that fails to meet the coefficient of friction of 0.75. The correction plan may include correction of existing HFST or remove-and-replace.

Test the coefficient of friction before opening HFST areas to public traffic. Submit results the same day the tests are performed.

Protect utilities, drainage structures, curbs, and any other structures within or adjacent to the treatment location from HFST materials.

37-6.04B Mixing and Application Methods

37-6.04B(1) General

Utilize one of the following methods to apply the resin binder and aggregate wearing course, under manufacturer’s recommendations:

1. Automated continuous application
2. Mechanical mixing and aggregate broadcast
3. Hand mixing and application

Application 2 must not be used on areas with a slope of greater than 2.5 percent in any direction.

Application 3 must not be used on areas greater than 200 square yards.

37-6.04B(2) Automated Continuous Application

Automated continuous application must be performed by an applicator vehicle with a minimum aggregate capacity of 30,000 lbs and a minimum of 1200 gallons of the resin binder. The applicator must continuously mix, meter, monitor and apply the resin binder and high friction aggregate in one continuous pass as one layer. Apply additional layers within 4 hours of the previous cured layer.

The applicator vehicle must be equipped with a built-in data management unit which is capable of producing real time data flow showing:

1. The volume of resin
2. The resin mil thickness on average throughout the application width
3. The volume of aggregate applied throughout the application width

The automated continuous application vehicle must have continuous pumping and proportioning devices that blend the binder within a controlled system. The binder must be blended and mixed to the ratio under the manufacturer's recommendations(+/- 2 percent by volume). The binder must be continuously applied once blended. The application vehicle must be capable of applying the minimum binder spread rate. Clean the mix head and delivery lines if application of the mixed resin is stopped for more than 30 minutes.

The high friction aggregate must be applied by the same automated continuous application vehicle that applies the resin binder to the pavement section. The automatic aggregate spreader must be capable of applying up to a continuous 12-foot width application.

Exposed uncured mixed resin binder must not be visible after the aggregate topping is applied. If exposed uncured mixed resin binder is visible after applying the aggregate topping, apply aggregate topping to the exposed areas using an approved application method.

Do not allow the mixed resin binder to do any of the following that may impair retention and bonding of the aggregate topping:

1. Separate
2. Cure
3. Dry
4. Be exposed
5. Harden

Do not contaminate the exposed uncured mixed resin binder by any form of contact including:

1. Walking
2. Standing

Replace contaminated sections of resin.

37-6.04B(3) Mechanical Mixing and Aggregate Broadcast

Mix resin binder with equipment capable of mixing, metering, monitoring and distributing while maintaining the designed, proper stoichiometric ratio. Uniformly spread the resin binder onto the surface using a serrated edge squeegee. Mechanically apply the high friction surface aggregate.

Any exposed areas of wet resin must be covered by hand immediately before the binder gells.

37-6.04B(4) Hand Mixing and Application

For approved low volume areas and areas less than 200 square yards, hand-mix the resin binder under manufacturer's recommendations. Uniformly spread the resin binder onto the surface using a serrated edge squeegee. Immediately broadcast the high friction surfacing aggregates until refusal.

37-6.04C Excess Aggregate Removal and Reuse

The excess aggregate may be reused. The aggregate must be reclaimed by a mechanical sweeper. The recovered aggregate must be clean, uncontaminated and dry.

Excess and loose aggregate must be removed from the traveled way and shoulders by street sweeping. Application of HFST on highway ramps requires a second street sweeping 24-48 hours after application on the ramp before placement of pavement markers and delineation.

^^

39 HOT MIX ASPHALT

Replace section 39 with:

39 HOT MIX ASPHALT, SUPERPAVE

39-1.01 GENERAL

39-1.01A Summary

Section 39 includes specifications for producing and placing HMA by mixing aggregate and asphalt binder at a mixing plant and spreading and compacting the HMA mixture.

Section 39 includes specifications for producing and placing WMA by mixing aggregate, asphalt binder and a warm mix technology at a mixing plant and spreading and compacting the WMA mixture.

You may produce HMA-SP (Type A) or RHMA-SP-G using an authorized warm mix asphalt (WMA) technology. For Department-authorized WMA technologies, go to the METS Web site

HMA includes one or more of the following types:

1. HMA, superpave (HMA-SP) Type A
2. RHMA, superpave, gap graded (RHMA-SP-G)
3. OGFC including HMA-O, and RHMA-O

The RSSs for section 39 do not apply

39-1.01B Definitions

coarse aggregate: Aggregate retained on a 1/4-inch screen

fine aggregate: Aggregate passing the 1/4-inch screen.

leveling course: Thin layer of HMA used to correct minor variations in the longitudinal and transverse profile of the pavement before placement of other pavement layers.

lower course: HMA-SP (Type A) layers below 0.2 feet from finished grade

miscellaneous areas: Areas outside the traveled way such as:

1. Median areas not including inside shoulders
2. Island areas
3. Sidewalks
4. Gutters
5. Gutter flares
6. Ditches
7. Overside drains
8. Aprons at the ends of drainage structures

modified binder: PG graded binder designated as polymer modified (PM) or terminal blend (TR), or any PG grade binder with rubber modifiers.

processed RAP: RAP that has been fractioned.

substitution rate: Amount of RAP aggregate substituted for virgin aggregate in percent.

supplemental fine aggregate: Aggregate passing the no. 30 sieve, including hydrated lime, portland cement, and fines from dust collectors.

surface course: Upper 0.2 feet of HMA-SP (Type A) exclusive of HMA-O or RHMA-O.

top layer: Final riding surface exclusive of HMA-O or RHMA-O.

WMA: HMA produced at temperatures no greater than 275 degrees F.

HMA with WMA technology: HMA produced using additives to aid with mixing and compaction of HMA produced at temperatures greater than 275 degrees F.

39-1.01C Submittals

39-1.01C(1) General

For miscellaneous areas and dikes, a JMF submittal is not required. For OGFC, submit a complete JMF submittal, except for asphalt binder content. The Department determines the asphalt binder content under California Test 368 within 20 days of your complete JMF submittal and provides you a *Caltrans Hot Mix Asphalt Verification* form.

For JMF mix design, JMF verification, production start-up, and each 10,000 tons, submit AASHTO T 283 and AASHTO T 324 (Modified) test results to the Engineer and electronically to:

Moisture_Tests@dot.ca.gov

At production start-up and within 1000 tons of the halfway point of production of HMA, submit samples split from your HMA production sample for AASHTO T 283 and AASHTO T 324 (Modified) tests to the Engineer and the Transportation Laboratory, Attention: Moisture Test.

Submit all completed quality control test results within 3 business days of a request. Submit all quality control tests except AASHTO T 283 within 7 days of a request. Submit AASHTO T 283 quality control tests within 14 days of a request.

For tests performed under AASHTO T324 (Modified) as specified in section 39-1.01D(1), submit test data and 1 tested sample set within 3 business days of sampling.

Submit a new JMF if you change any of the following:

1. Target asphalt binder percentage greater than ±0.2 percent
2. Asphalt binder supplier
3. Asphalt rubber binder supplier
4. Component materials used in asphalt rubber binder or percentage of any component materials
5. Combined aggregate gradation
6. Aggregate sources
7. WMA
8. LAS producer
9. Any material in the JMF

Submit a new JMF when the average binder content in a new fractionated RAP stockpile is more than ± 2.0 percent from the average binder content of the original fractionated RAP stockpile used in the mix design.

Submit a new JMF when the processed RAP specific gravity is more than ± 0.060 from the average maximum specific gravity reported on page 4 of your *Contractor Hot Mix Asphalt Design Data* form.

39-1.01C(2) Job Mix Formula

39-1.01C(2)(a) General

For each type of HMA shown, submit your proposed JMF on the *Contractor Job Mix Formula Proposal* form along with:

1. Mix design documentation on *Hot Mix Asphalt Design Data* form dated within 12 months of submittal
3. JMF verification on a *Caltrans Hot Mix Asphalt Verification* form, if applicable
3. JMF renewal on a *Caltrans Job Mix Formula Renewal* form, if applicable
4. MSDS for:
   - 4.1. Asphalt binder
   - 4.2. Base asphalt binder used in asphalt rubber binder

4.3. CRM and asphalt modifier used in asphalt rubber binder
4.4. Blended asphalt rubber binder mixture
4.5. Supplemental fine aggregate except fines from dust collectors
4.6. Antistrip additives
4.7. WMA technology

5. For RHMA-G-SP, asphalt rubber binder design and profile. The JMF must be based on a HMA mix design determined as described in the *Superpave Mix Design* SP-2 Manual by the Asphalt Institute

39-1.01C(2)(b) Mix Design

The HMA mix design must comply with AASHTO R 35 except:

1. Notes 3, 6, and 10 do not apply
2. AASHTO M 323 does not apply on combinations of aggregate gradations and asphalt binder contents to determine the OBC and HMA mixture qualities

For HMA with WMA additive technology, produce HMA mix samples for your mix design using your methodology for inclusion of WMA admixture in laboratory produced HMA. For WMA water injection foam technology, the use of foamed asphalt for mix design is not required.

Your *Hot Mix Asphalt Design Data* form must show documentation on aggregate quality.

For HMA mixtures utilizing RAP the maximum binder replacement is 25.0 percent for surface course and 40.0 percent for lower courses.

For HMA with a binder replacement percent less than or equal to 25 percent of optimum binder content, you may request that the performance graded asphalt binder grade with upper and lower temperature classifications be reduced by 6 degrees C from the specified grade.

For HMA with a binder replacement greater than 25 percent of optimum binder content and less than or equal to 40 percent of optimum binder content, you must use a performance graded asphalt binder grade with upper and lower temperature classifications reduced by 6 degrees C from the specified grade.

The mix design must comply with the quality characteristics of the following table:

Hot Mix Asphalt Mix Design Requirements

Quality characteristic	Test method	HMA-SP	
		Type A	RHMA-SP-G
Air voids content (%)[a]	AASHTO T 269[a]	$N_{initial}$ >8.0 N_{design} = 4.0 N_{max} >2.0	N_{design} Specification
Air voids content (%)[a] 1" gradings	AASHTO T 269[a]	$N_{initial}$ >8.0 N_{design} = 5.0 N_{max} >2.0	--
Gyration Compaction (number of gyrations)	AASHTO T 312	$N_{initial}$ 8 N_{design} 85 N_{max} 130	N_{design} 50–150[b]
Voids in mineral aggregate (% min.) 1/4" grading 3/8" grading 1/2" grading 3/4" grading 1" grading with NMAS=1" with NMAS=3/4"	SP-2 Asphalt Mixtures Volumetrics[c]	 16.5-19.5 15.5-18.5 14.5-17.5 13.5-16.5 13.5-16.5 13.5-16.5 14.5-17.5	 -- -- 18.0–23.0 18.0–23.0
Dust proportion 1/4" and 3/8 gradings 1/2"and 3/4" gradings 1" grading	SP-2 Asphalt Mixtures Volumetrics[c]	 0.6–1.3 0.6–1.3 0.6–1.3	Report Only
Hamburg wheel track (minimum number of passes at 0.5 inch average rut depth) PG 58 PG 64 PG-70 PG-76 or higher	AASHTO T 324 (Modified) [d,e]	 10,000 15,000 20,000 25,000	 15,000 20,000 25,000
Hamburg wheel track (inflection point minimum number of passes) PG 58 PG 64 PG-70 PG-76 or higher	AASHTO T 324 (Modified) [d,e]	 10,000 10,000 12,500 15,000	 10,000 10,000 12,500
Moisture susceptibility (minimum dry strength, psi)	AASHTO T 283[d,f]	100	100
Moisture susceptibility (minimum wet strength, psi)	AASHTO T 283[d,f,g]	70	70

[a]Calculate the air voids content of each specimen using AASHTO T 275 to determine bulk specific gravity AASHTO T 209 Method A to determine theoretical maximum specific gravity. Under AASHTO T 209 use a digital monometer and pycnometer when performing AASHTO T 209.
[b]Superpave gyratory compactor ram pressure may be increased to a maximum of 825kPa, and specimens may be held at a constant height for a maximum of 90 minutes.
[c]Measure bulk specific gravity using AASHTO T 275.
[d]Test plant produced HMA.
[e]Test as specified in section 39-1.01D(1).
[f]Test specimens must be 100mm or 150mm gyratory compacted HMA
[g]Freeze thaw required

If the test results for AASHTO T 283 or AASHTO T 324 (Modified) for untreated plant produced HMA is less than minimum requirements for HMA-mix design, determine the plasticity index of the aggregate blend under California Test 204. The antistrip treatment must be based on plasticity index in compliance with the following table:

Hot Mix Asphalt Antistrip Treatment Options

Quality characteristic	Test method	Treatment requirement
Plasticity index Plasticity index from 4 to 10[a] Plasticity index less than 4	California Test 204	Dry hydrated lime with marination Lime slurry with marination Liquid Dry hydrated lime without marination Dry hydrated lime with marination Lime slurry with marination

[a]If the plasticity index is greater than 10, do not use that aggregate blend.

If the tensile strength test result for treated plant produced RHMA-SP-G is less than the RHMA-SP-G mix design requirement for tensile strength, the minimum tensile strength requirement is waived, but you must use any of the following antistrip treatments:

1. HMA aggregate lime treatment – slurry method
2. HMA aggregate lime treatment – dry lime method
3. Liquid antistrip treatment using 0.5 to 1.0 percent liquid antistrip

39-1.01C(2)(c) Job Mix Formula Document Review

Allow the Engineer 5 business days from a complete JMF submittal, for document review of the aggregate qualities, mix design, and JMF. Do not start HMA production before verification and acceptance of JMF.

39-1.01C(2)(d) Job Mix Formula Verification

If you cannot submit a Department-verified JMF on a *Caltrans Hot Mix Asphalt Verification* form dated within 12 months before HMA production, the Engineer verifies the JMF.

For OGFC, the Engineer determines the asphalt binder content under California Test 368 within 20 days of your complete JMF submittal and provides you a *Hot Mix Asphalt Verification* form.

HMA produced with WMA technology for JMF verification must be produced using the WMA technology shown in the JMF submittal.

Based on your testing and production experience, you may submit an adjusted aggregate gradation TV on a *Contractor Job Mix Formula Proposal* form before verification testing. Aggregate gradation TV must be within the TV limits specified in the aggregate gradation tables.

Asphalt binder set point for HMA verification must be the OBC specified on your *Contractor Hot Mix Asphalt Design Data* form. When RAP is used, asphalt binder set point for HMA must be:

$$\text{Asphalt Binder Set Point} = \frac{\dfrac{BC_{OBC}}{\left(1 - \dfrac{BC_{OBC}}{100}\right)} - R_{RAP}\left[\dfrac{BC_{RAP}}{\left(1 - \dfrac{BC_{RAP}}{100}\right)}\right]}{100 + \dfrac{BC_{OBC}}{\left(1 - \dfrac{BC_{OBC}}{100}\right)}}$$

Where:
BC_{OBC} = optimum asphalt binder content, percent based on total weight of mix
R_{RAP} = RAP ratio by weight of aggregate
BC_{RAP} = asphalt binder content of RAP, percent based on total weight of RAP mix

For HMA, the Engineer verifies the JMF from samples taken from HMA produced by the plant to be used. Notify the Engineer at least 2 business days before sampling materials.

In the Engineer's presence and from the same production run, take samples of:

1. Aggregate – Coarse, fine, and supplemental fine aggregate from cold feed belts, or hot bins. Samples must include at least 120 lbs for each coarse aggregate, 80 lbs for each fine aggregate, and 10 lbs for each type of supplemental fines. The Department combines these aggregate samples to comply with the JMF TVs submitted on a *Contractor Job Mix Formula Proposal* form.
2. Asphalt binder - 2 quarts minimum Asphalt rubber binder with the components blended in the proportions to be used. Samples must be in four 1-quart cylindrical-shaped cans with open top and friction lids.
3. RAP - From the RAP system. Samples must be at least 50 lbs minimum.
4. HMA - 250 lbs minimum.

Sample aggregate from cold feed belts or hot bins.

You may sample from a different project including a non-Department project if you make arrangements for the Engineer to be present during sampling.

For aggregate, RAP, and HMA, split the samples into at least 4 parts and label their containers. Submit 3 parts to the Engineer and use 1 part for your testing.

After completion of the JMF document review, the Engineer verifies each proposed JMF within 20 days of receiving all verification samples and the JMF document submittal has been accepted. If you request, the Engineer verifies RHMA-G quality requirements within 7 days of receiving all verification samples, and the JMF document submittal has been accepted. Verification is testing for compliance with the specifications for:

1. Aggregate quality
2. Aggregate gradation (JMF TV ± tolerance)
3. Asphalt binder content (JMF TV ± tolerance)
4. HMA quality specified in the table Hot Mix Asphalt Mix Design Requirements except:
   4.1. Air voids content (design value ± 1.5 percent)
   4.2. VMA (minimum HMA mix design requirement 12.5-15.5)
   4.3. Dust proportion (0.6-1.3)

To verify the JMF air voids content the Engineer uses an average of three briquettes for air voids content, VMA, and dust proportion. The Engineer tests plant produced material.

If the Engineer verifies the JMF, the Engineer provides you a *Hot Mix Asphalt Verification* form.

If the Engineer's tests on plant-produced samples do not verify the JMF, the Engineer notifies you and you must submit a new JMF or submit an adjusted JMF based on your testing. JMF adjustments may include a change in:

1. Asphalt binder content target value up to ±0.2 percent from the OBC value submitted on *Hot Mix Asphalt Design Data* form except do not adjust the target value for asphalt rubber binder for RHMA-G-SP below 7.5 percent by total weight of mixture.
2. Aggregate gradation target values within the target value limits specified in the aggregate gradation table.

You may adjust the JMF only once due to a failed verification test. An adjusted JMF requires a new *Contractor Job Mix Formula Proposal* form and *Hot Mix Asphalt Design Data* form and verification of a plant-produced sample.

The Engineer reverifies the JMF if HMA production has stopped for longer than 30 days and the verified JMF is less than 12 months old.

For each HMA type and aggregate size specified, the Engineer verifies up to 2 proposed JMF submittals including a JMF adjusted after verification failure. If you submit more than 2 JMF for each type of HMA and aggregate size, the Engineer deducts $3,000 from payments for each verification exceeding this limit. This deduction does not apply to verifications initiated by the Engineer or if a JMF expires while HMA production is stopped longer than 30 days.

If you have a verified *Hot Mix Asphalt Verification* form, the Engineer will verify 1 binder source change for each HMA type and aggregate size specified. The Engineer deducts $2,000 from payments for this verification. This deduction does not apply to verifications initiated by the Engineer or JMF renewal.

39-1.01C(2)(e) Job Mix Formula Renewal

You may request a JMF renewal by submitting:

1. Proposed JMF on a *Contractor Job Mix Formula Proposal* form
2. Previously verified JMF documented on a *Caltrans Hot Mix Asphalt Verification* form dated within 12 months
3. Mix design documentation on a *Contractor Hot Mix Asphalt Design Data* form used for the previously verified JMF

If the Engineer requests, sample the following materials in the presence of the Engineer and place in labeled containers weighing no more than 50 lbs each:

1. Coarse, fine, and supplemental fine aggregate from cold feed belts, or hot bins. Samples must include at least 120 lbs for each coarse aggregate, 80 lbs for each fine aggregate, and 10 lbs for each type of supplemental fines. The Department combines these aggregate samples to comply with the JMF TVs submitted on a *Contractor Job Mix Formula Proposal* form.
2. RAP from the RAP system. Samples must be at least 50 lbs.
3. Asphalt binder from the binder supplier. Samples must be in two 1-quart cylindrical-shaped cans with open top and friction lids.
4. Asphalt rubber binder with the components blended in the proportions to be used. Samples must be in four 1-quart cylindrical-shaped cans with open top and friction lids.

Notify the Engineer at least 2 business days before sampling materials. For aggregate, RAP, and HMA, split samples into at least 4 parts. Submit 3 parts to the Engineer and use 1 part for your testing.

Allow the Engineer 5 business days from a complete JMF re-verification submittal for document review of the aggregate qualities, mix design, and JMF. Do not start HMA production before re-verification and acceptance of JMF.

The most recent aggregate quality test results within the past 12 months may be used for verification of JMF renewal or the Engineer may perform aggregate quality tests for verification of JMF renewal.

The Engineer verifies the JMF under section 39-1.01C(2)(d) except:

1. Engineer retains samples until you provide test results for your part on a *Contractor Job Mix Formula Renewal* form.
2. Department tests samples of materials obtained from the HMA production unit after you submit test results that comply with the specifications for the quality characteristics in section 39-1.01C(2)(b).
3. After completion of the JMF reverification document review, the Engineer verifies each proposed JMF within Engineer verifies each proposed JMF within 30 days of receiving verification samples.
4. You may not adjust the JMF due to a failed verification.
5. For each HMA type and aggregate gradation specified, the Engineer verifies at the Department's expense 1 proposed JMF renewal within a 12-month period.

If the Engineer verifies the JMF renewal, the Engineer provides you a *Caltrans Hot Mix Asphalt Verification* form.

39-1.01C(2)(f) Job Mix Formula Acceptance

You may start HMA production if:

1. The Engineer's review of the JMF shows compliance with the specifications
2. The Department has verified the JMF within 12 months before HMA production
3. The Engineer accepts the verified JMF

39-1.01C(2)(g) Job Mix Formula Modification

For an accepted JMF, you may change asphalt binder source or liquid antistrip producer one time during production.

Submit your modified JMF request a minimum of 10 business days before production. Each modified JMF submittal must consist of:

1. Proposed modified JMF on *Contractor Job Mix Formula Proposal* form, marked "Modified."
2. Mix design records on *Contractor Hot Mix Asphalt Design Data* form for the accepted JMF to be modified.
3. JMF verification on *Hot Mix Asphalt Verification* form for the accepted JMF to be modified.
4. Quality characteristics test results for the modified JMF as specified in section 39-1.01C(2)(b). Perform tests at the mix design OBC as shown on the Contractor Asphalt Mix Design Data form.

With an accepted modified JMF submittal, the Engineer verifies each modified JMF within 7 business days of receiving all verification samples.

The Engineer verifies the modified JMF after the modified JMF HMA is placed on the project and verification samples are taken within the first 750 tons following sampling requirements in section 39-1.01C(2)(d). The Engineer tests verification samples for compliance with:

1. Hamburg Wheel Track as shown in the table titled "HMA Mix Design Requirements"
2. Air void content at design value ±1.5 percent
3. Voids in mineral aggregate as shown in the table titled "HMA Mix Design Requirements"
5. Dust proportion, as shown in the table titled "HMA Mix Design Requirements"

The Engineer may test for moisture susceptibility, as shown in the table titled "HMA Mix Design Requirements"

If the modified JMF is verified, the Engineer revises your *Hot Mix Asphalt Verification* form to include the new asphalt binder source or new liquid antistrip producer. Your revised form will have the same expiration date as the original form.

If a modified JMF is not verified, stop production and any HMA placed using the modified JMF is rejected.

The Engineer deducts $2,000 from payments for each modified JMF verification. The Engineer deducts an additional $2,000 for each modified JMF verification that the Engineer tests for moisture susceptibility.

39-1.01C(2)(h) Warm Mix Asphalt

If WMA technology is used, submit the following with your proposed JMF submittal:

1. MSDS for WMA technology
2. For WMA water injection foam technology:
   - 2.1. Name of technology
   - 2.2. Laboratory Procedure LP-12 test result for foamed bitumen expansion ratio dated within 12 months of submittal
   - 2.3. Laboratory Procedure LP-12 test result for foamed bitumen half-life dated within 12 months of submittal
   - 2.4. Optimum foaming water content
   - 2.5. Proposed HMA production temperature range
3. For WMA additive technology:
   - 3.1. Name of technology
   - 3.2. Percent admixture by weight of binder and percent admixture by total weight of HMA as recommended by the manufacturer
   - 3.3. Methodology for inclusion of admixture in laboratory-produced HMA

3.4. Proposed HMA production temperature range

The HMA PPC must produce an electronic log of production data consisting of a series of snapshots captured at a maximum of 1-minute intervals throughout daily production. Each snapshot of production data must be a register of production activity at that time and not a summation of the data over the preceding interval to the previous snapshot. The amount of material represented by each snapshot is the amount produced during the 0.5-minute interval before and the 0.5-minute interval after the capture time. Collect and hold data for the duration of the contract and submit the electronic media, daily and upon request. The snapshot of production data must include the following:

1. Date of production
2. Production location
3. Time of day the data is captured
4. HMA mix type being produced and target binder rate
5. HMA additive type, brand, and target rate
6. Temperature of the binder and HMA mixture
7. For a continuous mix operation, the rate of flow of the dry aggregate calculated from the wet aggregate flow rate as determined by the conveyor scale
8. For a continuous mix plant operation, the rate of flow of the asphalt meter
9. For a continuous mix plant operation, the rate of flow of HMA additive meter
10. For a batch plant operation, actual batch weights of all ingredients
11. Dry aggregate to binder ratio calculated from metered ingredient output
12. Dry aggregate to HMA additive ratio calculated from metered output

At the end of each day's production shift submit electronic and printed media. Present data on electronic media in comma-separated values (CSV) or tab-separated values (TSV) format. Captured data, for the ingredients represented by production snapshot, must have allowances for sufficient fields to satisfy the amount of data required by these specifications and include data titles at least once per report.

39-1.01C(2)(i) Liquid Antistrip Treatment

If liquid antistrip (LAS) treatment is used, submit the following with your proposed JMF submittal:

1. MSDS for LAS.
2. One 1-pint sample.
3. Infrared analysis including copy of absorption spectra.
4. Certified copy of test results and an MSDS for each LAS lot.
5. Certificate of compliance for each LAS shipment. With each certificate of compliance, include:
   5.1. Your signature and printed name.
   5.2. Shipment number
   5.3. Material type.
   5.4. Material specific gravity
   5.5. Refinery.
   5.6. Consignee.
   5.7. Destination.
   5.8. Quantity.
   5.9. Contact or purchase order number.
   5.10. Shipment date
6. Proposed proportions for LAS. If you change the brand or type of LAS, submit a new JMF.

For each job site delivery of LAS, submit one 1/2-pint sample to METS. Submit shipping documents. Label each LAS sampling container with:

1. LAS type
2. Application rate
3. Sample date
4. Contract number

At the end of each day's production shift, submit production data in electronic and printed media. Present data on electronic media in tab delimited format. Use line feed carriage return with 1 separate record per line for each production data set. Allow sufficient fields for the specified data. Include data titles at least once per report. For each mixing operation type, submit the following items in order:

1. Batch mixing:
   - 1.1. Production date
   - 1.2. Time of batch completion
   - 1.3. Mix size and type
   - 1.4. Each ingredient's weight
   - 1.5. Asphalt binder content as a percentage of the total weight of mix
   - 1.6. LAS content as a percentage of the asphalt binder weight
2. Continuous mixing
   - 2.1. Production date
   - 2.2. Data capture time
   - 2.3. Mix size and type
   - 2.4. Flow rate of wet aggregate collected directly from the aggregate weigh belt
   - 2.5. Aggregate moisture content as percentage of the dry aggregate weight
   - 2.6. Flow rate of asphalt binder collected from the asphalt binder meter
   - 2.7. Flow rate of LAS collected from the LAS meter
   - 2.8. Asphalt binder content as percentage of the total weight of mix calculated from:
     - 2.8.1. Aggregate weigh belt output
     - 2.8.2. Aggregate moisture input
     - 2.8.3. Asphalt binder meter output
   - 2.9. LAS content as percentage of the asphalt binder weight calculated from:
     - 2.9.1. Asphalt binder meter output
     - 2.9.2. LAS meter output

39-1.01C(2)(j) Lime Treatment

If aggregate lime treatment is used, submit the following with your proposed JMF:

1. Exact lime proportions for fine and coarse virgin aggregate with the proposed JMF
2. If marination is required, the averaged aggregate quality test results within 24 hours of sampling
3. For dry lime aggregate treatment, a treatment data log from the dry lime and aggregate proportioning device in the following order:
   - 3.1. Treatment date
   - 3.2. Time of day the data is captured
   - 3.3 Aggregate size being treated
   - 3.4. HMA type and mix aggregate size
   - 3.5. Wet aggregate flow rate collected directly from the aggregate weigh belt
   - 3.6. Aggregate moisture content, expressed as a percent of the dry aggregate weight
   - 3.7. Flow rate of dry aggregate calculated from the flow rate of wet aggregate
   - 3.8. Dry lime flow rate
   - 3.9. Lime ratio from the accepted JMF for each aggregate size being treated
   - 3.10. Lime ratio from the accepted JMF for the combined aggregate
   - 3.11. Actual lime ratio calculated from the aggregate weigh belt output, the aggregate moisture input, and the dry lime meter output, expressed as a percent of the dry aggregate weight
   - 3.12. Calculated difference between the authorized lime ratio and the actual lime ratio
4. For lime slurry aggregate treatment, a treatment data log from the slurry proportioning device in the following order:
   - 4.1. Treatment date
   - 4.2. Time of day the data is captured
   - 4.3. Aggregate size being treated
   - 4.4. Wet aggregate flow rate collected directly from the aggregate weigh belt
   - 4.5. Moisture content of the aggregate just before treatment, expressed as a percent of the dry aggregate weight
   - 4.6. Dry aggregate flow rate calculated from the wet aggregate flow rate
   - 4.7. Lime slurry flow rate measured by the slurry meter
   - 4.8. Dry lime flow rate calculated from the slurry meter output

4.9. Authorized lime ratio for each aggregate size being treated
4.10. Actual lime ratio calculated from the aggregate weigh belt and the slurry meter output, expressed as a percent of the dry aggregate weight
4.11. Calculated difference between the authorized lime ratio and the actual lime ratio
4.12. Dry lime and water proportions at the slurry treatment time

Each day during lime treatment, submit the treatment data log on electronic media in tab delimited format on a removable CD-ROM storage disk. Each continuous treatment data set must be a separate record using a line feed carriage return to present the specified data on 1 line. The reported data must include data titles at least once per report.

39-1.01C(3) Asphalt Rubber Binder

For the asphalt rubber binder used, submit:

1. Log of production daily.
2. Certificate of compliance with test results for CRM and asphalt modifier with each truckload delivered to the HMA plant. The certificate of compliance for asphalt modifier must represent no more than 5,000 lbs.
3. Submit certified weight slips for the CRM and asphalt modifier furnished.

Submit a certificate of compliance for the asphalt rubber binder. With the certificate of compliance, submit test results for CRM and asphalt modifier with each truckload delivered to the HMA plant. A certificate of compliance for asphalt modifier must not represent more than 5,000 lbs.

39-1.01C(4) Reclaimed Asphalt Pavement

Submit QC test results for RAP gradation with the combined aggregate gradation within 2 days of taking RAP samples during HMA-SP (Type A) and production.

39-1.01C(5) Quality Control Plan

With your proposed JMF submittal, submit a QC plan for HMA. The QC plan must describe the organization and procedures for:

1. Controlling HMA quality characteristics
2. Obtaining samples, including sampling locations
3. Establishing, implementing, and maintaining QC
4. Determining when corrective actions are needed
5. Implementing corrective actions
6. Taking samples, including location of sampling

The QCP must address the elements affecting HMA quality including:

1. Aggregate
2. Asphalt binder
3. Additives
4. Production
5. Paving

The QC plan must include aggregate quality control sampling and testing during lime treatment.

The Engineer reviews the QC plan within 5 business days from the submittal. Do not start HMA production until the Engineer authorizes the plan.

If QC procedures, personnel, tester qualifications, or lab accreditation status change, submit a QC plan supplement at least 3 business days before implementing proposed changes.

39-1.01C(6) Inertial Profiler

At least 5 business days before start of initial profiling or changing profiler or operator, submit:

1. IP certification issued by the Department. The certification must be not more than 12 months old.
2. Operator certification for the IP issued by the Department. The operator must be certified for each different model of IP device operated. The certification must be not more than 12 months old.

3. List of manufacturer's recommended test procedures for IP calibration and verification.

As an alternative to the IP and operator certification by the Department, an equivalent certification from the Texas Transportation Institute will be accepted if the certification is dated before July 1, 2013 and is not more than 12 months old.

Within 2 business days after cross correlation testing, submit ProVAL profiler certification analysis report for cross correlation test results performed on test section to the Engineer and to the electronic mailbox address:

smoothness@dot.ca.gov

Within 2 business days after each day of inertial profiling, submit profile data to the Engineer and to the electronic mailbox address:

smoothness@dot.ca.gov

The profiling data must include:

1. Raw profile data for each lane.
2. ProVAL ride quality analysis report for IRIs of left and right wheel paths of each lane. Submit in pdf file format.
3. ProVAL ride quality analysis report for MRIs of each lane. Submit in pdf file format.
4. ProVAL smoothness assurance analysis report for IRIs of left wheel path. Submit in pdf file format.
5. ProVAL smoothness assurance analysis report for IRIs of right wheel path. Submit in pdf file format.
6. GPS data file for each lane in GPS exchange. Submit in GPS eXchange file format.
7. Manufacturer's recommended IP calibration and verification tests results.
8. AASHTO IP calibration and verification test results including bounce, block, and distance measurement instrument (DMI).

Submit the raw profile data in unfiltered electronic pavement profile file (PPF) format. Name the PPF file using the following naming convention:

YYYYMMDD_TTCCCRRR_D_L_W_S_X_PT.PPF

where:
YYYY = year
MM = Month, leading zero
DD = Day of month, leading zero
TT = District, leading zero
CCC = County, 2 or 3 letter abbreviation as shown in section 1-1.08
RRR = Route number, no leading zeros
D = Traffic direction as NB, SB, WB, or EB
L = Lane number from left to right in direction of travel
W = Wheel path as "L" for left, "R" for right, or "B" for both
S = Beginning station to the nearest foot (i.e., 10+20) or beginning post mile to the nearest hundredth (i.e., 25.06) no leading zero
X = Profile operation as "EXIST" for existing pavement, "INTER" for after prepaving smoothness correction, "PAVE" for after paving, and "CORR" for after final surface pavement correction
PT = Pavement type (i.e., HMA-SP, RHMA-SP-G, HMA-O, RHMA-O, RHMA-G, etc.)

Within 2 business days of performing straightedge measurements, submit areas requiring smoothness correction. Identify locations of smoothness correction by:

1. Location Number
2. District-County-Route
3. Beginning station or post mile to the nearest 0.01 mile
4. For correction areas within a lane:
   4.1. Lane direction as NB, SB, EB, or WB
   4.2. Lane number from left to right in direction of travel
   4.3. Wheel path as "L" for left, "R" for right, or "B" for both
5. For correction areas not within a lane:
   5.1. Identify pavement area (i.e., shoulder, weight station, turnout)
   5.2. Direction and distance from centerline as "L" for left or "R" for right
6. Estimated size of correction area

39-1.01C(7) Data Cores

At least 3 business days before starting coring, submit proposed methods and materials for backfilling data core holes.

Submit a summary of data cores taken and a photograph of each data core to the Engineer and to:

Coring@dot.ca.gov

39-1.01D Quality Control and Assurance

39-1.01D(1) General

AASHTO T 324 (Modified) is AASHTO T 324, "Hamburg Wheel-Track Testing of Compacted Hot Mix Asphalt (HMA)," with the following parameters:

1. Target air voids must equal 7 ± 1 percent for HMA-SP Type A, and HMA-SP
2. Specimen height must be 60 mm ± 1mm
3. Number of test specimens must be 4 (2 test sets)
4. Do not average test sets
5. Test specimen must be a 150 mm gyratory compacted specimen
6. Test temperature must be set at:
   6.1. 113 ± 2 degrees F for PG 58
   6.2. 122 ± 2 degrees F for PG 64
   6.3. 131 ± 2 degrees F for PG 70 and above
7. Measurements for impression must be taken at every 100 passes
8. Inflection point defined as the number of wheel passes at the intersection of the creep slope and the stripping slope
9. Testing shut off must be set at 25,000 passes

During production, take samples under California Test 125.

If the Engineer requests, sample the following materials in the presence of the Engineer and place in labeled containers weighing no more than 50 lbs each:

1. Coarse, fine, and supplemental fine aggregate from stockpiles, cold feed belts, or hot bins. Samples must include at least 150 lbs for each coarse aggregate, 100 lbs for each fine aggregate, and 10 lbs for each type of supplemental fines. The Department combines these aggregate samples to comply with the JMF target values submitted on *Contractor Job Mix Formula Proposal* form.
2. RAP from stockpiles or RAP system. Samples must be at least 100 lbs.
3. Asphalt binder from the binder supplier. Samples must be in four 1-quart cylindrical shaped cans with open top and friction lids.
4. Asphalt rubber binder with the components blended in the proportions to be used. Samples must be in four 1-quart cylindrical shaped cans with open top and friction lids.

Notify the Engineer at least 2 business days before sampling materials. For aggregate and RAP, split the samples into at least 4 parts. Submit 3 parts to the Engineer and use 1 part for your testing.

39-1.01D(2) Reserved

39-1.01D(3) Quality Control Plan

Implement your QC plan. If a change to your QC plan is needed, do not implement the change without authorization.

39-1.01D(4) Prepaving Conference

Meet with the Engineer at a prepaving conference at a mutually agreed time and place. Discuss the QC plan and the methods of performing production and paving work.

The following personnel must attend the prepaving conference:

1. Project Manager
2. Superintendent
3. HMA plant manager
4. HMA paving foreman
5. Technical representative for WMA technology, if WMA technology is used

39-1.01D(5) Quality Control Testing

Establish, maintain, and change a quality control system to ensure materials and work comply with the specifications. Submit quality control test results within 3 business days of a request.

Perform sampling and testing as specified in the following 4 tables:

Minimum Quality Control Requirements for Aggregate

Quality characteristic	Test method	Minimum sampling and testing frequency	HMA-SP		
			Type A	RHMA-SP-G	OGFC
Aggregate gradation[a]	AASHTO T 27	1 per 750 tons and any remaining part	JMF ± Tolerance[b]	JMF ± Tolerance[b]	JMF ± Tolerance[b]
Sand equivalent (min.)[c]	AASHTO T 176		47	47	--
Aggregate moisture content at continuous mixing plants and RAP moisture content at continuous mixing plants and batch mixing plants	AASHTO T 329	1 per 1500 tons and any remaining part	Report Only	Report Only	Report Only
Percent of crushed particles Coarse aggregate (%, min.) One fractured face Two fractured faces Fine aggregate (%, min) (Passing No. 4 sieve and retained on No. 8 sieve.) One fractured face	AASHTO T 335	One per 10,000 tons or 2 per project which-ever is more	 95 90 70	 -- 90 70	 90 90 90
Los Angeles Rattler (%, max.) Loss at 100 rev. Loss at 500 rev.	AASHTO T 96		12 40	12 40	12 40
Flat and elongated particles (%, max. by weight @ 5:1) 1/4" thru 3/4" grading 1" grading	ASTM D 4791		Report only 10	Report only	Report only
Fine aggregate angularity (%, min.)	AASHTO T 304, Method A		45	45	--

[a]If RAP is used, test the combined aggregate gradation under Laboratory Procedure LP-9.
[b]Comply with the allowable tolerances in section 39-1.01D(7).
[c]Report the average of 3 tests from a single split sample. Use of a Sand Reader Indicator is required as shown in AASHTO T 176, Figure 1. Sections 4.7, 4.8, 7.1.2, 8.4.2 and 8.4.3 do not apply.

Minimum Quality Control Requirements for in Place HMA

Quality Characteristic	Test method	Minimum sampling and testing frequency	HMA-SP		
			Type A	RHMA-SP-G	OGFC
Asphalt binder content (%)	AASHTO T 308 Method A	1 per 750 tons and any remaining part	JMF - 0.3, + 0.5	JMF - 0.4, + 0.5	JMF - 0.4, + 0.5
HMA moisture content (%, max.)	AASHTO T 329	1 per 2,500 tons but not less than 1 per paving day	1.0	1.0	1.0
Air voids content (%)[a] 1/4" and 3/8 gradings 1/2"and 3/4" gradings 1" grading with PG64-28M 1" grading with all other binders	AASHTO T 269	One per 4,000 tons or 2 per 5 business days, which-ever is more	 4 ± 1.5 4 ± 1.5 5 ± 1.5 4 ± 1.5	Specification ± 1.5	--
Voids in mineral aggregate (% min.) 1/4" grading 3/8" grading 1/2" grading 3/4" grading 1" grading with NMAS=1" with NMAS=3/4"	SP-2 Asphalt Mixtures Volumetrics[a]		 16.5-19.5 15.5-18.5 14.5-17.5 12.5-15.5 12.5-15.5 12.5-15.5 13.5-16.5	-- -- 18.0–23.0 18.0–23.0	--
Dust proportion 1/4" and 3/8" gradings 1/2" and 3/4" gradings 1" grading	SP-2 Asphalt Mixtures Volumetrics[a]		0.6–1.3 0.6–1.3 0.6–1.3	Report only	--

[a]Determine bulk specific gravity using AASHTO T 275.

Minimum Quality Control Requirements for in Place HMA

Quality Characteristic	Test method	Minimum sampling and testing frequency	HMA-SP		
			Type A	RHMA-SP-G	OGFC
Percent of theoretical maximum density (%) by core[a, b,c]	California Test 375	2 per paving day (min.)	92–97	92–97	--
Percent of theoretical maximum density by Nuclear gauge (%)[a, b, d]	California Test 375	3 per 250 tons but not less than 3 per paving day	92–97	92–97	--
Hamburg wheel track (minimum number of passes at 0.5 inch average rut depth) PG 58 PG 64 PG-70 PG-76 or higher	AASHTO T 324 (Modified)[e]	One per 10,000 tons or 1 per project which-ever is more	10,000 15,000 20,000 25,000	15,000 20,000 25,000 --	--
Hamburg wheel track (inflection point minimum number of passes) PG 58 PG 64 PG-70 PG-76 or higher	AASHTO T 324 (Modified)[e]		10,000 10,000 12,500 15,000	10,000 12,500 15,000 --	--
Moisture susceptibility (minimum dry strength, psi)	AASHTO T 283[f]	One per 10,000 tons or 1 per project which-ever is more	100	100	--
Moisture susceptibility (minimum wet strength, psi)	AASHTO T 283 [f,g]		70	70	--

[a]Determine theoretical maximum density if any of the following applies:

1. 1/2-inch, 3/8-inch, or 1/4-inch grading is used and the specified total paved thickness is at least 0.15 foot.
2. 1- inch and 3/4-inch aggregate grading is used and the specified total paved thickness is at least 0.20 foot.

[b]Determine percent of theoretical maximum density under California Test 375 except use:

1. AASHTO T 275 to determine in-place density of each density core instead of using the nuclear gauge in Part 4, "Determining In-Place Density By The Nuclear Density Device."
2. AASHTO T 209 Method A to determine theoretical maximum density instead of calculating test maximum density in Part 5, "Determining Test Maximum Density."

[c]Determine theoretical maximum density under AASHTO T 209 Method A at the frequency specified for Test Maximum Density in California Test 375, Part 5.D. Use a digital manometer and a pycnometer when performing AASHTO T 209.

[d]Verify gauge correlation to cores every 10,000 tons utilizing the average of two cores.

[e]Test as specified in section 39-1.01D(1).

[f]Test specimen must be 100 mm or 150 mm gyratory compacted specimens

[g]Freeze thaw required.

Miscellaneous Minimum Quality Control Requirements

Quality Characteristic	Test method	Minimum sampling and testing frequency	HMA		
			Type A	RHMA-SP-G	OGFC
Smoothness	LP-13	--	12-foot straightedge, localized roughness, and MRI	12-foot straightedge, localized roughness, and MRI	12-foot straightedge, localized roughness, and MRI
Asphalt rubber binder viscosity @ 375 °F (centipoises)	LP-11	Once per hour, minimum of 1 test per batch	--	1,500 – 4,000	1,500 – 4,000
Asphalt modifier	ASTM D 445 ASTM D 92 ASTM D 2007	1 per truckload delivered to the RHMA-G-SP production facility	--	Section 39-1.02D(2)(b)	Section 39-1.02D(2)(b)
Crumb rubber modifier	LP-10 CT 208 ASTM D 297	1 per truckload delivered to the RHMA-G-SP production facility	--	Section 39-1.02D(2)(e)	Section 39-1.02D(2)(e)

Prepare 3 briquettes for air voids content and VMA determination. Report the average of 3 tests.

For any quality characteristic except smoothness, if 2 consecutive quality control test results for 1 day's production do not comply with the specifications:

1. Notify the Engineer
2. Take corrective action
3. Show how you will comply with the specifications before resuming production and placement on the State highway

For any quality characteristic except smoothness, if any 3 quality control test results for 1 day's production do not comply with the specifications:

1. Stop production

2. Notify the Engineer
3. Take corrective action
4. Show how you will comply with the specifications before resuming production and placement on the State highway

39-1.01D(6) Asphalt Rubber Binder

Take asphalt rubber binder samples from the feed line connecting the asphalt rubber binder tank to the HMA plant. Sample and test asphalt rubber binder under Laboratory Procedure LP-11. Use an AASHTO-certified laboratory for testing.

Test asphalt rubber binder for compliance with the viscosity requirements in section 39-1.02D(2)(d). During asphalt rubber binder production and HMA production using asphalt rubber binder, measure viscosity every hour with not less than 1 reading for each asphalt rubber binder lot. Each asphalt binder lot consist of 1 or multiple batches of combined asphalt binder, asphalt modifier, and CRM proportioned under section 39-1.02D(2)(e). Log the measurements with the corresponding time and asphalt rubber binder temperature.

Sample and test gradation and wire and fabric content of CRM once per 10,000 lbs of scrap tire CRM and once per 3,400 lbs of high natural CRM. Sample and test scrap tire CRM and high natural CRM separately.

39-1.01D(7) Aggregate

Laboratories testing aggregate qualities and preparing the mix design and JMF must be qualified under AASHTO Materials Reference Laboratory program (AMRL), and the Department's Independent Assurance Program. Take samples under California Test 125.

Determine the aggregate moisture content in continuous mixing plants at a rate of 1 per 1500 tons and any remaining part.

39-1.01D(8) Reclaimed Asphalt Pavement

Sample and test processed RAP at a minimum frequency of 1 sample per 1000 tons with a minimum of 6 samples per fractionated stockpile to assure that its asphalt binder content and specific gravity meet the processed RAP quality characteristics. If a fractionated RAP stockpile is augmented, sample and test processed RAP quality characteristics at a minimum frequency of 1 sample per 500 tons of augmented RAP.

The quality characteristic for processed RAP asphalt binder content must be within ± 2.0 percent of the average fractionated RAP stockpile asphalt binder content when tested under ASTM D 2172 (Method B). If new fractionated RAP stockpiles piles are required, the average binder content of the new fractionated RAP stockpile must be within ± 2.0 percent of the average binder content of the original fractionated RAP stockpile.

The quality characteristic for maximum specific gravity for processed RAP must be within ± 0.06 when tested under AASHTO T 209, of the average maximum specific gravity reported on page 4 of your *Contractor Hot Mix Asphalt Design Data* form.

During production, sample RAP twice daily and perform QC testing for:

1. Aggregate gradation at least once a day under Laboratory Procedure LP-9
2. Moisture content at least twice a day

39-1.01D(9) Liquid Antistrip Treatment

For continuous mixing and batch mixing operations, sample asphalt binder before adding LAS. For continuous mixing operations, sample combined asphalt binder and LAS after the static mixer.

39-1.01D(10) Aggregate Lime Treatment

For lime slurry aggregate treatment and dry lime aggregate treatment with marination, sample and test before treatment at the minimum frequencies shown in the following table:

Aggregate Quality Control During Lime Treatment

Quality characteristic	Test method	Minimum sampling and testing frequency
Sand equivalent	AASHTO T 176	Once per 750 tons of untreated aggregate
Percent of crushed particles	AASHTO T 335	One per 10,000 tons or 2 per project whichever is more
Los Angeles Rattler	AASHTO T 96	
Fine aggregate angularity	AASHTO T 304 method A	
Flat and elongated particles	ASTM D4791	

Note: During lime treatment, sample coarse and fine aggregate from individual stockpiles. Combine aggregate in the JMF proportions. Run tests for aggregate quality in triplicate and report test results as the average of 3 tests.

For lime slurry aggregate treatment, determine the aggregate moisture content at least once during each 2 hours of treatment. Calculate moisture content under AASHTO T 329 and report it as a percent of dry aggregate weight. Use the moisture content calculations as a set point for the proportioning process controller.

39-1.01D(11) Production Start-up Evaluation

The Engineer evaluates HMA production and placement at production start-up.

Within the first 750 tons produced on the first day of HMA production, in the Engineer's presence and from the same production run, take samples of:

1. Aggregate
2. Asphalt binder
3. RAP
4. HMA

Sample aggregate from cold feed belts or hot bins. Take RAP samples from the RAP system. Sample HMA under California Test 125. You must identify your sampling location in your Quality Control Plan.

For aggregate, RAP, and HMA, split the samples into at least 4 parts and label their containers. Submit 3 split parts to the Engineer and keep 1 part.

You and the Engineer must test the split samples and report test results within 3 business days of sampling. If you proceed before receipt of the test results, the Engineer may consider the HMA placed to be represented by these test results.

Take 4-inch or 6-inch diameter density cores within the first 750 tons on the first day of HMA production. For each density core, the Engineer reports the bulk specific gravity determined under AASHTO T 275, Method A in addition to the percent of theoretical maximum density. You must test for in-place density at the density core locations and include them in your production tests for percent of theoretical maximum density.

39-1.01D(12) Nuclear Gauge Density

During HMA placement determine HMA density using a nuclear gauge. On the 1st day of production, develop a correlation factor between cores and nuclear gauge under California Test 375.

Take a minimum of 3 nuclear gauge density readings for every 250 tons of HMA placed at random locations you select.

39-1.01D(13) Smoothness

39-1.01D(13)(a) General

Section 39-1.01D(13) includes specifications for measuring pavement smoothness with an inertial profiler (IP) and straightedge, analyzing the data with FHWA's engineering software ProVAL, and correcting deficient smoothness.

Test pavement smoothness using an IP except use a 12-foot straightedge at the following locations:

1. Traffic lanes less than 1,000 feet in length including ramps, turn lanes, and acceleration and deceleration lanes
2. HMA pavement within 3 feet from and parallel to the construction joint formed between curbs, gutters, or existing pavement
3. Areas within 15 feet of manholes
4. Shoulders
5. Weigh-in-motion areas
6. Miscellaneous areas such as medians, gore areas, turnouts, and maintenance pullouts

Where IP testing is required, pavement smoothness for each lane must be determined by the international roughness index (IRI) for the left and right wheel paths in an individual lane and then averaging the results. The average of the IRIs from the left and right wheel paths for the same lane is the mean roughness index (MRI) of the lane. The wheel paths are a pair of lines 3 feet from and parallel to the edge of a lane. Left and right wheel paths are based on the direction of travel.

Where IP testing is required, identify areas of localized roughness. Areas of localized roughness must be identified using the ProVAL smoothness assurance analysis by calculating continuous IRI for each wheel path with a 25-foot interval using a 250 mm filter.

Collect profiling data under AASSHTO R 56 and analyze data using 250 mm and IRI filters.

39-1.01D(13)(b) Inertial Profiler Calibration and Verification Tests

IP equipment must display a current certification decal with expiration date.

Operate the IP according to the manufacturer's recommendations and AASHTO R57-10 at 1-inch recording intervals.

Notify the Engineer 2 business days before performing IP calibration and verification testing.

Conduct the following IP calibration and verification tests in the Engineer's presence each day before performing inertial profiling:

1. Block test. Verify the height sensor accuracy under AASHTO R57-10, section 5.3.2.3.
2. Bounce test. Verify the combined height sensor and accelerometer accuracy under AASHTO R57-10, section 5.3.2.3.2.
3. DMI test. Calibrate the accuracy of the testing procedure under AASHTO R56-10, section 8.4.
4. Manufacturer's recommended tests.

Conduct cross correlation IP verification test in the Engineer's presence before performing initial profiling. Verify cross correlation IP verification test at least annually. Conduct 5 repeat runs of the IP on an authorized test section. The test section must be on an existing asphalt concrete pavement surface 0.1 mile long. Calculate a cross correlation to determine the repeatability of your device under Section 8.3.1.2 of AASHTO R56-10 using ProVAL profiler certification analysis with a 3 feet maximum offset. The cross correlation must be a minimum of 0.92.

For each 0.1 mile section, your IRI values must be within 10 percent of the Department's IRI values. The Engineer may order you to recalibrate your IP equipment and reprofile. If your results are inaccurate due to operator error, the Engineer may disqualify your IP operator.

39-1.01D(13)(c) Acceptance Criteria

For areas that require pavement smoothness determined using an IP, the pavement surface must:

1. Have no areas of localized roughness with an IRI greater than 120 in/mi
2. Comply with the MRI requirements shown in the following tables for a 0.1 mile section:

HMA[a] Pavement Smoothness Acceptance Criteria

HMA thickness	MRI requirement
> 0.20 foot	60 in/mi or less
≤0.20 foot	75 in/mi or less

[a] Except OGFC

OGFC Pavement Smoothness Acceptance Criteria

OGFC placement on	MRI requirement
New construction, or HMA overlay	60 in/mi or less
Existing pavement	75 in/mi or less
Milled surface	75 in/mi or less

For areas that require pavement smoothness determined using a 12-foot straightedge, the HMA pavement surface must not vary from the lower edge of the straightedge by more than:

1. 0.01 foot when the straightedge is laid parallel with the centerline
2. 0.02 foot when the straightedge is laid perpendicular to the centerline and extends from edge to edge of a traffic lane
3. 0.02 foot when the straightedge is laid within 24 feet of a pavement conform

Pavement smoothness may be accepted based on your testing in the absence of the Department's testing.

39-1.01D(13)(d) Smoothness Testing

Notify the Engineer of start location by station and start time at least 2 business days before profiling.

Remove foreign objects on the pavement surface before profiling.

Mark the beginning and ending station on the pavement shoulder before profiling. Stationing must be the same when profiling more than one surface.

While collecting the profile data to determine IRI, record the following locations in the raw profile data:

1. Begin and end of all bridge approach slabs
2. Begin and end of all bridges
3. Begin and end of all culverts visible on the roadway surface

Determine the MRI for 0.1-mile fixed sections using the ProVAL ride quality analysis with a 250 mm filter. Profile the left and right wheel paths of each lane. Calculate the MRI of each lane. A partial section less than 0.1 mile that is the result of an interruption to continuous pavement surface must comply with the MRI specifications for a full section. Adjust the MRI for a partial section to reflect a full section based on the proportion of a section paved.

Determine the areas of localized roughness using a continuous IRI for each wheel path with a 25-foot interval using a 250 mm filter. Localized roughness greater than 120 in/mi must be corrected regardless of the IRI values of a 0.1-mile section.

Determine the MRI of the HMA, except OGFC. If the MRI of the final pavement surface is greater than the MRI acceptance requirement in the table titled "HMA Pavement Smoothness Acceptance Criteria" in section 39-1.01D(13)(c), correct to the MRI acceptance requirement in the table.

The final surface of HMA must meet MRI acceptance requirements in the table titled "HMA Pavement Smoothness Acceptance Criteria" in section 39-1.01D(13)(c), before placing HMA-O or RHMA-O.

Determine the MRI of the OGFC. If OGFC MRI is greater than the accepted value in the table titled "OGFC Pavement Smoothness Acceptance Criteria" in section 39-1.01D(13)(c), correct to the MRI acceptance requirement in the table.

39-1.01D(13)(e) Straightedge

Measure areas that require 12-foot straightedge. If the straightedge measurement is greater than the accepted value in section 39-1.01D(13)(c), correct to the acceptance requirement.

39-1.01D(13)(f) Smoothness Correction

If the final surface of the pavement does not comply with section 39-1.01D(13)(c), grind the pavement to within specified tolerances, remove and replace it, or place an overlay of HMA. Do not start corrective work until your method is authorized.

Smoothness correction of the final pavement surface must leave at least 75 percent of the specified HMA thickness. If ordered, core the pavement at the locations determined by the Engineer. Coring, including traffic control, is change order work. Remove and replace deficient pavement areas where the overlay thickness is less than 75 percent of the thickness specified as determined by the Engineer.

If you choose to correct OGFC, the Engineer determines if the corrective method causes raveling. OGFC that is raveling must be removed and replaced.

Corrected HMA pavement areas must be uniform rectangles with edges:

1. Parallel to the nearest HMA pavement edge or lane line
2. Perpendicular to the pavement centerline

On ground areas not to be overlaid with OGFC, apply fog seal coat under section 37-2.

Where corrections are made within areas requiring testing with IP, reprofile the entire lane length with the IP device.

Where corrections are made within areas requiring testing with a 12-foot straightedge, retest the corrected area with the straightedge.

39-1.01D(13)(g) Prepaving Inertial Profiler

Not Used

39-1.01D(13)(h) Prepaving Grinding

Not Used

39-1.01D(14) Density Cores

Take 4-inch or 6-inch density cores to determine percent of theoretical maximum density. Take a minimum of 2 density cores each paving day from random locations you select. Backfill and compact holes with authorized material.

39-1.01D(15) Data Cores

Data core summary and data core digital photographs are required to document the pavement structural section. Take data cores that include the completed HMA pavement, underlying base, and subbase material. Protect data cores and surrounding pavement from damage.

Take 4-inch or 6-inch diameter data cores:

1. At the beginning, end, and every 1/2 mile within the paving limits of each route on the project
2. After all paving is complete
3. From the center of the specified lane

On a 2-lane roadway, take data cores from either lane. On a 4-lane roadway, take data cores from each direction in the outermost lane. On a roadway with more than 4 lanes, take data cores from the median lane and the outermost lane in each direction. After coring, backfill and compact core holes with authorized material.

Each core must include the stabilized materials encountered. You may choose not to recover unstabilized material but you must identify the material. Unstabilized material includes:

1. Granular material
2. Crumbled or cracked stabilized material
3. Sandy or clayey soil

Prepare a summary for each data core, the summary must include:

1. Project identification number
2. Date cored
3. Core identification number
4. Type of materials recovered
5. Type and approximate thickness of unstabilized material not recovered
6. Total core thickness
7. Thickness of each individual material to within:
   7.1. For recovered material, 1/2 inch
   7.2. For unstabilized material, 1.0 inch
8. Location including:
   8.1. County
   8.2. Route
   8.3. Post mile
   8.4. Lane number
   8.5. Lane direction
   8.6. Station

Each data core digital photograph must include a ruler laid next to the data core. Each photograph must include:

1. The core
2. Project identification number
3. Core identification number
4. Date cored
5. County
6. Route
7. Post mile
8. Lane number
9. Lane direction

After data core summary and photograph submittal, dispose of cores.

39-1.01D(16) Engineer's Acceptance

The Engineer samples materials for testing under California Test 125 and tests under the applicable test method except samples may only be taken from one of the following:

1. Automatic sampling device, with sample taken from truck
2. Mat behind the paver

The Engineer's sampling and testing is independent of your QC sampling and testing, statistically-based, and random.

If you request, the Engineer splits samples and provides you with a part.

The Engineer prepares 3 briquettes for air voids content and VMA determination. The Engineer reports the average of 3 tests.

The Engineer accepts HMA based on:

1. Accepted JMF
2. Authorized QC plan
3. Visual inspection
4. Compliance quality characteristics of the following 4 tables:

HMA Aggregate Acceptance

Quality characteristic	Test method	HMA-SP		
		(Type A)	RHMA-SP-G	OGFC
Aggregate gradation[a, b]	AASHTO T 27	JMF ± Tolerance[c]	JMF ± Tolerance[c]	JMF ± Tolerance[c]
Sand equivalent (min.)[d]	AASHTO T 176	47	47	--
Percent of crushed particles Coarse aggregate (%, min.) One fractured face Two fractured faces Fine aggregate (%, min) (Passing No. 4 sieve and retained on No. 8 sieve.) One fractured face	AASHTO T 335	 95 90 70	 -- 90 70	 90 90 90
Los Angeles Rattler (%, max.) Loss at 100 rev. Loss at 500 rev.	AASHTO T 96	 12 40	 12 40	 12 40
Fine aggregate angularity (%, min.)	AASHTO T 304 Method A	45	45	--
Flat and elongated particles (%, max. by weight @ 5:1) 1/4" and 3/8" gradings 1/2" and 3/4" gradings 1" grading	ASTM D 4791	 Report only Report only 10	 Report only Report only	Report only

[a]The Engineer determines combined aggregate gradations containing RAP under Laboratory Procedure LP-9.
[b]"X" denotes the sieves the Engineer considers for the specified aggregate gradation.
[c]The tolerances must comply with the allowable tolerances in section 39-1.02E.
[d]The Engineer reports the average of 3 tests from a single split sample.

HMA Mix Acceptance

Quality characteristic	Test method	HMA-SP		
		Type A	RHMA-SP-G	OGFC
Asphalt binder content (%)	AASHTO T 308 Method A	JMF -0.3, +0.5	JMF - 0.4, +0.5	JMF - 0.4, +0.5
HMA moisture content (%, max.)	AASHTO T 329	1.0	1.0	1.0
Air voids content (%) [a, b]	AASHTO T 269	4 ± 1.5	Specification ± 1.5	--
Voids in mineral aggregate, Laboratory produced HMA[d] (% min.) 1/4" grading 3/8" grading 1/2" grading 3/4" grading 1" grading with NMAS=1" with NMAS=3/4"	SP-2 Asphalt Mixtures Volumetrics[c]	 16.5-19.5 15.5-18.5 14.5-17.5 13.5-16.5 13.5-16.5 14.5-17.5	-- -- 18.0–23.0 18.0–23.0	--
Voids in mineral aggregate, Plant Produced HMA (% min.) 1/4" grading 3/8" grading 1/2" grading 3/4" grading 1" grading with NMAS=1" with NMAS=3/4"	SP-2 Asphalt Mixtures Volumetrics[c]	 15.5-18.5 14.5-17.5 13.5-16.5 12.5-15.5 12.5-15.5 13.5-16.5	-- -- 18.0–23.0 18.0–23.0	--
Dust proportion 1/4" and 3/8" gradings 1/2" and 3/4" gradings 1" grading	SP-2 Asphalt Mixtures Volumetrics[c]	0.6–1.3 0.6–1.3 0.6–1.3	Report only	--
Percent of theoretical maximum density (%) by core[e f, g]	California Test 375	92–97	92–97	--

[a]The Engineer reports the average of 3 tests from a single split sample.
[b]The Engineer determines the bulk specific gravity of each lab-compacted briquette under AASHTO T 275 , and theoretical maximum specific gravity under AASHTO T 209, Method A.
[c]Determine bulk specific gravity using AASHTO T 275.
[d]The Engineer determines the laboratory prepared HMA value for mix design verification only
[e]The Engineer determines percent of theoretical maximum density if any of the following:

1. 1/2-inch, 3/8-inch, or 1/4-inch grading is used and the specified total paved thickness is at least 0.15 foot.
2. 1 inch, and 3/4-inch aggregate grading is used and the specified total paved thickness is at least 0.20 foot.

[f]The Engineer determines percent of theoretical maximum density under California Test 375 except the Engineer uses:

1. AASHTO T 275 to determine in-place density of each density core instead of using the nuclear gauge in Part 4, "Determining In-Place Density By The Nuclear Density Device."
2. AASHTO T 209 Method A to determine theoretical maximum density instead of calculating test maximum density in Part 5, "Determining Test Maximum Density."

[g]The Engineer determines theoretical maximum density (AASHTO T 209 Method A) at the frequency specified for Test Maximum Density under California Test 375, Part 5. D.

HMA Acceptance In Place

Quality characteristic	Test method	HMA-SP		
		Type A	RHMA-SP-G	OGFC
Hamburg wheel track (minimum number of passes at 0.5 inch average rut depth)	AASHTO T 324 (Modified)[a]			--
PG 58		10,000	15,000	
PG 64		15,000	20,000	
PG-70		20,000	25,000	
PG-76 or higher		25,000	--	
Hamburg wheel track (inflection point minimum number of passes)	AASHTO T 324 (Modified)[a]			--
PG 58		10,000	10,000	
PG 64		10,000	12,500	
PG-70		12,500	15,000	
PG-76 or higher		15,000	--	
Moisture susceptibility (minimum dry strength, psi)	AASHTO T 283[b]	100	100	--
Moisture susceptibility (minimum wet strength, psi)	AASHTO T 283[b,c]	70	70	--

[a]Test as specified in section 39-1.01D(1).
[b]Test specimens must be 100mm or 150mm gyratory compacted HMA
[c]Freeze thaw required.

Miscellaneous Quality HMA Acceptance

Quality characteristic	Test method	HMA-SP		
		Type A	RHMA-SP-G	OGFC
Smoothness	LP-13	12-foot straightedge, localized roughness, and MRI	12-foot straightedge, localized roughness, and MRI	12-foot straightedge, localized roughness, and MRI
Asphalt rubber binder viscosity @ 375 °F (centipoises)	LP-11	--	1,500–4,000	1,500–4,000
Asphalt modifier	ASTM D 445 ASTM D 92 ASTM D 2007	--	Section 39-1.02D(2)(b)	Section 39-1.02D(2)(b)
Crumb rubber modifier	LP-10 CT 208 ASTM D 297	--	Section 39-1.02D(2)(e)	Section 39-1.02D(2)(e)

No single test result may represent more than the smaller of 750 tons or 1 day's production.

For any single quality characteristic except smoothness, if 2 acceptance test results for 1 day's production do not comply with the specifications:

1. Stop production.
2. Take corrective action.
3. In the Engineer's presence, take samples and split each sample into 4 parts. Test 1 part for compliance with the specifications and submit 3 parts to the Engineer. The Engineer tests 1 part for compliance with the specifications and reserves and stores 2 parts.
4. Demonstrate compliance with the specifications before resuming production and placement on the State highway.

The Engineer tests the density core you take from each 250 tons of HMA-SP (Type A) and RHMA-SP-G production. The Engineer determines the percent of theoretical maximum density for each density core by determining the density core's density and dividing by the theoretical maximum density.

The Engineer determines the percent of theoretical maximum density from density cores taken from the final layer measured the full depth of the total paved HMA-SP (Type A), and RHMA-SP-G thickness if any of the following applies:

1. If 1/2-inch, 3/8-inch, or 1/4-inch aggregate grading is used and the specified total paved thickness is at least 0.15 foot and any layer is less than 0.15 foot.
2. If 1 inch, or 3/4-inch aggregate grading is used and the specified total paved thickness is at least 0.20 foot and any layer is less than 0.20 foot.

If the percent of theoretical maximum density does not comply with the specifications, the Engineer may accept the HMA-SP (Type A) and RHMA-SP-G and the Department deducts payment based on the factors shown in the following tables:

Reduced Payment Factors for Percent of Theoretical Maximum Density

HMA-SP (Type A) and RHMA-SP-G Percent of Theoretical maximum density	Reduced Payment Factor	HMA-SP (Type A) and RHMA-G-SP Percent of Theoretical maximum density	Reduced Payment Factor
92.0	0.0000	97.0	0.0000
91.9	0.0125	97.1	0.0125
91.8	0.0250	97.2	0.0250
91.7	0.0375	97.3	0.0375
91.6	0.0500	97.4	0.0500
91.5	0.0625	97.5	0.0625
91.4	0.0750	97.6	0.0750
91.3	0.0875	97.7	0.0875
91.2	0.1000	97.8	0.1000
91.1	0.1125	97.9	0.1125
91.0	0.1250	98.0	0.1250
90.9	0.1375	98.1	0.1375
90.8	0.1500	98.2	0.1500
90.7	0.1625	98.3	0.1625
90.6	0.1750	98.4	0.1750
90.5	0.1875	98.5	0.1875
90.4	0.2000	98.6	0.2000
90.3	0.2125	98.7	0.2125
90.2	0.2250	98.8	0.2250
90.1	0.2375	98.9	0.2375
90.0	0.2500	99.0	0.2500
< 90.0	Remove and Replace	> 99.0	Remove and Replace

39-1.01D(17) Dispute Resolution

You and the Engineer must work together to avoid potential conflicts and to resolve disputes regarding test result discrepancies. Notify the Engineer within 5 business days of receiving a test result if you dispute the test result.

If you or the Engineer dispute each other's test results, submit quality control test results and copies of paperwork including worksheets used to determine the disputed test results. An independent third party (ITP) performs referee testing. Before the ITP participates in a dispute resolution, the ITP must be qualified under AASHTO Materials Reference Laboratory program (AMRL), and the Department's Independent Assurance Program. The ITP must be independent of the project. By mutual agreement, the ITP is chosen from:

1. A Department laboratory
2. A Department laboratory in a district or region not in the district or region the project is located
3. The Transportation Laboratory
4. A laboratory not currently employed by you or your HMA producer

If split QC or acceptance samples are not available, the ITP uses any available material representing the disputed HMA for evaluation.

39-1.02 MATERIALS

39-1.02A General

Use RAP aggregate for HMA-SP (Type A) and as part of the virgin aggregate in a quantity up to a maximum of 25.0 percent of the aggregate blend.

Do not use RAP aggregate for RHMA-SP-G and OGFC

For replace asphalt concrete surfacing use HMA-SP (Type A)

For the safety edge, use the same type of HMA used for the adjacent lane or shoulder.

For water injection WMA technology, the foaming bitumen must have the following quality characteristics:

Quality Requirements for Foaming Bitumen

Quality characteristic	Test method	Requirement
Expansion ratio (minimum)	LP-12	4
Half-life (seconds minimum)	LP-12	4

If OGFC is specified, treat aggregate for OGFC with the same antistrip treatment used for HMA .

For miscellaneous areas and dikes:

1. Choose the 3/8-inch or 1/2-inch HMA-SP (Type A) and aggregate gradations.
2. Minimum asphalt binder content must be 6.8 percent for 3/8-inch aggregate and 6.0 percent for 1/2-inch aggregate. If you request and the Engineer authorizes, you may reduce the minimum asphalt binder content.
3. Choose asphalt binder Grade PG 70-10 or use the same grade specified for HMA-SP.

39-1.02B Geosynthetic Pavement Interlayer

Geosynthetic pavement interlayer must comply with section 88 for paving fabric or paving mat.

39-1.02C Tack Coat

Tack coat must comply with the specifications for asphaltic emulsion or asphalt binder. Choose the type and grade.

Notify the Engineer if you dilute asphaltic emulsion with water. The weight ratio of added water to asphaltic emulsion must not exceed 1 to 1.

Measure added water either by weight or volume under section 9-1.02 or you may use water meters from water districts, cities, or counties. If you measure water by volume, apply a conversion factor to determine the correct weight.

With each dilution, submit:

1. Weight ratio of water to bituminous material in the original asphaltic emulsion
2. Weight of asphaltic emulsion before diluting
3. Weight of added water
4. Final dilution weight ratio of water to asphaltic emulsion

39-1.02D Asphalt Binder

39-1.02D(1) General

Asphalt binder in HMA must comply with section 92.

For HMA-SP (Type A), the grade of binder must be PG 64-16.

For RHMA-SP-G, the grade of asphalt binder must be PG-64-10.

For replace asphalt surfacing, the grade of binder must be PG 64-16.

Asphalt binder for geosynthetic pavement interlayer must comply with section 92. Choose from Grades PG 64-10, PG 64-16, or PG 70-10.

LAS-treated asphalt binder must comply with the specifications for asphalt binder. Do not use LAS as a substitute for asphalt binder.

39-1.02D(2) Asphalt Rubber Binder

39-1.02D(2)(a) General

Use asphalt rubber binder in RHMA-SP-G, and RHMA-O. Asphalt rubber binder must be a combination of:

1. Asphalt binder
2. Asphalt modifier
3. CRM

The combined asphalt binder and asphalt modifier must be 80.0 ± 2.0 percent by weight of the asphalt rubber binder.

Determine the amount of asphalt rubber binder to be mixed with the aggregate for RHMA-SP-G as follows:

1. Base the calculations on the average of 3 briquettes produced at each asphalt rubber binder content.
2. Plot asphalt rubber binder content versus average air voids content for each set of three specimens and connect adjacent points with a best-fit curve.
3. Calculate voids in mineral aggregate (VMA) for each specimen, average each set, and plot the average versus asphalt rubber binder content.
4. Calculate the dust proportion and plot versus asphalt rubber binder content.
5. From the curve plotted in Step 2, select the theoretical asphalt rubber binder content at:
   5.1. 4 percent air voids for PG 64-16 or less.
   5.2. 5 percent air voids for PG 70-10 or greater.
6. At the selected asphalt rubber binder content, evaluate corresponding dust proportion to verify compliance with requirements. If necessary, develop an alternate composite aggregate gradation to conform to the RHMA-SP-G requirements.
7. Record the asphalt rubber binder content in Step 5 as the Optimum Bitumen Content (OBC).
8. To establish a recommended range, use the OBC as the high value and 0.2 percent less as the low value. The recommended range must not extend below 7.5 percent by total weight of the mix. If the OBC is 7.5 percent, then there is no recommended range, and 7.5 percent is the recommended value.

Laboratory mixing and compaction must comply with AASHTO R 35, except the mixing temperature of the aggregate must be between 300 degrees F and 325 degrees F. The mixing temperature of the asphalt-rubber binder must be between 375 degrees F and 425 degrees F. The compaction temperature of the combined mixture must be between 290 degrees F and 320 degrees F.

39-1.02D(2)(b) Asphalt Modifier

Asphalt modifier must be a resinous, high flash point, and aromatic hydrocarbon, and comply with:

Asphalt Modifier for Asphalt Rubber Binder

Quality characteristic	Test method	Requirement
Viscosity, m^2/s (x 10^{-6}) at 100 °C	ASTM D 445	X ± 3[a]
Flash Point, CL.O.C., °C	ASTM D 92	207 minimum
Molecular Analysis		
Asphaltenes, percent by mass	ASTM D 2007	0.1 maximum
Aromatics, percent by mass	ASTM D 2007	55 minimum

[a]The symbol "X" is the proposed asphalt modifier viscosity. "X" must be between 19 and 36. A change in "X" requires a new asphalt rubber binder design.

Asphalt rubber modifier must be from 2.0 percent to 6.0 percent by weight of the asphalt binder in the asphalt rubber binder.

39-1.02D(2)(c) Asphalt Rubber Binder Design and Profile

Submit a proposal for asphalt rubber binder design and profile. In the design, include the asphalt, asphalt modifier, and CRM and their proportions. The profile is not a performance specification and only serves to indicate expected trends in asphalt rubber binder properties during binder production. The profile must include the same component sources for the asphalt rubber binder used.

Design the asphalt rubber binder from testing you perform for each quality characteristic and for the reaction temperatures expected during production. The 24-hour (1,440-minute) interaction period determines the design profile. At a minimum, mix asphalt rubber binder components, take samples, and perform and record the tests shown in the following table:

Asphalt Rubber Binder Reaction Design Profile

Test	Minutes of reaction [a]							Limits
	45	60	90	120	240	360	1440	
Cone penetration @ 77 °F, 0.10-mm (ASTM D 217)	X [b]				X		X	25–70
Resilience @ 77 °F, percent rebound (ASTM D 5329)	X				X		X	18 min.
Field softening point, °F (ASTM D 36)	X				X		X	125–165
Viscosity, centipoises (LP-11)	X	X	X	X	X	X	X	1,500–4,000

[a] Six hours (360 minutes) after CRM addition, reduce the oven temperature to 275 °F for 16 hours. After the 16-hour (1,320-minutes) cool down after CRM addition, reheat the binder to the reaction temperature expected during production for sampling and testing at 24 hours (1,440 minutes).
[b] "X" denotes required testing

39-1.02D(2)(d) Asphalt Rubber Binder

After interacting for at least 45 minutes, asphalt rubber binder must have the values for the quality characteristics shown in the following table:

Asphalt Rubber Binder

Quality characteristic	Test for quality control or acceptance	Test method	Value	
			Minimum	Maximum
Cone penetration @ 77 °F, 0.10 mm	Acceptance	ASTM D 217	25	70
Resilience @ 77 °F, percent rebound	Acceptance	ASTM D 5329	18	--
Field softening point, °F	Acceptance	ASTM D 36	125	165
Viscosity @ 375 °F, centipoises	Quality control	LP-11	1,500	4,000

39-1.02D(2)(e) Crumb Rubber Modifier

CRM must consist of a ground or granulated combination of scrap tire CRM and high natural CRM. CRM must be 75.0 ± 2.0 percent scrap tire CRM and 25.0 ± 2.0 percent high natural CRM by total weight of CRM. Scrap tire CRM must be from any combination of automobile tires, truck tires, or tire buffings.

Sample and test scrap tire CRM and high natural CRM separately. CRM must comply with:

Crumb Rubber Modifier for Asphalt Rubber Binder

Quality characteristic	Test method	Requirement
Scrap tire CRM gradation (% passing No. 8 sieve)	LP-10	100
High natural CRM gradation (% passing No. 10 sieve)	LP-10	100
Wire in CRM (% max.)	LP-10	0.01
Fabric in CRM (% max.)	LP-10	0.05
CRM particle length (inch max.) [a]	--	3/16
CRM specific gravity [a]	California Test 208	1.1 – 1.2
Natural rubber content in high natural CRM (%) [a]	ASTM D 297	40.0 – 48.0

[a]Test at mix design and for Certificate of Compliance.

Only use CRM ground and granulated at ambient temperature. If steel and fiber are cryogenically separated, it must occur before grinding and granulating. Only use cryogenically produced CRM particles that can be ground or granulated and not pass through the grinder or granulator.

CRM must be dry, free-flowing particles that do not stick together. CRM must not cause foaming when combined with the asphalt binder and asphalt modifier. You may add calcium carbonate or talc up to 3 percent by weight of CRM.

39-1.02E Aggregate

Aggregate must be clean and free from deleterious substances.

Gradations are based on nominal maximum aggregate size (NMAS).

The aggregate for HMA-SP (Type A) must comply with the 3/4-inch grading.

The aggregate for RHMA-SP-G must comply with the 3/4-inch grading.

The aggregate for replace asphalt concrete surfacing, HMA-SP-(Type A) must comply with the 3/4-inch grading.

Aggregate gradation must be within the TV limits for the specified sieve size shown in the following tables:

Aggregate Gradation
(Percentage Passing)
HMA-SP (Type A)

1 inch HMA-SP (Type A)

Sieve Sizes	Target Value Limits	Allowable Tolerance
1"	100	—
3/4"	88-93	TV ± 5
1/2"	72-85	TV ± 6
3/8”	55-70	TV ± 6
No. 4	35-52	TV ± 7
No. 8	22-40	TV ± 5
No. 30	8-24	TV ± 4
No. 200	3–7	TV ± 2

3/4-inch HMA-SP (Type A)

Sieve Sizes	Target Value Limits	Allowable Tolerance
1"	100	—
3/4"	90–98	TV ± 5
1/2"	70–90	TV ± 6
No. 4	42–58	TV ± 5
No. 8	29–43	TV ± 5
No. 30	10–23	TV ± 4
No. 200	2–7	TV ± 2

1/2-inch HMA-SP (Type A)

Sieve Sizes	Target Value Limits	Allowable Tolerance
3/4"	100	--
1/2"	95–98	TV ± 5
3/8"	72–95	TV ± 5
No. 4	52–69	TV ± 5
No. 8	35–55	TV ± 5
No. 30	15–30	TV ± 4
No. 200	2–8	TV ± 2

3/8-inch HMA-SP (Type A)

Sieve Sizes	Target Value Limits	Allowable Tolerance
1/2"	100	--
3/8"	95–98	TV ± 5
No. 4	55–75	TV ± 5
No. 8	30–50	TV ± 5
No. 30	15–35	TV ± 5
No. 200	2–9	TV ± 2

1/4-inch HMA-SP (Type A)

Sieve Sizes	Target Value Limits	Allowable Tolerance
3/8"	100	--
No. 4	95–98	TV ± 5
No. 8	70–80	TV ± 6
No. 30	34–45	TV ± 5
No. 200	2–12	TV ± 4

Aggregate Gradation
(Percentage Passing)
Rubberized Hot Mix Asphalt - Gap Graded (RHMA-SP-G)

3/4-inch RHMA-SP-G

Sieve Sizes	Target Value Limits	Allowable Tolerance
1"	100	--
3/4"	95–98	TV ± 5
1/2"	83–87	TV ± 6
3/8"	65–70	TV ± 5
No. 4	28–42	TV ± 6
No. 8	14–22	TV ± 5
No. 200	0–6	TV ± 2

1/2-inch RHMA-SP-G

Sieve Sizes	Target Value Limits	Allowable Tolerance
3/4"	100	--
1/2"	90–98	TV ± 6
3/8"	83–87	TV ± 5
No. 4	28–42	TV ± 6
No. 8	14–22	TV ± 5
No. 200	0–6	TV ± 2

Aggregate Gradation
(Percentage Passing)
Open Graded Friction Course (OGFC)

1-inch OGFC

Sieve Sizes	Target Value Limits	Allowable Tolerance
1 1/2"	100	--
1"	99–100	TV ± 5
3/4"	85–96	TV ± 5
1/2"	55–71	TV ± 6
No. 4	10–25	TV ± 7
No. 8	6–16	TV ± 5
No. 200	1–6	TV ± 2

1/2-inch OGFC

Sieve Sizes	Target Value Limits	Allowable Tolerance
3/4"	100	--
1/2"	95–100	TV ± 6
3/8"	78–89	TV ± 6
No. 4	28–37	TV ± 7
No. 8	7–18	TV ± 5
No. 30	0–10	TV ± 4
No. 200	0–3	TV ± 2

3/8-inch OGFC

Sieve Sizes	Target Value Limits	Allowable Tolerance
1/2"	100	--
3/8"	90–100	TV ± 6
No. 4	29–36	TV ± 7
No. 8	7–18	TV ± 6
No. 30	0–10	TV ±5
No. 200	0–3	TV ± 2

Aggregate gradation must be before the addition of asphalt binder and must include supplemental fines. The Engineer tests for aggregate grading under AASHTO T 27, note 4, and AASHTO T 11 do not apply. Use a mechanical sieve shaker. Aggregate shaking time must not exceed 10 minutes for both course and fine aggregate portions.

Choose a sieve size TV within each target value limits shown in the tables titled "Aggregate Gradation."

Before the addition of asphalt binder and lime treatment, aggregate must comply with:

Aggregate Quality

Quality characteristic	Test method	HMA-SP		
		Type A	RHMA-G-SP	OGFC
Percent of crushed particles Coarse aggregate (% min.) One fractured face Two fractured faces Fine aggregate (% min) (Passing No. 4 sieve and retained on No. 8 sieve.) One fractured face	AASHTO T 335	 95 90 70	 -- 90 70	 90 90 90
Los Angeles Rattler (% max.) Loss at 100 Rev. Loss at 500 Rev.	AASHTO T 96	 12 40	 12 40	 12 40
Sand equivalent (min.)[a, b]	AASHTO T 176	47	47	--
Fine aggregate angularity (% min.)	AASHTO T 304 Method A	45	45	--
Flat and elongated particles (% max. by weight @ 5:1)	ASTM D 4791	10	10	10

[a]Reported value must be the average of 3 tests from a single sample.
[b]Use of a Sand Reader Indicator is required as shown in AASHTO T 176, Figure 1. Sections 4.7, 4.8, 7.1.2, 8.4.2 and 8.4.3 do not apply

39-1.02F Reclaimed Asphalt Pavement

For HMA-SP (Type A), substitute RAP aggregate for part of the virgin aggregate in a quantity up to a maximum of 25.0 percent of the aggregate blend

Provide enough space for meeting all RAP handling requirements at your facility. Provide a clean, graded base, well drained area for stockpiles.

If RAP is from multiple sources blend the RAP thoroughly and completely before fractionating.

Fractionate RAP stockpiles into 2 sizes, a coarse fraction RAP retained on 3/8-inch screen, and a fine fraction RAP passing 3/8-inch screen.

RAP Stockpile Fractionation Gradation Requirements

Quality Characteristic	Test Method	Requirement
Course	California Test 202[a]	100% passing the 1 inch screen
Fine	California Test 202 [a]	98%-100% passing the 3/8 inch screen

[a] Maximum mechanical shaking time is 10 minutes

Isolate the processed RAP stockpiles from other materials. Store processed RAP in conical or longitudinal stockpiles. Processed RAP must not be agglomerated or be allowed to congeal in large stockpiles.

39-1.02G Liquid Antistrip

LAS total amine value must be 325 minimum when tested under ASTM D 2074.

Use only 1 LAS type or brand at a time. Do not mix LAS types or brands.

Store and mix LAS under the manufacturer's instruction.

39-1.02H Lime

Lime for treating aggregate must be high-calcium hydrated lime and comply with section 24-2.02B.

39-1.02I Water

Water for lime treated aggregate must comply with section 24-2.02C.

39-1.02J Hot Mix Asphalt Production

39-1.02J(1) General

Produce HMA in a batch mixing plant or a continuous mixing plant.

HMA plants must be Department-qualified. Before production, the HMA plant must have a current qualification under the Department's Materials Plant Quality Program.

Weighing and metering devices used for the production of additive enhanced HMA must comply with the requirements of the MPQP. If a loss-in-weight meter is used for dry HMA additive, the meter must have an automatic and integral material delivery control system for the refill cycle. Proportion aggregate by hot or cold feed control. During production, you may adjust hot or cold feed proportion controls for virgin aggregate and RAP.

For HMA produced using WMA technology production of HMA must be at a temperature between 240 and 325 degrees F.

For the Method compaction, HMA produced using WMA technology must be produced at the temperatures specified in section 39-1.03E(4).

HMA-SP (Type A) must not exceed 25 percent RAP by weight of the virgin aggregate. Do not use RAP with RHMA-SP-G, RHMA-O, or OGFC.

39-1.02J(2) Mixing

Mix HMA ingredients into a homogeneous mixture of coated aggregates.

Asphalt binder must be from 275 to 375 degrees F when mixed with aggregate.

Asphalt rubber binder must be from 375 to 425 degrees F when mixed with aggregate.

When mixed with asphalt binder, aggregate must not be more than 325 degrees F except aggregate for OGFC with unmodified asphalt binder must be not more than 275 degrees F. Aggregate temperature specifications do not apply to RAP.

HMA must not be more than 325 degrees F.

39-1.02J(3) Asphalt Rubber Binder

Asphalt rubber binder blending plants must be authorized under the Departments Materials Plant Quality Program

Deliver scrap tire CRM and high natural CRM in separate bags.

Either proportion and mix asphalt binder, asphalt modifier, and CRM simultaneously or premix the asphalt binder and asphalt modifier before adding CRM. If you premix asphalt binder and asphalt modifier, mix them for at least 20 minutes. When you add CRM, the asphalt binder and asphalt modifier must be from 375 to 440 degrees F.

Do not use asphalt rubber binder during the first 45 minutes of the reaction period. During this period, the asphalt rubber binder mixture must be between 375 degrees F and the lower of 425 or 25 degrees F below the asphalt binder's flash point indicated in the MSDS.

If any asphalt rubber binder is Not Used within 4 hours after the reaction period, discontinue heating. If the asphalt rubber binder drops below 375 degrees F, reheat before use. If you add more scrap tire CRM to the reheated asphalt rubber binder, the binder must undergo a 45-minute reaction period. The added scrap tire CRM must not exceed 10 percent of the total asphalt rubber binder weight. Reheated and reacted asphalt rubber binder must comply with the viscosity specifications for asphalt rubber binder in section 39-1.02D(2). Do not reheat asphalt rubber binder more than twice.

39-1.02J(4) Proportioning Warm Mix Asphalt Technologies

Proportion all ingredients by weight. The HMA plant process controller (PPC) must be the sole source of ingredient proportioning control and be fully interfaced with all scales and meters used in the production process. The addition of the HMA additive must be controlled by the PPC.

Calibrate the loss-in-weight meter by:

1. Including at least 1 complete system refill cycle during each calibration test run
2. Operating the device in a normal run mode for 10 minutes immediately before starting the calibration process
3. Isolating the scale system within the loss-in-weight feeder from surrounding vibration
4. Checking the scale system within the loss-in-weight feeder for accuracy before and after the calibration process and daily during mix production
5. Using a 15-minute or 250-pound-minimum test run size for a dry ingredient delivery rate of less than 1 ton/hr
6. Complying with the limits of Table B, "Conveyor Scale Testing Extremes," in the MPQP

Produce additive enhanced HMA by using either a continuous mixing or a batch type HMA plant.

Liquid ingredient additive, including a normally dry ingredient made liquid, must be proportioned with a mass flow meter at continuous mixing plants. Use a mass flow meter or a container scale to proportion liquid additives at batch mixing plants.

Continuous mixing plants using HMA additives must comply with the following:

1. Dry ingredient additives for continuous production must be proportioned with a conveyor scale or a loss-in-weight meter.
2. HMA PPC and ingredient measuring systems must be capable of varying all ingredient feed rates proportionate with the dry aggregate delivery at all production rates and rate changes.
3. Liquid HMA additive must enter the production stream with the binder. Dry HMA additive must enter the production stream at or before the mixing area.
4. If dry HMA additives are used at continuous mixing HMA plants, baghouse dust systems must return all captured material to the mix.
5. HMA additive must be proportioned to within ±0.3 percent of the target additive rate.

Batch mixing plants using HMA additives must comply with the following:

1. Metered HMA additive must be placed in an intermediate holding vessel before being added to the stream of asphalt binder as it enters the pugmill.
2. If a container scale is used, weigh additive before combining with asphalt binder. Keep the container scale separate from other ingredient proportioning. The container scale capacity must be no more than twice the volume of the maximum additive batch size. The container scale's graduations must be smaller than the proportioning tolerance or 0.001 times the container scale capacity.
3. Dry HMA additive proportioning devices must be separate from metering devices for the aggregates and asphalt binder. Proportion dry HMA additive directly into the pugmill or place in an intermediate holding vessel to be added to the pugmill at the appropriate time in the batch cycle. Dry ingredients for batch production must be proportioned with a hopper scale.
4. Zero tolerance for the HMA additive batch scale is ±0.5 percent of the target additive weight. The indicated HMA additive batch scale weight may vary from the preselected weight setting by up to ±1.0 percent of the target additive weight.

The HMA plant must have a sampling device in the feed line connecting the additive storage to the additive metering system. The sampling equipment must comply with California Test 125.

39-1.02J(5) Liquid Antistrip Treatment

Perform liquid antistrip treatment (LAS) when the HMA mix design determines LAS treatment of HMA is required. LAS must be from 0.5 to 1.0 percent by weight of asphalt binder.

If 3 consecutive sets of recorded production data show actual delivered LAS weight is more than ±1 percent of the authorized mix design LAS weight, stop production and take corrective action.

If a set of recorded production data shows actual delivered LAS weight is more than ±2 percent of the authorized mix design LAS weight, stop production. If the LAS weight exceeds 1.2 percent of the asphalt binder weight, do not use the HMA represented by that data.

The continuous mixing plant controller proportioning the HMA must produce a production data log. The log consists of a series of data sets captured at 10-minute intervals throughout daily production. The data must be a production activity register and not a summation. The material represented by the data is the quantity produced 5 minutes before and 5 minutes after the capture time. For the duration of the Contract, collected data must be stored by the plant controller or a computer's memory at the plant.

The Engineer orders proportioning operations stopped for any of the following if you:

1. Do not submit data
2. Submit incomplete, untimely, or incorrectly formatted data
3. Do not take corrective actions
4. Take late or unsuccessful corrective actions
5. Do not stop production when proportioning tolerances are exceeded
6. Use malfunctioning or failed proportioning devices

If you stop production, notify the Engineer of any corrective actions taken before resuming.

39-1.02J(6) Aggregate Lime Treatment

Perform aggregate lime treatment when the HMA mix design determines aggregate lime treatment is required. Notify the Engineer at least 24 hours before the start of aggregate treatment.

Do not treat RAP.

For aggregate dry lime treatment, marinate aggregate if the plasticity index determined under California Test 204 is from 4 to 10.

For lime slurry aggregate treatment, treat aggregate separate from HMA production, stockpile and marinate the aggregate.

If marination is required:

1. Treat and marinate coarse and fine aggregates separately.
2. Treat the aggregate and stockpile for marination only once.
3. Treat the aggregate separate from HMA production.

The lime ratio is the pounds of dry hydrated lime per 100 lbs. of dry virgin aggregate expressed as a percentage. Water content of slurry or untreated aggregate must not affect the lime ratio.

Aggregate gradations must have the lime ratio ranges shown in the following table:

Aggregate gradation	Lime ratio percent
Coarse	0.4–1.0
Fine	1.5–2.0
Combined	0.8–1.5

You may reduce the combined aggregate lime ratio for OGFC to 0.5 from 1.0 percent.

The lime ratio for fine and coarse aggregate must be within ±0.2 percent of the lime ratio in the accepted JMF. The lime ratio must be within ±0.2 percent of the authorized lime ratio when you combine the individual aggregate sizes in the JMF proportions. The lime ratio must be determined before the addition of RAP.

The device controlling lime and aggregate proportioning must produce a treatment data log. The log consists of a series of data sets captured at 10-minute intervals throughout daily treatment. The data must be a treatment activity register and not a summation. The material represented by a data set is the quantity produced 5 minutes before and 5 minutes after the capture time. For the duration of the Contract, collected data must be stored by the controller.

If 3 consecutive sets of recorded treatment data indicate deviation more than 0.2 percent above or below the lime ratio in the accepted JMF, stop treatment.

If a set of recorded treatment data indicates a deviation of more than 0.4 percent above or below the lime ratio in the accepted JMF, stop treatment and do not use the material represented by that set of data in HMA.

If 20 percent or more of the total daily treatment indicates deviation of more than 0.2 percent above or below the lime ratio in the accepted JMF, stop treatment and do not use the day's treated aggregate in HMA.

If you stop treatment for noncompliance, you must implement corrective action and successfully treat aggregate for a 20-minute period. Notify the Engineer before beginning the 20-minute treatment period.

39-1.02J(7) Proportioning Dry Lime

Proportion dry lime by weight with a continuous operation.

If you use a batch-type proportioning operation for HMA production, control proportioning in compliance with the specifications for continuous mixing plants. Use a separate dry lime aggregate treatment operation from HMA batching operations including:

1. Pugmill mixer
2. Controller
3. Weigh belt for the lime
4. Weigh belt for the aggregate

If using a continuous mixing operation for HMA without lime marinated aggregates, use a controller that measures the blended aggregate weight after any additional water is added to the mixture. The controller must determine the quantity of lime added to the aggregate from the aggregate weigh belt input in connection with the manually input total aggregate moisture, the manually input target lime content, and the lime proportioning system output. Use a continuous aggregate weigh belt and pugmill mixer for the lime treatment operation in addition to the weigh belt for the aggregate proportioning to asphalt binder in the HMA plant. If you use a water meter for moisture control for lime treatment, the meter must comply with Materials Plant Quality Program manual.

At the time of mixing dry lime with aggregate, the aggregate moisture content must ensure complete lime coating. The aggregate moisture content must not cause aggregate to be lost between the point of weighing the combined aggregate continuous stream and the dryer. Add water for mixing and coating aggregate to the aggregate before dry lime addition. Immediately before mixing lime with aggregate, water must not visibly separate from aggregate.

The HMA plant must be equipped with a bag-house dust system. Material collected in the dust system must be returned to the mix.

39-1.02J(8) Proportioning Lime Slurry

Proportion lime and water with a continuous or batch operation.

Add lime to the aggregate as slurry consisting of mixed dry lime and water at a ratio of 1 part lime to from 2 to 3 parts water by weight. The slurry must completely coat the aggregate.

Immediately before mixing lime slurry with the aggregate, water must not visibly separate from the aggregate.

39-1.02J(9) Mixing Dry Lime and Aggregate

Mix aggregate, water, and dry lime with a continuous pugmill mixer with twin shafts. Immediately before mixing lime with aggregate, water must not visibly separate from the aggregate. Store dry lime in a uniform and free-flowing condition. Introduce dry lime to the pugmill in a continuous operation. The introduction must occur after the aggregate cold feed and before the point of proportioning across a weigh belt and the aggregate dryer. Prevent loss of dry lime.

The pugmill must be equipped with paddles arranged to provide sufficient mixing action and mixture movement. The pugmill must produce a homogeneous mixture of uniformly coated aggregates at mixer discharge.

If the aggregate treatment operation is stopped longer than 1 hour, clean the equipment of partially treated aggregate and lime.

Aggregate must be completely treated before introduction into the mixing drum.

39-1.02J(10) Mixing Lime Slurry and Aggregate

Proportion lime slurry and aggregate by weight in a continuous operation.

39-1.02J(11) Production

Before virgin aggregate is treated, it must comply with the aggregate quality specifications. Do not test treated aggregate for quality control except for gradation. The Department does not test treated aggregate for acceptance except for gradation.

The Engineer determines the combined aggregate gradation during HMA production after you have treated the aggregate.

Treated aggregate must not have lime balls or clods.

For any of the following, the Engineer orders proportioning operations stopped if you:

1. Do not submit the treatment data log
2. Do not submit the aggregate quality control data for marinated aggregate
3. Submit incomplete, untimely, or incorrectly formatted data
4. Do not take corrective actions
5. Take late or unsuccessful corrective actions
6. Do not stop treatment when proportioning tolerances are exceeded
7. Use malfunctioning or failed proportioning devices

If you stop treatment, notify the Engineer of any corrective actions taken and conduct a successful 20-minute test run before resuming treatment.

If marination is required, marinate treated aggregate in stockpiles from 24 hours to 60 days before using in HMA. Do not use aggregate marinated longer than 60 days.

39-1.02J(12) Spreading and Compacting Equipment

39-1.02J(12)(a) General

Paving equipment for spreading must be:

1. Self-propelled
2. Mechanical
3. Equipped with a screed or strike-off assembly that can distribute HMA the full width of a traffic lane
4. Equipped with a full-width compacting device
5. Equipped with automatic screed controls and sensing devices that control the thickness, longitudinal grade, and transverse screed slope

Install and maintain grade and slope references.

The screed must produce a uniform HMA surface texture without tearing, shoving, or gouging.

The paver must not leave marks such as ridges and indentations unless you can eliminate them by rolling.

Rollers must be equipped with a system that prevents HMA from sticking to the wheels. You may use a parting agent that does not damage the HMA or impede the bonding of layers.

In areas inaccessible to spreading and compacting equipment:

1. Spread the HMA by any means to obtain the specified lines, grades and cross sections.
2. Use a pneumatic tamper, plate compactor, or equivalent to achieve thorough compaction.

39-1.02J(12)(b) Method Compaction Equipment

For method compaction, each paver spreading HMA must be followed by 3 rollers:

1. One vibratory roller specifically designed to compact HMA. The roller must be capable of at least 2,500 vibrations per minute and must be equipped with amplitude and frequency controls. The roller's gross static weight must be at least 7.5 tons.
2. One oscillating type pneumatic-tired roller at least 4 feet wide. Pneumatic tires must be of equal size, diameter, type, and ply. The tires must be inflated to 60 psi minimum and maintained so that the air pressure does not vary more than 5 psi.
3. One steel-tired, 2-axle tandem roller. The roller's gross static weight must be at least 7.5 tons.

Each roller must have a separate operator. Rollers must be self-propelled and reversible.

Compact RHMA-G-SP under the specifications for compacting HMA except do not use pneumatic-tired rollers.

Compact OGFC with steel-tired, 2-axle tandem rollers. If placing over 300 tons of OGFC per hour, use at least 3 rollers for each paver. If placing less than 300 tons of OGFC per hour, use at least 2 rollers for each paver. Each roller must weigh between 126 to 172 lbs per linear inch of drum width. Turn the vibrator off.

39-1.02J(12)(c) Material Transfer Vehicle

A material transfer vehicle (MTV) must be used when placing RHMA-SP-G, HMA-O, or RHMA-O.

The MTV must:

1. Either receive HMA directly from trucks or use a windrow pickup head to load it from a windrow deposited on the roadway surface.
2. Transfer HMA directly into the paver's receiving hopper or feed system.
3. Remix the HMA, with augurs, before loading the paver.
4. Have sufficient capacity to prevent stopping the paver.

39-1.03 CONSTRUCTION

39-1.03A General

Do not pave HMA on a wet pavement or frozen surface.

You may deposit HMA in a windrow and load it in the paver if:

1. Paver is equipped with a hopper that automatically feeds the screed
2. Loading equipment can pick up the windrowed material and deposit it in the paver hopper without damaging base material
3. Activities for deposit, pickup, loading, and paving are continuous
4. HMA temperature in the windrow does not fall below 260 degrees F

You may place HMA in 1 or more layers on areas less than 5 feet wide and outside the traveled way, including shoulders. You may use mechanical equipment other than a paver for these areas. The equipment must produce uniform smoothness and texture.

HMA handled, spread, or windrowed must not stain the finished surface of any improvement, including pavement.

Do not use petroleum products such as kerosene or diesel fuel to release HMA from trucks, spreaders, or compactors.

HMA must be free of:

1. Segregation
2. Coarse or fine aggregate pockets
3. Hardened lumps

39-1.03B Miscellaneous Areas and Dikes

For miscellaneous areas and dikes, prepare the area to receive HMA. Preparing the area includes excavating and backfilling as needed. Spread with a self-propelled spreader. After spreading, miscellaneous HMA must be ready for compacting without further shaping.

Compact miscellaneous HMA with a vibratory roller providing a minimum of 7,000 lbs centrifugal force. With the vibrator on, compact at least 3 complete coverages over each layer, overlapping to prevent displacement. The speed of the vibratory roller in miles per hour must not exceed the vibrations per minute divided by 1,000. If the layer thickness is less than 0.08 foot, turn the vibrator off. Complete the 1st coverage before the mixture's temperature drops below 250 degrees F.

The finished surface must be:

1. Textured uniformly
2. Compacted firmly
3. Without depressions, humps, and irregularities
4. In compliance with the straightedge specifications for smoothness

39-1.03C Replace Asphalt Concrete Surfacing

Remove existing asphalt concrete surfacing and underlying base and replace with HMA. The Engineer determines the exact limits of replaced asphalt concrete surfacing.

Place replacement HMA under section 39-1.03E.

Replace asphalt concrete in a lane before the lane is specified to be opened to traffic under section 12-4.

Before removing asphalt concrete, outline the replacement area and cut neat lines with a saw or grind to full depth of the existing asphalt concrete. Do not damage asphalt concrete and base remaining in place.

Dispose of removed material.

If the base is excavated beyond the specified plane, replace it with HMA. The Department does not pay for this HMA

39-1.03D Surface Preparation

39-1.03D(1) General

Prepare subgrade or apply tack coat to surfaces receiving HMA. If specified, place geosynthetic pavement interlayer over a coat of asphalt binder.

39-1.03D(2) Subgrade

Subgrade to receive HMA-SP (Type A) must comply with the compaction and elevation tolerance specifications in the sections for the material involved. Subgrade must be free of loose and extraneous material. If HMA-SP (Type A) is paved on existing base or pavement, remove loose paving particles, dirt, and other extraneous material by any means including flushing and sweeping.

39-1.03D(3) Tack Coat

Apply tack coat:

1. To existing pavement including planed surfaces
2. Between HMA layers
3. To vertical surfaces of:
   - 3.1. Curbs
   - 3.2. Gutters
   - 3.3. Construction joints

39-1.03D(4) Geosynthetic Pavement Interlayer

Place geosynthetic pavement interlayer in compliance with the manufacturer's recommendations.

Before placing the geosynthetic pavement interlayer and asphalt binder:

1. Repair cracks 1/4 inch and wider, spalls, and holes in the pavement. Repairing cracks is change order work.
2. Clean the pavement of loose and extraneous material.

Immediately before placing the interlayer, apply 0.25 gallon ± 0.03 gallon of asphalt binder per square yard of interlayer or until the fabric is saturated. Apply asphalt binder the width of the geosynthetic pavement interlayer plus 3 inches on each side. At interlayer overlaps, apply asphalt binder on the lower interlayer the same overlap distance as the upper interlayer.

Align and place the interlayer with no overlapping wrinkles, except a wrinkle that overlaps may remain if it is less than 1/2 inch thick. If the overlapping wrinkle is more than 1/2 inch thick, cut the wrinkle out and overlap the interlayer no more than 2 inches.

The minimum HMA thickness over the interlayer must be 0.12 foot thick including conform tapers. Do not place the interlayer on a wet or frozen surface.

Overlap the interlayer borders between 2 inches and 4 inches. In the direction of paving, overlap the following roll with the preceding roll at any break.

You may use rolling equipment to correct distortions or wrinkles in the interlayer.

If asphalt binder tracked onto the interlayer or brought to the surface by construction equipment causes interlayer displacement, cover it with a small quantity of HMA.

Before placing HMA on the interlayer, do not expose the interlayer to:

1. Traffic except for crossings under traffic control and only after you place a small HMA quantity
2. Sharp turns from construction equipment
3. Damaging elements

Pave HMA on the interlayer during the same work shift.

39-1.03E Transporting, Spreading, and Compacting

39-1.03E(1) General

You may deposit HMA in a windrow and load it in the paver if:

1. Paver is equipped with a hopper that automatically feeds the screed
2. Loading equipment can pick up the windrowed material and deposit it in the paver hopper without damaging base material
3. Activities for deposit, pick-up, loading, and paving are continuous
4. HMA temperature in the windrow does not fall below 260 degrees F

HMA deposited in a windrow on the roadway surface must not extend more than 100 feet in front of the loading equipment or MTV.

You may pave HMA in 1 or more layers on areas less than 5 feet wide and outside the traveled way including shoulders. You may use mechanical equipment other than a paver for these areas. The equipment must produce a uniform smoothness and texture.

HMA handled, spread, or windrowed must not stain the finished surface of any improvement including pavement.

Do not use petroleum products such as kerosene or diesel fuel to release HMA from trucks, spreaders, hand tools or compactors.

HMA must be free of:

1. Segregation
2. Coarse or fine aggregate pockets
3. Hardened lumps

Longitudinal joints in the top layer must match specified lane edges. Alternate longitudinal joint offsets in lower layers at least 0.5 foot from each side of the specified lane edges. You may request other longitudinal joint placement patterns.

If the number of lanes change, pave each through lane's top layer before paving a tapering lane's top layer. Simultaneous to paving a through lane's top layer, you may pave an adjoining area's top layer including shoulders. Do not operate spreading equipment on any area's top layer until completing final compaction.

39-1.03E(2) Leveling

If a leveling course using HMA-SP (Type A) is specified, fill and level irregularities and ruts with HMA-SP (Type A) before spreading HMA over base, existing surfaces, or bridge decks. You may use mechanical equipment other than a paver for these areas. The equipment must produce a uniform smoothness and texture. HMA used to change an existing surface's cross slope or profile is not paid for as HMA leveling.

If placing HMA against the edge of existing pavement, sawcut or grind the pavement straight and vertical along the joint and remove extraneous material without damaging the surface remaining in place. If placing HMA against the edge of a longitudinal or transverse construction joint and the joint is damaged or not placed to a neat line, sawcut or grind the pavement straight and vertical along the joint and remove extraneous material without damaging the surface remaining in place. Repair or remove and replace damaged pavement at your expense.

39-1.03E(3) Compaction

Rolling must leave the completed surface compacted and smooth without tearing, cracking, or shoving. Complete finish rolling activities before the pavement surface temperature is:

1. Below 150 degrees F for HMA-SP (Type A) with unmodified binder
2. Below 140 degrees F for HMA-SP (Type A) with modified binder
3. Below 200 degrees F for RHMA-SP-G

If a vibratory roller is used as a finish roller, turn the vibrator off.

Do not use a pneumatic tired roller to compact RHMA-SP-G.

If a 3/4-inch aggregate grading is specified, you may use a 1/2-inch aggregate grading if the specified paved thickness is from 0.15 to 0.20 foot thick.

Spread and compact HMA as specified for method compaction in section 39-1.03E(4) for any of the following conditions:

1. Specified paved thickness is less than 0.15 foot.
2. Specified paved thickness is less than 0.20 foot and a 3/4-inch aggregate grading is specified and used.
3. Specified paved thickness is less than 0.25 foot and a 1-inch aggregate grading is specified and used.
4. You spread and compact at:
   - 4.1. Asphalt concrete surfacing replacement areas
   - 4.2. Leveling courses
   - 4.3. Areas the Engineer determines conventional compaction and compaction measurement methods are impeded

Do not open new HMA pavement to traffic until its mid-depth temperature is below 160 degrees F.

If you request and the Engineer authorizes, you may cool HMA-SP (Type A) with water when rolling activities are complete. Apply water under section 17.

Spread sand at a rate between 1 pound and 2 pounds per square yard on new RHMA-SP-G, and RHMA-O pavement when finish rolling is complete. Sand must be free of clay or organic matter. Sand must comply with section 90-1.02C(3). Keep traffic off the pavement until spreading sand is complete.

39-1.03E(4) Method Compaction

Pave HMA in maximum 0.25-foot thick compacted layers.

If the surface to be paved is both in sunlight and shade, pavement surface temperatures are taken in the shade.

Spread HMA-SP (Type A) only if atmospheric and surface temperatures are:

Minimum Atmospheric and Surface Temperatures

Compacted Layer Thickness, feet	Atmospheric,°F		Surface,°F	
	Unmodified Asphalt Binder	Modified Asphalt Binder[a]	Unmodified Asphalt Binder	Modified Asphalt Binder[a]
< 0.15	55	50	60	55
0.15 – 0.25	45	45	50	50

[a]Except asphalt rubber binder.

If the asphalt binder for HMA-SP (Type A) is:

1. Unmodified asphalt binder, complete:
   1.1. First coverage of breakdown compaction before the surface temperature drops below 250 degrees F
   1.2. Breakdown and intermediate compaction before the surface temperature drops below 190 degrees F
   1.3. Finish compaction before the surface temperature drops below 150 degrees F
2. Modified asphalt binder, complete:
   2.1. First coverage of breakdown compaction before the surface temperature drops below 240 degrees F
   2.2. Breakdown and intermediate compaction before the surface temperature drops below 180 degrees F
   2.3. Finish compaction before the surface temperature drops below 140 degrees F

For RHMA-SP-G:

1. Only spread and compact if the atmospheric temperature is at least 55 degrees F and the surface temperature is at least 60 degrees F.
2. Complete the first coverage of breakdown compaction before the surface temperature drops below 285 degrees F.
3. Complete breakdown and intermediate compaction before the surface temperature drops below 250 degrees F.
4. Complete finish compaction before the surface temperature drops below 200 degrees F.
5. If the atmospheric temperature is below 70 degrees F, cover loads in trucks with tarpaulins. The tarpaulins must completely cover the exposed load until you transfer the mixture to the paver's hopper or to the pavement surface.

For HMA-O with unmodified asphalt binder:

1. Only spread and compact if the atmospheric temperature is at least 55 degrees F and the surface temperature is at least 60 degrees F.
2. Complete first coverage using 2 rollers before the surface temperature drops below 240 degrees F.
3. Complete all compaction before the surface temperature drops below 200 degrees F.
4. If the atmospheric temperature is below 70 degrees F, cover loads in trucks with tarpaulins. The tarpaulins must completely cover the exposed load until you transfer the mixture to the paver's hopper or to the pavement surface.

For HMA-O with modified asphalt binder except asphalt rubber binder:

1. Only spread and compact if the atmospheric temperature is at least 50 degrees F and the surface temperature is at least 50 degrees F.
2. Complete first coverage using 2 rollers before the surface temperature drops below 240 degrees F.
3. Complete all compaction before the surface temperature drops below 180 degrees F.
4. If the atmospheric temperature is below 70 degrees F, cover loads in trucks with tarpaulins. The tarpaulins must completely cover the exposed load until you transfer the mixture to the paver's hopper or to the pavement surface.

For RHMA-O:

1. Only spread and compact if the atmospheric temperature is at least 55 degrees F and surface temperature is at least 60 degrees F.
2 Complete the 1st coverage using 2 rollers before the surface temperature drops below 280 degrees F.
3. Complete compaction before the surface temperature drops below 250 degrees F.
4. If the atmospheric temperature is below 70 degrees F, cover loads in trucks with tarpaulins. The tarpaulins must completely cover the exposed load until the mixture is transferred to the paver's hopper or to the pavement surface.

For RHMA-SP-G and OGFC, tarpaulins are not required if the time from discharge to truck until transfer to the paver's hopper or the pavement surface is less than 30 minutes.

HMA compaction coverage is the number of passes needed to cover the paving width. A pass is 1 roller's movement parallel to the paving in either direction. Overlapping passes are part of the coverage being made and are not a subsequent coverage. Do not start a coverage until completing the prior coverage.

Start rolling at the lower edge and progress toward the highest part.

Perform breakdown compaction of each layer of HMA-SP (Type A), and RHMA-SP-G with 3 coverages using a vibratory roller. The speed of the vibratory roller in miles per hour must not exceed the vibrations per minute divided by 1,000. If the HMA-SP (Type A) or RHMA-SP-G layer thickness is less than 0.08 foot, turn the vibrator off.

The Engineer may order fewer coverages if the HMA-SP (Type A), or RHMA-SP-G layer thickness is less than 0.15 foot.

The Engineer may order fewer coverages if the layer thickness is less than 0.20 foot.

Perform intermediate compaction of each layer of HMA-SP (Type A) and RHMA-SP-G with 3 coverages using a pneumatic-tired roller at a speed not to exceed 5 mph.

Perform finish compaction of HMA-SP (Type A) and RHMA-SP-G with 1 coverage using a steel-tired roller.

Compact OGFC with 2 coverages using steel-tired rollers.

39-1.03F Rumble Strips

Construct rumble strips in the top layer of HMA surfacing by ground-in or rolled-in methods.

Choose between ground-in or rolled-in rumble strips.

Select the method and equipment for constructing ground-in indentations.

Do not construct rumble strips on structures or approach slabs.

Construct rumble strips within 2 inches of the specified alignment. The grinding equipment must be equipped with a sighting device enabling the operator to maintain the rumble strip alignment.

Indentations must comply with the specified dimensions within 0.06 inch in depth and 10 percent in length and width.

The Engineer orders grinding or removal and replacement of noncompliant rumble strips to bring them within specified tolerances. Ground surface areas must be neat and uniform in appearance.

The grinding equipment must be equipped with a vacuum attachment to remove residue from the roadbed.

Dispose of removed material.

On ground areas, apply fog seal coat under section 37-2.

39-1.03G Vertical Joints

Do not leave a vertical joint more than 0.15 foot high between adjacent lanes open to traffic.

Before opening the lane to public traffic, pave shoulders and median borders adjacent to a lane being paved.

Place HMA on adjacent traveled way lanes so that at the end of each work shift the distance between the ends of HMA layers on adjacent lanes is between 5 to 10 feet. Place additional HMA along the transverse edge at each lane's end and along the exposed longitudinal edges between adjacent lanes. Hand rake and compact the additional HMA to form temporary conforms. You may place Kraft paper or another approved bond breaker under the conform tapers to facilitate the taper removal when paving operations resume.

39-1.03H Tapered Notched Wedge

For divided highways with an HMA lift thickness greater than 0.15 foot, you may construct a 1-foot wide tapered notched wedge joint as a longitudinal joint between adjacent lanes open to traffic. A vertical notch of 0.75 inch maximum must be placed at the top and bottom of the tapered wedge.

The tapered notched wedge must retain its shape while exposed to traffic. Pave the adjacent lane within 1 day.

Construct the tapered portion of the tapered notched wedge with an authorized strike-off device. The strike-off device must provide a uniform slope and must not restrict the main screed of the paver.

You may use a device attached to the screed to construct longitudinal joints that will form a tapered notched wedge in a single pass. The tapered notched wedge must be compacted to a minimum of 91 percent compaction.

Perform QC testing on the completed tapered notch wedge joint as follows:

1. Perform field compaction tests at the rate of 1 test for each 750-foot section along the joint. Select random locations for testing within each 750-foot section.
2. Perform field compaction tests at the centerline of the joint, 6 inches from the upper vertical notch, after the adjacent lane is placed and before opening the pavement to traffic.
3. Determine maximum density test results.
4. Determine percent compaction of the longitudinal joint as the ratio of the average of the field compaction values and the maximum density test results.

For acceptance of the completed tapered notch wedge joint, take two 4- or 6-inch diameter cores 6 inches from the upper vertical notch of the completed longitudinal joint for every 3,000 feet at locations designated by the Engineer. Take cores after the adjacent lane is placed and before opening the pavement to traffic. Cores must be taken in the presence of the Engineer and must be marked to identify the test sites. Submit the cores. One core will be used for determination of the field density and 1 core will be used for dispute resolution. The Engineer determines:

1. Field compaction by measuring the bulk specific gravity of the cores under AASHTO T 275, Method A
2. Percent compaction as the ratio of the average of the bulk specific gravity of the core for each day's production to the maximum density test value

in the Engineer's verification testing and in the computation of any quality factor and process control.

Determine percent compaction values each day the joint is completed and submit values within 24 hours of testing. If the percent compaction of 1 day's production is less than 91 percent, that day’s notched wedge joint is rejected. Discontinue placement of the tapered notched wedge and notify the Engineer of changes you will make to your construction process in order to meet the specifications.

39-1.03I Shoulders, Medians, and Other Road Connections

Until the adjoining through lane's top layer has been paved, do not pave the top layer of:

1. Shoulders
2. Tapers
3. Transitions

4. Road connections
5. Driveways
6. Curve widenings
7. Chain control lanes
8. Turnouts
9. Turn pockets

If the number of lanes changes, pave each through lane's top layer before paving a tapering lane's top layer. Simultaneous to paving a through lane's top layer, you may pave an adjoining area's top layer, including shoulders. Do not operate spreading equipment on any area's top layer until completing final compaction.

39-1.03J Widening

Not Used

39-1.03K Edge Treatment

The edge of roadway where the safety edge treatment is to be placed must have a solid base, free of debris such as loose material, grass, weeds, or mud. Grade areas to receive the safety edge as required.

The safety edge treatment must be placed monolithic with the adjacent lane or shoulder and shaped and compacted with a device attached to the paver.

The device must be capable of shaping and compacting HMA to the required cross section as shown. Compaction must be by constraining the HMA to reduce the cross sectional area by 10 to 15 percent. The device must produce a uniform surface texture without tearing, shoving, or gouging and must not leave marks such as ridges and indentations. The device must be capable of transition to cross roads, driveways, and obstructions.

For safety edge treatment, the angle of the slope must not deviate by more than ± 5 degrees from the angle shown. Measure the angle from the plane of the adjacent finished pavement surface.

If paving is done in multiple lifts, the safety edge treatment must be placed with each lift.

Short sections of hand work are allowed to construct transitions for safety edge treatment.

39-1.03L Conform Tapers

Place shoulder conform tapers concurrently with the adjacent lane's paving.

39-1.04 PAYMENT

The weight of each HMA mixture shown in the Bid Item List is the combined mixture weight.

If tack coat, asphalt binder, and asphaltic emulsion are paid as separate bid items, their bid items are measured under section 92 or section 94.

If recorded batch weights are printed automatically, the bid item for HMA is measured by using the printed batch weights, provided:

1. Total aggregate and supplemental fine aggregate weight per batch is printed. If supplemental fine aggregate is weighed cumulatively with the aggregate, the total aggregate batch weight must include the supplemental fine aggregate weight.
2. Total asphalt binder weight per batch is printed.
3. Each truckload's zero tolerance weight is printed before weighing the first batch and after weighing the last batch.
4. Time, date, mix number, load number and truck identification is correlated with a load slip.
5. Copy of the recorded batch weights is certified by a licensed weigh master and submitted.

Place hot mix asphalt dike of the type specified is measured along the completed length.

Place hot mix asphalt (miscellaneous areas) is measured as the in-place compacted area.

If replace asphalt concrete surfacing is shown the bid item for replace asphalt concrete is measured based on the specified dimensions and any adjustments ordered.

HMA dike is paid for as place hot mix asphalt dike of the type specified in the Bid Item List and by weight for hot mix asphalt.

HMA specified to be placed in miscellaneous areas is paid for as place hot mix asphalt (miscellaneous area) and by weight for hot mix asphalt.

If HMA (miscellaneous area) is shown, the bid item for place hot mix asphalt (miscellaneous area) is limited to the areas shown and is in addition to the bid items for the materials involved.

The Department does not adjust the unit price for an increase or decrease in the prepaving grinding day quantity.

If the Engineer determines more time is required for prepaving grinding than the Contract allows for and if prepaving grinding is a controlling activity, the Engineer makes a time adjustment.

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40 CONCRETE PAVEMENT

Add between the 1st and 2nd paragraphs in section 40-1.01C(7) of the RSS for section 40:

As an alternative to the inertial profiler and operator certification by the Department, equivalent Texas Transportation Institute certification is accepted if the certification is dated before July 1, 2013 and is not more than 12 months old.

Replace section 40-1.01C(8) of the RSS for section 40 with:

40-1.01C(8) Coefficient of Thermal Expansion

Submit 4 test specimens fabricated from a single sample of concrete for coefficient of thermal expansion testing under AASHTO T 336.

Submit your coefficient of thermal expansion test data at:

http://169.237.179.13/cte/

Replace "Reserved" in section 40-1.01D(3) of the RSS for section 40 with:

Your personnel required to attend the prepaving conference must also complete just-in-time-training (JITT) for individual precast slab replacement and precast jointed concrete pavement. Provide the facility for the training.

At least 5 business days before JITT, submit:

1. Instructor's name and listed experience
2. Training location
3. One copy of:
   - 3.1. Course syllabus
   - 3.2. Handouts
   - 3.3. Presentation materials

The Engineer provides training evaluation forms, and each attendee must complete them 5 business days after JITT, submit completed training evaluation forms to the Engineer and the electronic mailbox address:

Construction_Engineering_HQ@dot.ca.gov

JITT may be an extension of the prepaving conference and must be:

1. At least 4 hours long
2. Conducted at a mutually agreed place

3. Completed at least 20 days before you start paving activities
4. Conducted during normal working hours

JITT instructor must be on Approved Precast JITT Instructor List available at:

http://www.dot.ca.gov/hq/maint/Pavement/Offices/Pavement_Engineering/index.html. Upon JITT completion, the instructor must issue a certificate of completion to each participant.

The Department reimburses you for 1/2 of the cost for providing the JITT. The Engineer determines the costs under section 9-1.04 except no markups are added. Costs include training materials; facility use; and the JITT instructor's wages including the instructor's travel, lodging, meals and presentation materials. The Department does not pay your costs for attending JITT.

Replace section 40-1.01D(7)(a)(v) of the RSS for section 40 with:

40-1.01D(7)(a)(v) Coefficient of Thermal Expansion Testing

Test for coefficient of thermal expansion under AASHTO T 336. Test at field qualification and at a frequency of 1 test for each 5,000 cu yd of paving but not less than 1 test for projects with less than 5,000 cu yd of concrete. This test is Not Used for acceptance.

Replace section 40-8 of the RSS for section 40 with:

PRECAST JOINTED CONCRETE PAVEMENT

40-8.01 GENERAL

40-8.01A Summary

Section 40-8 includes specifications for furnishing and installing precast jointed concrete pavement (PJCP). Furnishing includes pretensioning PJCP before shipping to the jobsite. Installing includes grading PJCP in place, furnishing and installing dowel bars at joints between PJCP and the existing pavement, and sealing non-compliant joints.

Before submitting shop drawings, field verify the geometry described.

For horizontal curves and transition to horizontal curves, PJCP must match the superelevation and superelevation transition of the roadway.

Install dowel bars at joints between PJCP and the existing concrete in compliance with the requirement in either section 41-8 or “Dowel Bar Slots” of these special provisions.

40-8.01B Definitions

system: consists of features that are part of design, construction, and performance of the precast pavement product. These features are included, but are not limited to size and thickness of panels and slabs, joints, elements cast in the panels, load transfer method, techniques for providing grade control for the panels, etc.

40-8.01CSubmittals

40-8.01C(1) General

Section 40-1.01C does not apply.

Section 50-1.01C(3) does not apply.

In addition to the requirements in section 90-4, submit PJCP shop drawings to the Engineer and to Office of Concrete Pavement and Pavement Foundation, MS # 5, 5900 Folsom Boulevard, Sacramento, CA 95819. Submit 4 sets to the Engineer and 1 set to the Office of Concrete Pavement and Pavement Foundation. Allow 15 business days for review.

Shop drawings must include:

1. Details for furnishing PJCP panels including:
   1.1. Panel layout and coordinating panel identification system
   1.2. Your survey notes for field verification of the existing profile and grade information described
   1.3. Your adjusted panel dimensions including calculations for:
      1.3.1. Pavement profile with any superelevation or transition
      1.3.2. Horizontal curve locations
      1.3.3. Allowable fabrication tolerances
      1.3.4. Allowable installation tolerances
   1.4. Method for casting connection elements within the specified alignment for final placement
   1.5. Underslab grout vents locations on the details shown for panels.
   1.6. Finishing methods and procedures
   1.7. Methods and procedures for handling and transport
2. Details, methods and procedures for installing PJCP panels including:
   2.1. Methods and procedures for handling panels
   2.2. Methods and procedures for supporting and adjusting grade of the PJCP during installation
   2.3. Methods and procedures for installing joint filler, joints, and joint seals
   2.4. Details and methods for connecting to the existing pavement
3. Detail method for repairing damaged during removal from the forms or transport and installation

Submit separate prestressing shop drawings for pretensioning PJCP before shipping to the jobsite. Shop drawings must comply with section 50-1.01C(3) except (1) the 6th and 8th paragraphs do not apply, and (2) do not send them to the OSD.

40-8.01C(2) Alternative System

PJCP specifications and plans are provided as the basic design. You may submit a request to use alternative system. Shop drawing must demonstrate compliance with the following requirements for alternative system:

1. Precast panels with no prestressing must have:
   1.1. One or two layers of reinforcement.
   1.2. Maximum dimension (width or length) equal to or less than 15 feet.
   1.3. Thickness in compliance with the requirements for cast-in-place concrete pavement, unless justification is made to reduce the thickness
2. Load transfer between panels must be provided by dowel bars installed using one of the following methods (dowel bar retrofitting is not acceptable):
   2.1. Underslab slots
   2.2. Narrow slots
   2.3. Side holes for sliding dowels
3. Panels with a width or length greater than 15 feet must be pretensioned in that direction. Panels may be pretensioned in both directions.
4. At locations of superelevation transition for curves with radii smaller than 2000’, either:
   4.1. Use proprietary warped or folded panels
   4.2. Use cast-in-place concrete pavement (if allowed by engineer)
5. Drop-in panels must be reinforced or pre-tensioned and at least 8 feet long.
6. Panels must be set to grade using one of the following methods, or any other method that provides acceptable results:
   6.1. Shimming
   6.2. Treated bedding layer (must be used when there is no risk of erosion)
   6.3 Leveling bolts
   6.4. Grading brackets
   6.5. Grading beams
7. When truck lanes are not cast together (monolithic), the panel edge must be supported or the longitudinal joint must have a system to prevent relative vertical displacement of panels. This could be, but is not limited to:
   7.1. Additional reinforcement at the longitudinal edges
   7.2. Key and keyway
   7.3. Dual keyway with shear key
   7.4. Tie bar retrofit

8. Bond breaker must comply with section 36-2.if applicable.
9. You could use synthetic fibers in concrete mix, provided that fibers do not adversely impact panel surface final finishing and grindings.

For alternate system approval, the process consists of two parts. In Part 1, the system designer shall prepare and submit specification, fabrication drawings, and installation procedures. Part 2 of the approval process will consist of construction and evaluation of a trial installation. Final approval and use as an approved system will be given once the specification, fabrication drawings and installation procedures have been completed, the trial panels have been installed in accordance with the approved process and the Department has determined the trial installation is successful.

If you choose to use patented components in the PJCP, you must pay any applicable royalties.

Any new idea or process proposed for use as part of the alternative design which is approved and allowed on the job will become property of the Department and no patent could be filed for that idea or process.

40-8.01D Quality Control and Assurance

40-8.01D(1) General

Section 40-1.01D does not apply.

Test the coefficient of thermal expansion for each mix design.

Provide a QC manager.

Arrange for a prepaving conference facility and hold the conference after submitting the shop drawings, and 10 business days before beginning installation activities including test strip. Discuss methods of performing the installation work.

At the minimum, the meeting must be attended by your:

1. Project superintendent
2. QC manager
3. Installation construction foreman
4. Subcontractor's workers including:
   4.1. Fabricator’s project manager
   4.2. Personnel responsible for saw cutting, underslab grouting and joint sealing

40-8.01D(2) Quality Control Testing

Construct pavement test strips and obtain authorization of your test strips before starting other paving work. Test strips must comply with the authorized shop drawings and be:

1. At least 300 feet long
2. Same width as shown on the authorized shop drawings
3. Same cross-section dimensions as for the highest rate of superelevation as shown on the authorized shop drawings

Notify the Engineer at least 25 working days before you start test strip installation.

Allow 3 business days for test strip review.

Test strip is rejected if:

1. Panels do not comply with alignment criteria
2. Before grouting, there are voids more than 1/4 inch between base and bottom of panel
3. Joint width exceeds its acceptable tolerance
4. Surface varies more than 0.02 foot from a 12-foot straightedge's lower edge
5. Wheel path's individual high points are greater than 0.025 foot in 25 feet
6. Final finishing does not comply with the specifications except coefficient of friction
7. Excess grout flows from under the panel into joints, dowel slots or onto the pavement.

If the test strip complies with the acceptance criteria except for the coefficient of friction, you may grind the test strip under section 42. If the test strip complies with the acceptance criteria after grinding, you may request to leave the test strip in place.

If the test strip does not comply with the panel alignment criteria, submit revised shop drawings that include your proposed changes to correct the alignment. After the revised submittals are authorized, install a new test strip. Repeat this process until the test strip complies with the acceptance criteria.

Remove and dispose of rejected test strips.

Construct additional test strips if you change:

1. Methods and equipment including:
   1.1. Fabrication plants
   1.2. Panel lifting, shipment, and delivery methods
   1.3. Grouting equipment
   1.4. Connections to the existing pavement
2. Base layer preparation method
3. Grout mix for under the slab
4. Panel leveling methods

If you have successfully installed PJCP on a previous Department project and used the same fabrication plant, installation equipment and procedures, and personnel, you may request authorization to start precasting without a test strip. Your request must include supporting documentation from the previous Department project.

40-8.01D(3) Pavement Acceptance

40-8.01D(3)(a) General

Construct PJCP panels to the dimensions shown on the authorized shop drawings. PJCP panels are rejected if the fabricated dimensions are not within the tolerances shown in the following table:

PJCP Panel Fabrication

Dimension	Tolerances
Length (longer dimension)	+/- 1/4 inch
Width (shorter dimension)	+/- 1/8 inch
Nominal thickness	+/- 1/16 inch
Edge alignment straightness measured from a horizontal plane	+/- 1/8 inch
Skew at the ends	+/- 1/8 inch
Batter	+/- 1/16 inch
Position of pre-tensioning strands	+/- 1/8 inch, vertical[a]
	+/- 1/8 inch, horizontal
Diagonal difference of the corner to corner measurement	+/- 1/8 inch
Position of lifting anchors	+/- 3 inches

[a]Measured from the bottom of the panel

The profile and grade of the finished PJCP must match the existing pavement including any superelevation and superelevation transition. PJCP panels are rejected if the installed alignments are not within the tolerances shown in the following table:

PJCP Panel Installation

Alignment	Tolerances
Vertical at transverse joints	+/- 1/4 inch
Vertical at longitudinal joints	+/- 1/4 inch
Horizontal at transverse joints	+/- 1/8 inch
Horizontal at longitudinal joints	+/- 1/4 inch

40-8.02 MATERIALS

40-8.02A General

Proportion grout for under slab grouting under ASTM C938 or use prepackaged grout complying with ASTM C1107. Fine aggregate, if used, must meet grading 2 in ASTM C637. Proportion the ingredients of the grout to meet the following properties:

Quality Characteristic	Test method	Requirement
Strength at 1 hr: @73°F @45°F	ASTM C942	 2500 PSI Min 2000 PSI Min
Strength at 7 days @73°F @45°F	ASTM C942	 7600 PSI Min 6000 PSI Min
Expansion	ASTM C940	0 to 3%
Bleeding at 30 min	ASTM C940	0.1% Max
Eflux Time	ASTM C939	15 to 30 seconds
Grout Bond Strength, bond to dry PCC	CT 551	150 PSI Min in 24 hr
Note: For tests at 45°F condition materials to 45°F for 24 hr before fabrication and store specimens at 45°F until testing		

Polyester concrete must comply with section 41-1.

Tack coat must comply with section 39.

For panels with traffic loop detectors, use glass fiber reinforced polymer (GFRP) in compliance with section 40-1.02C of these special provisions

40-8.02B Prestressing

Prestressing must comply with section 50-1.02.

Transverse pretensioning strand must be either 0.5 or 0.6 inch diameter and comply with ASTM Designation: A 416, Grade 270 (low relaxation).

40-8.02C Expansion Cap

Expansion cap must comply with section 41-8.

40-8.02D Joint Filler

Joint filler material must comply with ASTM D1742 or ASTM D7174, and must be compatible with polyester concrete.

40-8.03 CONSTRUCTION

40-8.03A General

Prestressing must comply with section 50-1.03. The specification for a “member” applies to a PJCP panel.

Where existing pavement is replaced with PJCP, replace only the portion of pavement where the work will be completed during the same lane closure. If installation of the PJCP is not completed during the same lane closure, comply with the specifications for temporary roadway pavement structure in section 41-9.

40-8.03B Furnishing PJCP

If the roadway alignment is on a curve with a radius less than 2, 500 feet, place the reinforcement along a single plane. If the curve does not allow the spacing shown between transverse bar reinforcement and prestress, space them a distance that is between one half the specified spacing and the specified spacing. Before casting, grease the section of dowels extending into the expansion sleeve.

Lifting devises must be recessed at least 1/2 inch below the panel surface and a distance of at least 0.2L from any panel edge.

After casting and before curing, comply with section 40-1.03M.

Section 40-1.03N does not apply. Cure PJCP under section 90-4.

Section 40-1.03O does not apply.

PJCP must have a minimum compressive strength of 4,000 psi prior to the time of releasing the pretensioning strands or moving PJCP panels. Before shipping, PJCP must have a minimum compressive strength of 6,000 psi and must have cured for at least 14 days.

40-8.03C Installing PJCP

40-8.03C(1) General

The existing concrete pavement at transverse or longitudinal joints may be saw cut to a maximum of 3 inches from the joint in order to provide a straight edge for a uniform conform.

Grade the new base or existing base and remove loose and unstable material. If leveling course is used, limit the maximum thickness of the layer to 2”.

Before installing PJCP, clean and dry the surface of the base and place the bond breaker on it. The bond breaker must be free of wrinkles and overlapped at least 6 inches in the same direction as the panel installation.

Before installing PJCP panels, attach joint filler across the full length and depth of the transverse and longitudinal joint faces, except at joint keys and keyways. Place the top of the joint filler flush with the top edge of the panel and extend it to the bottom edge of the panel. Secure joint filler to the panel face and prevent it from moving, curling, tearing and other damage. Repair or replace damaged joint filler.

At dowel slots, joint filler installation must comply with caulking and foam inserts requirement.

When truck lanes are not cast together (monolithic), the panel edge must be supported or the longitudinal joint must have a system to prevent relative vertical displacement of panels. Use additional reinforcement at the longitudinal edges, key and keyway, dual keyway with shear key, or tie bar retrofit.

40-8.03C(2) Backfilling

Fill dowel bar slots in the PJCP and existing concrete pavement as soon as possible after panel placement and prior to opening to traffic. Fill dowel bar slots with polyester concrete in accordance with section 41-8, or ”Dowel Bar Slots” in these special provisions.

Backfill voids or recessions in the panels related to lifting anchors with polyester concrete.

40-8.03C(3) Grouting

Before grouting, fill dowel bar slots and shear keys with polyester concrete. Consolidate the polyester concrete in place and finish the surface to match the pavement surface. If cracks appear, replace the concrete.

If dowel bar slots and shear keys are unable to be filled prior to opening to traffic, install temporary covers to withstand traffic loading. Fasten the temporary covers to the panels so they are not disturbed or dislodged by traffic and are flush with the surface of the pavement. Do not leave temporary covers at any given location for more than 48 hours. Do not use other filler materials before final filling. Clean slots before filling.

Inject grout under section 41-2.01C and 41-2.03, except do not drill holes, or inject water into holes. Complete underslab grouting prior to opening to traffic.

Backfill any gaps that remain between PJCP and existing AC to remain in place with underslab grout.

40-8.03D Panel Repair

Repair panels damaged during removal from forms and handling in compliance with section 41-4, except polyester concrete must be used. Repairs are required when any surface of the panel is damaged and will affect ride quality, assembly of the panels, or long-term performance of the pavement. Treat the cracks that do not extend to the full depth of a panel with a high molecular weight methacrylate resin in compliance with section 41--3” Crack Treatment".

Seal joints that have a width greater than ¼ inch and less than ¾ inch with preformed compression seals in compliance with section 41-5. For joint widths between ¾” and 1 ½”, use the details shown on the plans. Remove and replace the panels when joint width is more than 1 ½”.

40-8.04 PAYMENT

Precast jointed concrete pavement is measured based on the dimensions shown.

If the Engineer accepts a test strip for use as roadway, the test strip is paid for as precast jointed concrete pavement.

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41 CONCRETE PAVEMENT REPAIR

INDIVIDUAL PRECAST SLAB REPLACEMENT

41-12.01 GENERAL

41-12.01A Summary

Section 41-12 includes specifications for furnishing and installing precast concrete panels (PCP) for individual slab replacement (ISR) as individual precast slab replacement (IPSR). Furnishing includes pretensioning individual pavement panels before casting. Comply with section 40-1 except where noted in these special provisions.

Before submitting shop drawings, field verify the geometry described,

Use dowel bars at connections between IPSR and the existing concrete. Comply with section 41-8, 41-10 , or “Dowel Bar Slots” of these special provisions.

41-12.01B Definitions

system: consists of features that are part of design, construction, and performance of the precast pavement product. These features are included, but are not limited to size and thickness of panels and slabs, joints, elements cast in the panels, load transfer method, techniques for providing grade control for the panels, etc.

41-12.01C Submittals

41-12.01C(1) General

Section 40-1.01C does not apply.

Section 50-1.01C(3) does not apply.

In addition to the requirements in section 90-4, submit IPSR shop drawings to the Engineer and to Office of Concrete Pavement and Pavement Foundation, MS # 5, 5900 Folsom Boulevard, Sacramento, CA 95819. Submit 4 sets to the Engineer and 1 set to the Office of Concrete Pavement and Pavement Foundation. Allow 15 business days for review.

Shop drawings must include:

1. Details for furnishing PCP including:
   - 1.1. Panel layout and coordinating panel identification system
   - 1.2. Panel material
   - 1.3. Complete details and substantiating calculations of the method and materials proposed for use in the pretensioned activities, including the addition or rearrangement of reinforcing steel
   - 1.4. Your adjusted panel dimensions including calculations for:
     - 1.4.1 Allowable fabrication tolerances
     - 1.4.2. Allowable installation tolerances
   - 1.5. Method for casting connection elements within the specified alignment for final placement
   - 1.6. Grout vent locations on the details shown
   - 1.7. Finishing methods and procedures

1.8. Methods and procedures for handling and transport
2. Details, methods and procedures for installing PCP panels including:
   2.1. Methods and procedures for handling panels
   2.2. Methods and procedures for supporting and adjusting grade of the PCP during installation
   2.3. Methods and procedures for installing joint filler, joints, and joint seals
   2.4. Details and methods for connecting to the existing pavement
3. Detail method for repairing damaged during removal from the forms or transport and installation.

The shop drawings must comply with section 50-1.02, except (1) the 3rd and 6th paragraphs do not apply, and (2) do not send them to the Office of Structural Materials.

41-12.01C(2) Alternative Design

In lieu of the design shown on the project plans, you may propose an alternative design which has different panel dimensions within the limits specified in this specification, is conventionally reinforced or pretensioned in two directions, and has a different arrangement for load transfer using dowel bars.

For alternate design, the thickness of panels must not be less than 0.65 feet. The maximum width or length must not exceed 36 feet. Panels must be pretensioned or reinforced; however, pretensioning is required for any alternate panel design with length or width greater than 15 feet.

If an alternative design is proposed, it must be submitted in writing to the Engineer. The Engineer has 20 business days to review and evaluate the alternate design after a complete plan has been received. The Engineer approves shop drawings for the alternate design based on requirements under Submittals. Do not perform precasting until the alternate design is approved by the Engineer.

In addition to the above information listed for shop drawings, the alternate design submittal must include:

1. Location and position of threaded lifting inserts and grout capping methods.
2. A rearrangement of pretensioning and/or reinforcing steel. Bar reinforcement details must meet or exceed the Department's design.
3. Panel connection and joint sealant details.
4. Dowel bar slot and installation details at all transverse construction joints.
5. Manufacturer's installation instructions.

Any new idea or process proposed for use as part of the alternative design which is approved and allowed on the job will become property of the Department and no patent could be filed for that idea or process.

41-12.01C(3) System Approval

Final approval and use on the project will be given once the Trial Panels have been successfully completed and the fabrication and installation details have been completed to the satisfaction of the Engineer.

For alternate system approval, the process consists of two parts. In Part 1, the system designer shall prepare and submit specification, fabrication drawings, and installation procedures. Part 2 of the approval process will consist of construction and evaluation of a trial installation. Final approval and use as an approved system will be given once the specification, fabrication drawings and installation procedures have been completed, the trial panels have been installed in accordance with the approved process and the Department has determined the trial installation is successful.

41-12.01D Quality Control and Assurance

41-12.01D(1) General

Section 40-1.01D does not apply.

Test the coefficient of thermal expansion for each mix design.

Provide a QC manager.

Arrange for a prepaving conference facility and hold the conference after submitting the shop drawings, and 10 business days before beginning installation activities including trial panels. Discuss methods of performing the installation work including:

1. An outline procedure for transportation of PCP.
2. An outline procedure for installation and testing of PCP including trial panels.
3. A contingency plan for correcting problems in transportation and installation of PCP. Include the quantity and location of standby material in your contingency plan.
4. A contingency plan for correcting problems with existing base
5. Provisions for determining if IPSR must be suspended and temporary roadway pavement structure constructed.

At the minimum, the meeting must be attended by your:

1. Project superintendent
2. QC manager
3. Installation construction foreman
4. Subcontractor's workers including:
   4.1. Fabricator's project manager
   4.2. Personnel responsible for saw cutting, underslab grouting and joint sealing

41-12.01D(2) Quality Control Testing

Construct pavement trial panels and obtain authorization of your trial panels before starting paving work. Trial panels must comply with the authorized shop drawings and:

1. Be at a mutually-agreed location that is on the roadway within the project limits
2. Installed under similar traffic conditions and with the same equipment to be used for the planned installation, and under similar atmospheric and temperature conditions expected during replacement operations.
3. Be at least 12' wide and 8' long
4. Be the maximum panel thickness shown on shop drawings
5. Include two adjacent panels for each proposed panel design (e.g. pretensioned or reinforced)
6. For locations with three or more panels in a row, include location with highest superelevation transition

Notify the Engineer at least 25 working days before you start trial panel installation.

Allow 3 business days for trial panel review.

Trial panels are rejected if:

1. Before grouting, there are voids more than 1/2 inch between base and bottom of panel
2. Joint width exceeds 1-1/2 inch
3. Surface varies more than 0.02 foot from a 12-foot straightedge's lower edge
4. Final finishing does not comply with the specifications
5. Excess grout flows from under the panel into joints, dowel slots or onto the pavement.

If the trial panels comply with the acceptance criteria except for the coefficient of friction, you may grind the trial panels under section 42. If the trial panels comply with the acceptance criteria after grinding, you may request to leave the trial panels in place.

Remove and dispose of rejected trial panels.

Construct additional trial panels if you change:

1. Methods and equipment including:
   1.1. Fabrication plants
   1.2. Panel lifting, shipment, and delivery methods
   1.3. Grouting equipment
   1.4. Connections to the existing pavement

2. Grout mix for under the slab
3. Base material preparation methods
4. Panel leveling methods

If you have successfully installed IPSR on a previous Department project and used the same fabrication plant, installation equipment and procedures, and personnel, you may request authorization to start precasting without a trial panel installation. Your request must include supporting documentation from the previous Department project.

41-12.01D(3) Pavement Acceptance

41-12.01D(3)(a) General

Construct PCP to the dimensions shown on the authorized shop drawings. PCP is rejected if the fabricated dimensions are not within the tolerances shown in the following table:

PCP Fabrication

Dimension	Tolerances
Length (longer dimension)	+/- 1/4 inch
Width (shorter dimension)	+/- 1/4 inch
Nominal thickness	+/- 1/8 inch
Edge alignment straightness measured from a horizontal plane	+/- 1/8 inch
Skew at the ends	+/- 1/8 inch
Batter	+/- 1/8 inch
Position of pretensioning strands	+/- 1/4 inch, vertical[a]
	+/- 1/4 inch, horizontal
Diagonal difference of the corner to corner measurement	+/- 1/4 inch
Position of lifting anchors	+/- 6 inches

[a]Measured from the bottom of the panel

The profile and grade of the finished PCP must match the existing pavement including any superelevation and superelevation transition. PCP is rejected if the installed alignments are not within the tolerances shown in the following table:

PCP Installation

Alignment	Tolerances
Vertical at transverse joints	+/- 1/4 inch
Vertical at longitudinal joints	+/- 1/4 inch
Horizontal at transverse joints	+/- 1/4 inch
Horizontal at longitudinal joints	+/- 1/4 inch

41-12.02 MATERIALS

41-12.02A General

Bond breaker must be white polyethylene sheeting and at least 6-mils thick. Sheeting must comply with ASTM C 171.

Proportion grout for under slab grouting under ASTM C938 or use prepackaged grout complying with ASTM C1107. Fine aggregate, if used, must meet grading 2 in ASTM C637. Proportion the ingredients of the grout to meet the following properties:

Quality Characteristic	Test method	Requirement
Strength at 1 hr: @73°F @45°F	ASTM C942	 2500 PSI Min 2000 PSI Min
Strength at 7 days @73°F @45°F	ASTM C942	 7600 PSI Min 6000 PSI Min
Expansion at 30 min	ASTM C940	0 to 3%
Bleeding at 30 min	ASTM C940	0.1% Max
Eflux Time	ASTM C939	15 to 30 seconds
Grout Bond Strength, bond to dry PCC	CT 551	150 PSI Min in 24 hr
Note: For tests at 45°F condition materials to 45°F for 24 hr before fabrication and store specimens at 45°F until testing		

Polyester concrete must comply with section 41-1.

For panels with traffic loop detectors, use glass fiber reinforced polymer (GFRP) in compliance with section 40-1.02C of these special provisions.

41-12.02B Prestressing

Prestressing must comply with section 50-1.02.

Transverse pretensioning strand must be either 0.5 or 0.6 inch diameter, and comply with ASTM Designation: A 416, Grade 270 (low relaxation).

41-12.02C Joint Seal

Joint seal for transverse and longitudinal joints must be silicone and comply with section 41-5.

41-12.02D Expansion Cap

Expansion cap must comply with section 41-8.

41-12.02E Joint Filler

Joint filler material must comply with either ASTM D1752 or ASTM D7174 and must be compatible with polyester concrete.

41-12.03 CONSTRUCTION

41-12.03A General

Prestressing must comply with section 50-1.03. The specifications for a “member” apply to a PCP.

If installation of the IPSR is not completed during the same lane closure, comply with the specifications for temporary roadway pavement structure in section 41-9.

41-12.03B Furnishing PCP

If the roadway alignment is on a curve with a radius less than 2,500 feet, place the reinforcement along a single plane. If the curve does not allow the spacing shown between transverse bar reinforcement and prestress, space them a distance that is between one half the specified spacing and the specified spacing. Before casting, grease the section of dowel bars extending into the expansion sleeve.

Lifting devices must be recessed at least 1/2 inch below the panel surface and a distance of at least 0.2L from any panel edge.

After casting and before curing, comply with section 40-1.03M.

Section 40-1.03N does not apply. Cure PCP under section 90-4.

Section 40-1.03O does not apply.

PCP must have a minimum compressive strength of 4,000 psi prior to the time of releasing the pretensioning strands or moving PCP. Before shipping, PCP must have a minimum compressive strength of 6,000 psi and must have cured for at least 14 days.

41-12.03C Installing PCP

41-12.03C(1) General

The existing concrete pavement at longitudinal joints may be saw cut to a maximum of 3 inches from the joint in order to provide a straight edge for a uniform conform.

Grade the new base or existing base, and remove loose and unstable material. If leveling course is used, limit the maximum thickness of the layer to 2”.

Before installing PCP, clean and dry the surface of the base and place the bond breaker on it. The bond breaker must be free of wrinkles and overlapped at least 6 inches in the same direction as the panel installation.

Before installing PCP, attach joint filler across the full length and depth of the transverse and longitudinal panel faces. Place the top of the joint filler recessed below the top of the panel and extend it to the bottom edge of the panel. Secure joint filler to the panel face and prevent it from moving, curling, tearing and other damage. Repair or replace damaged joint filler.

At dowel slots, joint filler installation must comply with caulking and foam inserts requirement.

41-12.03C(2) Backfilling

Fill dowel bar slots in the PCP and existing concrete pavement as soon as possible after panel placement and prior to opening to traffic. Fill dowel bar slots with polyester concrete in accordance with "Dowel Bar Slots,” of these special provisions.

If you are not able to fill dowel bar slots prior to opening to traffic, cover the slots with temporary covers that withstand traffic loading. The temporary covers must be fastened to panels so that they are not disturbed by traffic and must be flush with the surface of the pavement. Do not leave temporary covers at any given location for more than 48 hours. Do not use other filler materials before final filling. Clean slots before filling.

Backfill voids or recessions in the panels related to lifting anchors with polyester concrete.

Before installing IPSR, attach joint filler across the full length and depth of the transverse and longitudinal joint faces. Place the top of the joint filler recessed below the top of the IPSR panel and extend it to the bottom edge of the panel. Secure joint filler to the joint face and prevent it from moving, curling, tearing and other damage. Repair or replace damaged joint filler. Comply with caulking and foam inserts at dowel slots.

41-12.03C(3) Grouting

Inject grout under section 41-2.01C and 41-2.03, except do not drill holes, or inject water into holes. Underslab grouting must occur prior to opening to traffic.

Backfill any gaps that remain between IPSR and existing AC to remain in place with underslab grout.

41-12.03C(4) Final Finishing

If the Engineer determines by visual inspection the final texturing may not comply with the specifications for coefficient of friction, the Engineer tests to determine coefficient of friction. Open the pavement to traffic and the Department tests for coefficient of friction. If pavement does not comply with the specifications for coefficient of friction, groove or grind the pavement under Section 42.

The Engineer tests for smoothness using a 12-foot straightedge placed parallel with and perpendicular to the centerline. Straightedge smoothness specifications do not apply to the pavement surface placed within 12 inches of existing concrete pavement except you must place a straightedge longitudinally with the midpoint coincident with the transverse construction joint. The concrete pavement surface must be within 0.02 foot of the straightedge's lower edge. Correct pavement at contact joints not in compliance with straightedge smoothness specifications within 48 hours of performing test by grinding.

41-12.03C(5) Joints

If joints are wider than 1/4 inch, seal the joints. Install joint filler before grinding. Seal joints after grinding is completed in compliance with section 41-5.

For joint widths between ¾" and 1 ½", use the details shown on the plans. Remove and replace the panels when joint width is more than 1 ½".

Clean the faces of joints and underlying base from loose material and contaminants.

41-12.03D Panel Repair

Repair panels damaged during removal from forms and handling in compliance with section 41-4, except polyester concrete must be used. Repairs are required when any surface of the panel or panel edges are damaged and will affect ride quality, assembly of the panels, or long-term performance of the pavement. Repairs to panel edges and panel surface must not protrude above the surface.

Treat the cracks that do not extend to the full depth of a panel with a high molecular weight methacrylate resin in compliance with section 41-3, except a grey sand cover must be used.

41-12.04 PAYMENT

Individual precast slab replacement is measured based on the dimensions shown.

If the Engineer accepts trial panels for use as roadway, the trial panels are paid for as individual precast slab replacement.

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DIVISION VI STRUCTURES

49 PILING

Replace"Reserved" in section 49-3.02A(4)(b) with:

Schedule and hold a preconstruction meeting for CIDH concrete pile construction (1) at least 5 business days after submitting the pile installation plan and (2) at least 10 days before the start of CIDH concrete pile construction. You must provide a facility for the meeting.

The meeting must include the Engineer, your representatives, and any subcontractors involved in CIDH concrete pile construction.

The purpose of this meeting is to:

1. Establish contacts and communication protocol between you and your representatives, any subcontractors, and the Engineer
2. Review the construction process, acceptance testing, and anomaly mitigation of CIDH concrete piles

The Engineer will conduct the meeting. Be prepared to discuss the following:

1. Pile placement plan, dry and wet
2. Acceptance testing, including gamma-gamma logging, cross-hole sonic logging, and coring
3. *Pile Design Data Form*
4. Mitigation process
5. Timeline and critical path activities
6. Structural, geotechnical, and corrosion design requirements
7. Future meetings, if necessary, for pile mitigation and pile mitigation plan review
8. Safety requirements, including Cal/OSHA and Tunnel Safety Orders

Add to section 49-3.02B(6)(c):

The synthetic slurry must be one of the materials shown in the following table:

Material	Manufacturer
SlurryPro CDP	KB INTERNATIONAL LLC 735 BOARD ST STE 209 CHATTANOOGA TN 37402 (423) 266-6964
Super Mud	PDS CO INC 105 W SHARP ST EL DORADO AR 71731 (870) 863-5707
Shore Pac GCV	CETCO CONSTRUCTION DRILLING PRODUCTS 2870 FORBS AVE HOFFMAN ESTATES IL 60192 (800) 527-9948
Terragel or Novagel Polymer	GEO-TECH SERVICES LLC 220 N. ZAPATA HWY STE 11A-449A LAREDO TX 78043 (210) 259-6386

Use synthetic slurries in compliance with the manufacturer's instructions. Synthetic slurries shown in the above table may not be appropriate for a given job site.

Synthetic slurries must comply with the Department's requirements for synthetic slurries to be included in the above table. The requirements are available from the Offices of Structure Design, P.O. Box 168041, MS# 9-4/11G, Sacramento, CA 95816-8041.

SlurryPro CDP synthetic slurry must comply with the requirements shown in the following table:

SLURRYPRO CDP

Property	Test	Value
Density During drilling Before final cleaning and immediately before placing concrete	Mud Weight (density), API 13B-1, section 1	 ≤ 67.0 pcf[a] ≤ 64.0 pcf[a]
Viscosity During drilling Before final cleaning and immediately before placing concrete	Marsh Funnel and Cup. API 13B-1, section 2.2	 50–120 sec/qt ≤ 70 sec/qt
pH	Glass electrode pH meter or pH paper	6.0–11.5
Sand content, percent by volume Before final cleaning and immediately before placing concrete	Sand, API 13B-1, section 5	≤ 0.5 percent

[a]If authorized, you may use slurry in salt water. The allowable density of slurry in salt water may be increased by 2 pcf.
Slurry temperature must be at least 40 degrees F when tested.

Super Mud synthetic slurry must comply with the requirements shown in the following table:

SUPER MUD

Property	Test	Value
Density During drilling Before final cleaning and immediately before placing concrete	Mud Weight (Density), API 13B-1, section 1	 ≤ 64.0 pcf[a] ≤ 64.0 pcf[a]
Viscosity During drilling Before final cleaning and immediately before placing concrete	Marsh Funnel and Cup. API 13B-1, section 2.2	 32–60 sec/qt ≤ 60 sec/qt
pH	Glass electrode pH meter or pH paper	8.0–10.0
Sand content, percent by volume Before final cleaning and immediately before placing concrete	Sand, API 13B-1, section 5	≤ 0.5 percent

[a]If authorized, you may use slurry in salt water. The allowable density of slurry in salt water may be increased by 2 pcf.
Slurry temperature must be at least 40 degrees F when tested.

Shore Pac GCV synthetic slurry must comply with the requirements shown in the following table:

SHORE PAC GCV

Property	Test	Value
Density During drilling Before final cleaning and immediately before placing concrete	Mud Weight (Density), API 13B-1, section 1	 ≤ 64.0 pcf[a] ≤ 64.0 pcf[a]
Viscosity During drilling Before final cleaning and immediately before placing concrete	Marsh Funnel and Cup. API 13B-1, section 2.2	 33–74 sec/qt ≤ 57 sec/qt
pH	Glass electrode pH meter or pH paper	8.0–11.0
Sand content, percent by volume Before final cleaning and immediately before placing concrete	Sand, API 13B-1, section 5	≤ 0.5 percent

[a]If authorized, you may use slurry in salt water. The allowable density of slurry in salt water may be increased by 2 pcf.
Slurry temperature must be at least 40 degrees F when tested.

Terragel or Novagel Polymer synthetic slurry must comply with the requirements shown in the following table:

TERRAGEL OR NOVAGEL POLYMER

Property	Test	Value
Density During drilling Before final cleaning and immediately before placing concrete	Mud Weight (Density), API 13B-1, section 1	 ≤ 67.0 pcf[a] ≤ 64.0 pcf[a]
Viscosity During drilling Before final cleaning and immediately before placing concrete	Marsh Funnel and Cup. API 13B-1, section 2.2	 45–104 sec/qt ≤ 104 sec/qt
pH	Glass electrode pH meter or pH paper	6.0–11.5
Sand content, percent by volume Before final cleaning and immediately before placing concrete	Sand, API 13B-1, section 5	≤ 0.5 percent

[a]If authorized, you may use slurry in salt water. The allowable density of slurry in salt water may be increased by 2 pcf.
Slurry temperature must be at least 40 degrees F when tested.

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51 CONCRETE STRUCTURES

Delete the 5th paragraph of the RSS for section 51-1.04.

Add to section 51-1.04:

The payment quantity for concrete barrier does not include the area of rustic rock barrier pattern on the concrete barrier.

Add to section 51-5.04:

The payment quantity for structure approach slab does not include the area of structure excavation (bridge) and structure backfill (bridge) shown below approach slabs.

Replace the 6th paragraph in section 51-5.03E with:

Except when using RSC, magnesium phosphate concrete, or modified high-alumina-based concrete, allow 12 hours between placing the paving notch extension and the approach slab.

Add to section 51-5.03E:

When using RSC, the 1-hour compressive strength must be at least 2,500 psi. Allow at least 1 hour between placing the paving notch extension and the approach slab.

When using magnesium phosphate concrete or modified high-alumina-based concrete, allow at least 1 hour between placing the paving notch extension and the approach slab.

Add to section 51-5.04:

Excavation and backfill at paving notch extensions must comply with section 19-3 and are included in the payment for paving notch extension.

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59 PAINTING

Add to section 59-6.03:

The painted concrete must match color no. 37925 of FED-STD-595.

Replace "Reserved" in section 59-13 with:

59-13 PREPARE AND CLEAN CONCRETE SURFACES

59-13.01 GENERAL

Section 59-13 includes specifications for preparing and cleaning concrete surfaces which contain existing paint.

59-13.02 MATERIALS

Not Used

59-13.03 CONSTRUCTION

Clean previously painted concrete surfaces under section 59-1.03C before performing any painting activities. Use any of the cleaning methods or combination of the methods under section 59-1.03C. Remove gloss from existing paint without removing sound paint. Remove paint that becomes loose, curled, lifted, or that loses its bond after cleaning to sound paint or concrete surface.

Any of the preparing and cleaning methods which damages concrete finish surfaces to be painted, in the opinion of Engineer, must not be used.

Removal of existing paint must comply with section 14-11.08.

Where existing lighting systems have been removed, provide a Class 1 Surface Finish.

59-13.04 PAYMENT

Not Used

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83-1.02B(1)(a)(iii) Submittals

Submit a mix design for the minor concrete to be used. The mix design must show proportions of:

1. Coarse aggregate
2. Fine aggregate
3. Cementitious material
4. Reinforcing fiber
5 Water

Include compressive strength test results with your mix design.

Submit the quantity in pounds of crumb rubber aggregate with your certificate of compliance for crumb rubber aggregate if used.

83-1.02B(1)(a)(iv) Quality Control and Assurance

Not Used

83-1.02B(1)(b) Materials

83-1.02B(1)(b)(i) General

Not Used

83-1.02B(1)(b)(ii) Minor Concrete

Minor concrete must include reinforcing fibers and may include crumb rubber aggregate.

Section 90-2.02B does not apply. Minor concrete must contain at least:

1. 505 pounds of cementitious material per cubic yard if crumb rubber aggregate is used
2. 400 pounds of cementitious material per cubic yard if crumb rubber aggregate is Not Used

The 3rd paragraph of section 90-2.02C does not apply. Minor concrete must have a maximum aggregate size of 3/8 inch.

All ingredients must be added at the concrete plant before delivery to the job site.

You may use volumetric proportioning under ASTM C 685/C 685M or section 90-3.02B.

Minor concrete must have a 28-day compressive strength from 1,400 to 1,800 psi.

83-1.02B(1)(b)(iii) Crumb Rubber Aggregate

Crumb rubber aggregate must consist of ground or granulated scrap tire rubber from automobile and truck tires. Tire buffings are not allowed. Crumb rubber aggregate must be ground and granulated at ambient temperature.

The gradation of the crumb rubber aggregate must comply with the requirements shown in the following table:

Gradation Requirements

Sieve size	Percentage passing
1/2"	100
3/8"	90–100
1/4"	35–45
No. 4	5–15
No. 8	0–5
No. 16	0

Crumb rubber aggregate must not contain more than 0.01 percent of wire by mass of crumb rubber and must be free of oils and volatile organic compounds.

Commingling of crumb rubber from different sources is not allowed.

The crumb rubber aggregate must be 3.5 ± 0.5 percent by weight of the concrete.

83-1.02B(1)(b)(iv) Reinforcing Fibers

Reinforcing fibers for minor concrete must be:

1. Manufactured specifically for use as concrete reinforcement from one of the following:
   1.1. Polypropylene, polyethylene, or a combination of both.
   1.2. Copolymer of polypropylene and polyethylene.
2. Blended ratio from 4 to 5.67 parts by weight of macro synthetic fibers to 1 part by weight of micro synthetic fibers. Synthetic fibers must be:
   2.1. Nonfibrillated macro fibers with individual fiber lengths less than 2 ± 1/2 inch.
   2.2. Fibrillated or monofilament micro fibers of various lengths and thicknesses.
3. Supplied in sealed, degradable bags of appropriate size for adding whole bags to concrete batches.
4. From a commercial source.

The reinforcing fiber content of minor concrete must be from 5 to 6 lb/cu yd.

83-1.02B(1)(b)(v) Coloring Agent

If a color for concrete is specified in section 83-1.02B(1)(b)(i), the coloring agent must be integral to the concrete mix and added at the concrete plant.

83-1.02B(1)(b)(vi) Block-Out Material

Use a commercially available expanded polystyrene foam for the block-out material. The expanded polystyrene foam must have a compressive strength of 13 ± 5 psi at 10 percent deformation when tested under ASTM D1621.

You may substitute an alternative material that meets the compressive strength requirements of the expanded polystyrene foam if authorized.

83-1.02B(1)(c) Construction

83-1.02B(1)(c)(i) General

Areas to receive vegetation control must be cleared of vegetation, trash, and debris. Dispose of removed material.

83-1.02B(1)(c)(ii) Earthwork

Excavate areas to receive vegetation control. Where vegetation control abuts the existing surfacing, the edge of the existing surfacing must be on a neat line or must be cut on a neat line to a minimum depth of 2 inches before removing the surfacing. The finished elevation of the excavated area to receive vegetation control must maintain planned flow lines, slope gradients, and contours of the job site.

Grade areas to receive vegetation control to a smooth, uniform surface and compact to a relative compaction of not less than 95 percent.

Dispose of surplus excavated material uniformly along the adjacent roadway except as specified in section 14-11.

83-1.02B(1)(c)(iii) Block Out

If block-out material is supplied in more than 1 piece, tape the pieces together to make a smooth surface on the top and sides.

Ensure block-out material does not move during concrete placement.

83-1.02B(1)(c)(iv) Placing Minor Concrete

Place minor concrete for vegetation control by hand.

Strike off and compact minor concrete with a mechanical or vibratory screed device. Apply a broom finish. Match the finished grade to the adjacent section of vegetation control, pavement, shoulder, or existing grade.

If the curing compound method is used for colored concrete, use curing compound no. 6.

83-1.02B(1)(d) Payment

Not Used

Replace item 1 in the 7th paragraph of section 83-1.02B with:

1. Wood posts

Replace item 2 in the 7th paragraph of section 83-1.02B with:

2. Wood blocks for line posts

Add to section 83-1.02B:

The offset from the face of the Type WB-31 transition railing to the hinge point must be a minimum of 3'-6".

The offset from the face of the adjacent midwest guardrail system to the hinge point must be transitioned from the offset at the Type WB-31 transition railing to 4'-0" using a ratio of 6:1.

Replace "Reserved" in section 83-1.02B(1) with:

83-1.02B(1)(a) General

Section 83-1.02B(1) includes specifications for constructing vegetation control areas around guardrail posts with minor concrete.

Submit the mix design for the minor concrete to be used for vegetation control. Include compressive strength test results with your mix design.

Submit the quantity in pounds of crumb rubber with your certificate of compliance for crumb rubber aggregate if used.

83-1.02B(1)(b) Materials

83-1.02B(1)(b)(i) General

Not Used

83-1.02B(1)(b)(ii) Minor Concrete

Minor concrete must include reinforcing fibers and may include crumb rubber aggregate.

Section 90-2.02B does not apply. Minor concrete must contain at least:

1. 505 pounds of cementitious material per cubic yard if crumb rubber aggregate is used
2. 400 pounds of cementitious material per cubic yard if crumb rubber aggregate is Not Used

The 3rd paragraph of section 90-2.02C does not apply. Minor concrete must have a maximum aggregate size of 3/8 inch.

You may use volumetric proportioning under ASTM C 685/C 685M or section 90-3.02B.

Minor concrete must have a 28-day compressive strength from 1,400 to 1,800 psi.

83-1.02B(1)(b)(iii) Crumb Rubber Aggregate

Crumb rubber must consist of ground or granulated scrap tire rubber from automobile and truck tires. Tire buffings are not allowed. Crumb rubber aggregate must be ground and granulated at ambient temperature.

The gradation of the crumb rubber aggregate must meet the requirements of the following table:

Gradation Requirements

Sieve size	Percentage passing
1/2"	100
3/8"	90–100
1/4"	35–45
No. 4	5–15
No. 8	0–5
No. 16	0

Crumb rubber aggregate must not contain more than 0.01 percent of wire by mass of crumb rubber and must be free of oils and volatile organic compounds.

Commingling of crumb rubber from different sources is not allowed.

The crumb rubber aggregate must be 3.5 ± 0.5 percent by weight of the concrete.

83-1.02B(1)(b)(iv) Reinforcing Fibers

Reinforcing fibers for minor concrete must be:

1. Polypropylene fibers with an engineered sinusoidal contoured profile manufactured specifically for use as concrete reinforcement.
2. Blended ratio of 4 parts by weight of coarse monofilament fibers with maximum individual fiber lengths of 2 ± 1/2 inch and 1 part by weight of fine fibrillated polypropylene fibers of various lengths and thicknesses. If the coarse and fine reinforcing fibers are supplied by the same manufacturer, they may be premixed in a sealed 5 lb degradable bag.
3. From a commercial source.
4. Concrete ingredient as described in your mix design and as recommended by the manufacturer.

The reinforcing fiber content of minor concrete must be 5 lb/cu yd.

83-1.02B(1)(b)(v) Coloring Agent

If a color for concrete is specified in section 83-1.02B(1)(b)(i), the coloring agent must be integral to the concrete mix and added at the concrete plant.

If the curing compound method is used, use curing compound no. 6.

83-1.02B(1)(b)(vi) Block-Out Material

Use a commercially-available expanded polystyrene foam for the block-out material. The expanded polystyrene foam must have a compressive strength of 13 ± 5 psi at 10 percent deformation when tested under ASTM D1621.

You may substitute any appropriate material that meets the compressive strength requirements of the expanded polystyrene foam if authorized.

83-1.02B(1)(c) Construction

83-1.02B(1)(c)(i) General

Areas to receive vegetation control must be cleared of vegetation, trash, and debris. Dispose of removed material.

83-1.02B(1)(c)(ii) Earthwork

Excavate areas to receive vegetation control. Where vegetation control abuts the existing surfacing, the edge of the existing surfacing must be on a neat line or must be cut on a neat line to a minimum depth of 2 inches before removing the surfacing. The finished elevation of the excavated area to receive vegetation control must maintain planned flow lines, slope gradients, and contours of the job site.

Grade areas to receive vegetation control to a smooth, uniform surface and compact to a relative compaction of not less than 95 percent.

Dispose of surplus excavated material uniformly along the adjacent roadway except as specified in section 14-11.

83-1.02B(1)(c)(iii) Block Out

Install block-out material as shown.

If block-out material is supplied in more than 1 piece, tape the pieces together to make a smooth surface on the top and sides.

Ensure block-out material does not move during concrete placement.

83-1.02B(1)(c)(iv) Placing Minor Concrete

Place minor concrete for vegetation control by hand.

Strike off and compact minor concrete with a mechanical or vibratory screed device. Apply a broom finish. Match the finished grade to the adjacent section of vegetation control, pavement, shoulder, or existing grade.

83-1.02B(1)(d) Payment

Vegetation control (minor concrete) is measured from the actual areas placed. The Department does not pay for vegetation control (minor concrete) placed outside the dimensions shown.

Replace section 83-1.02C(2) with:

83-1.02C(2) Alternative In-Line Terminal System

Alternative in-line terminal system must be furnished and installed as shown on the plans and under these special provisions.

The allowable alternatives for an in-line terminal system must consist of one of the following or a Department-authorized equal.

1. TYPE SKT-MGS TERMINAL SYSTEM - Type SKT-MGS terminal system must be a SKT 350 sequential kinking terminal, system length 53'-1-1/2", manufactured by Road Systems, Inc., located in Big Spring, Texas, and must include items detailed for Type SKT-MGS terminal system shown on the plans. The SKT 350 sequential kinking terminal can be obtained from the distributor, Universal Industrial Sales, P.O. Box 699, Pleasant Grove, UT 84062, telephone (801) 785–0505 or from the distributor, Gregory Highway Products, 4100 13th Street, S.W., Canton, OH 44708, telephone (330) 477–4800.
2. TYPE ET-31 TERMINAL SYSTEM - Type ET-31 terminal system must be an ET-31 (4-tube system) extruder terminal, system length 53'-1-1/2", as manufactured by Trinity Highway Products, LLC, and must include items detailed for Type ET-31 terminal system shown on the plans. The ET-2000 PLUS (4-tube system) extruder terminal can be obtained from the manufacturer, Trinity Highway Products, LLC, P.O. Box 99, Centerville, UT 84012, telephone (800) 772–7976.
3. TYPE 31" X-TENSION - Type 31" X-Tension terminal system must be a 31" X-Tension Guard Rail End Terminal as manufactured by Barrier Systems, Inc., located in Vacaville, CA, and must include items detailed for Type 31" X-Tension terminal system shown on the plans. The 31" X-Tension Guard Rail End Terminal can be obtained from the distributor, Statewide Safety and Signs, Inc., 130 Grobric Court, Fairfield, CA 94533, telephone (800) 770–2644.

Submit a certificate of compliance for terminal systems.

Terminal systems must be installed under the manufacturer's installation instructions and these specifications. Each terminal system installed must be identified by painting the type of terminal system in neat black letters and figures 2 inches high on the backside of the rail element between system posts numbers 4 and 5. Paint must be metallic acrylic resin type spray paint. Before applying terminal system identification, the surface to receive terminal system identification must be removed of all dirt, grease, oil, salt, or other contaminants by washing the surface with detergent or other suitable cleaner. Rinse thoroughly with fresh water and allow to fully dry.

For Type ET-31 terminal system, install a hinged breakaway post at Post 1 and 6'-0" steel yielding terminal posts at Posts 2 through 6. The hinged breakaway post must be, at the Contractor's option, either driven, with or without pilot holes, or placed in drilled holes. If placed in a pilot or drilled hole, space

around the hinged breakaway post must be backfilled with selected earth, free of rock, placed in layers approximately 4 inches thick and each layer must be moistened and thoroughly compacted.

For Type SKT-MGS terminal system, install the soil tube with soil plate attached at Post 1, hinged breakaway post at Post 2, and 6'-0" W6 x 9 steel posts at Posts 3 through 8. Use a W6 x 15 steel post at Post 1. The soil tube with soil plate must be, at the Contractor's option, driven with or without pilot holes, or placed in drilled holes. Space around the steel foundation tubes must be backfilled with selected earth, free of rock, placed in layers approximately 4 inches thick and each layer must be moistened and thoroughly compacted.

For Type ET-31 terminal system, install 4'-6" steel foundation tubes with soil plates attached or 6'-0" soil tubes at Posts 1 and 2. Install 6'-0" controlled release terminal posts at Posts 3 through 6. The steel foundation tubes must be, at the Contractor's option, either driven, with or without pilot holes, or placed in drilled holes. Space around the steel foundation tubes must be backfilled with selected earth, free of rock, placed in layers approximately 4 inches thick and each layer must be moistened and thoroughly compacted. The wood terminal posts must be inserted into the steel foundation tubes by hand and must not be driven. Before the wood terminal posts are inserted, the inside surfaces of the steel foundation tubes to receive the wood posts must be coated with a grease that will not melt or run at a temperature of 149 degrees F or less. The edges of the wood terminal posts may be slightly rounded to facilitate insertion of the post into the steel foundation tubes.

For Type SKT-MGS terminal system, install the soil tube with soil plate attached at Post 1, breakaway cable terminal post at Post 2, and controlled release terminal posts at Posts 3 through 8. The soil tube must be, at the Contractor's option, driven with or without pilot holes, or placed in a drilled hole. Space around the steel foundation tube must be backfilled with selected earth, free of rock, placed in layers approximately 4 inches thick and each layer must be moistened and thoroughly compacted. A wood post must be inserted into the steel foundation tube by hand. Before the wood terminal post is inserted, the inside surfaces of the steel foundation tube to receive the wood post must be coated with a grease that will not melt or run at a temperature of 149 degrees F or less. The edge of the wood post may be slightly rounded to facilitate insertion of the post into the steel foundation tube.

For Type 31" X-Tension terminal system, the steel post and soil anchor must be, at the Contractor’s option, driven with or without pilot holes, or placed in drilled holes. Space around the steel post and soil anchor must be backfilled with selected earth, free of rock, placed in layers approximately 4 inches thick and each layer must be moistened and thoroughly compacted. The wood terminal posts must be inserted into the drilled holes by hand and backfilled in the same manner as the steel post and soil anchor. Wood terminal posts must not be driven. All blocks must be wood or plastic.

For Type 31" X-Tension terminal system, the steel bottom post and I-beam post must be placed in drilled hole. The soil anchor and steel line posts must be, at the Contractor's option, either driven or placed in drilled holes. Space around the steel bottom post, steel line posts and soil anchor must be backfilled with selected earth, free of rock, placed in layers approximately 4 inches thick and each layer must be moistened and thoroughly compacted. All blocks must be plastic.

After installing the terminal system, dispose of surplus excavated material in a uniform manner along the adjacent roadway where designated by the Engineer.

Replace section 83-1.02C(3) with:

83-1.02C(3) Alternative Flared Terminal System

Alternative flared terminal system must be furnished and installed as shown on the plans and under these special provisions.

The allowable alternatives for a flared terminal system must consist of one of the following or a Department-authorized equal.

1. TYPE FLEAT-MGS TERMINAL SYSTEM - Type FLEAT-MGS terminal system must be a Flared Energy Absorbing Terminal 350, system length 37'-6", manufactured by Road Systems, Inc., located in Big Spring, Texas, and must include items detailed for Type FLEAT-MGS terminal system shown on the plans. The Flared Energy Absorbing Terminal 350 can be obtained from the distributor, Universal Industrial Sales, P.O. Box 699, Pleasant Grove, UT 84062, telephone (801) 785–0505 or

from the distributor, Gregory Industries, Inc., 4100 13th Street, S.W., Canton, OH 44708, telephone (330) 477–4800.

2. TYPE SRT-31 TERMINAL SYSTEM - Type SRT-31 terminal system must be an SRT-350 Slotted Rail Terminal (6-post system), system length 37'-6", as manufactured by Trinity Highway Products, LLC, and must include items detailed for Type SRT-31 terminal system shown on the plans. The SRT-350 Slotted Rail Terminal (6-post system) can be obtained from the manufacturer, Trinity Highway Products, LLC, P.O. Box 99, Centerville, UT 84012, telephone (800) 772–7976.
3. TYPE 31" X-TENSION - Type 31" X-Tension terminal system must be a 31" X-Tension Guard Rail End Terminal as manufactured by Barrier Systems, Inc., located in Vacaville, CA, and must include items detailed for Type 31" X-Tension terminal system shown on the plans. The 31" X-Tension Guard Rail End Terminal can be obtained from the distributor, Statewide Safety and Signs, Inc., 130 Grobric Court, Fairfield, CA 94533, telephone (800) 770–2644.

Submit a certificate of compliance for terminal systems.

Terminal systems must be installed under the manufacturer's installation instructions and these specifications. Each terminal system installed must be identified by painting the type of terminal system in neat black letters and figures 2 inches high on the backside of the rail element between system posts numbers 4 and 5. Paint must be metallic acrylic resin type spray paint. Before applying terminal system identification, the surface to receive terminal system identification must be removed of all dirt, grease, oil, salt, or other contaminants by washing the surface with detergent or other suitable cleaner. Rinse thoroughly with fresh water and allow to fully dry.

For Type SRT-31 terminal system, install a cable release post at Post 1 and 6'-0" steel yielding terminal posts at Posts 2 through 6. The cable release post and steel yielding terminal posts must be, at the Contractor's option, either driven, with or without pilot holes, or placed in drilled holes. If placed in pilot or drilled holes, space around the posts must be backfilled with selected earth, free of rock, placed in layers approximately 4 inches thick and each layer must be moistened and thoroughly compacted.

For Type FLEAT-MGS terminal system, install the soil tube with soil plate attached at Post 1, hinged breakaway post at Post 2, and 6'-0" W6 x 9 steel posts at Posts 3 through 7. Use a W6 x 15 steel post at Post 1. The soil tube with soil plate must be, at the Contractor's option, driven with or without pilot holes, or placed in drilled holes. Space around the steel foundation tubes must be backfilled with selected earth, free of rock, placed in layers approximately 4 inches thick and each layer must be moistened and thoroughly compacted.

For Type FLEAT-MGS terminal system, install the soil tubes with soil plate attached at Posts 1 and 2, breakaway cable terminal posts at Posts 1 and 2, and controlled release terminal posts at Posts 3 through 6. The soil tubes with soil plates must be, at the Contractor's option, driven with or without pilot holes, or placed in drilled holes. Space around the steel foundation tubes must be backfilled with selected earth, free of rock, placed in layers approximately 4 inches thick and each layer must be moistened and thoroughly compacted. The breakaway cable terminal posts must be inserted into the steel foundation tubes by hand and must not be driven.

For Type 31" X-Tension terminal system, the steel post and soil anchor must be, at the Contractor’s option, driven with or without pilot holes, or placed in drilled holes. Space around the steel post and soil anchor must be backfilled with selected earth, free of rock, placed in layers approximately 4 inches thick and each layer must be moistened and thoroughly compacted. The wood terminal posts must be inserted into the drilled holes by hand and backfilled in the same manner as the steel post and soil anchor. Wood terminal posts must not be driven. All blocks must be wood or plastic.

For Type 31" X-Tension terminal system, the steel bottom post and I-beam post must be placed in drilled hole. The soil anchor and steel line posts must be, at the Contractor's option, either driven or placed in drilled holes. Space around the steel bottom post, steel line posts and soil anchor must be backfilled with selected earth, free of rock, placed in layers approximately 4 inches thick and each layer must be moistened and thoroughly compacted. All blocks must be plastic.

After installing the terminal system, dispose of surplus excavated material in a uniform manner along the adjacent roadway where designated by the Engineer.

Replace section 83-2.02E(4) with:

83-2.02E(4) Type SCI SMART Crash Cushion

Type SCI SMART 100 GM crash cushion must be installed where shown.

Type SCI SMART 100 GM crash cushion and additional components must comply with the descriptions shown in the following table:

Bid item description	Manufacturer's product description
Crash cushion (Type SCI SMART 100 GM)	SCI 100 GM

The successful bidder can obtain from the following distributors the Type SCI SMART 100 GM crash cushion manufactured by SCI Products, Inc., 2500 Production Drive, St. Charles, IL 60174, telephone (800) 327-4417:

1. D&M Traffic Services, Inc., 845 Reed Street, Santa Clara, CA 95050, telephone (408) 436-1127, FAX (408) 436-1675
2. Southern California: Traffic Control Service, Inc., 1818 E. Orangethorpe, Fullerton, CA 92831-5324, telephone (714) 526–9500, FAX (714) 526–9561

The price quoted by the manufacturer for Type SCI SMART 100 GM crash cushion, is $20,387.00, not including sales tax.

The above prices will be firm for orders placed within 30 days of Contract award, and provided delivery is accepted within 90 days after the order is placed.

Install the crash cushion (Type SCI SMART 100 GM) under the manufacturer's instructions.

Concrete anchorage devices used for attaching the crash cushion to the base slab must be limited to those that have been provided by the manufacturer.

The concrete anchor slab must comply with sections 51 and 52.

The concrete anchor slab must be constructed of concrete containing not less than 590 pounds of cementitious material per cubic yard.

Submit a copy of the manufacturer's plan and parts list, for each model installed, as an informational submittal.

Submit a certificate of compliance for each model of Type SCI SMART 100 GM crash cushion.

Payment for structure excavation, structure backfill, and concrete anchor slab with bar reinforcing steel is included in the payment for crash cushion (Type SCI SMART 100 GM).

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86 ELECTRICAL SYSTEMS

Add to the end of the 1st paragraph of the RSS for section 86-1.01:

This work is shown on plan sheets labeled E, EE, and SES. The work involved in each bid item is shown on a sheet with a title matching the bid item description except for the following bid items:

1. Maintaining the existing traffic management system during construction

Replace "Reserved" in section 86-1.06B with:

Traffic Management System (TMS) elements include, but are not limited to ramp metering (RM) system, communication system, traffic monitoring stations, video image vehicle detection system (VIVDS), microwave vehicle detection system (MVDS), loop detection system, changeable message sign (CMS) system, extinguishable message sign (EMS) system, highway advisory radio (HAR) system, closed circuit television (CCTV) camera system, roadway weather information system (RWIS), visibility sensor, and fiber optic system.

If the construction activities require existing TMS elements to be nonoperational or off line, and if temporary or portable TMS elements are not shown, provide for temporary or portable TMS elements. Obtain authorization on the type of temporary or portable TMS elements and installation method.

Fiber optic system as-built drawings will be available as an *Information Handout* as specified in section 2-1.06B.

Before work, the Engineer, the Department's Traffic Operations Electrical representatives, and you must jointly conduct a pre-construction operational status check of all existing TMS elements and each element's communication status with the Traffic Management Center (TMC), including existing TMS elements not shown and elements that may not be impacted by your activities. The Department's Traffic Operations Electrical representatives will certify the TMS elements' location and status, and provide a copy of the certified list of the existing TMS elements within the project limits to you. The status list will include the operational, defined as having full functionality, and the nonoperational components.

Obtain authorization at least 72 hours before interrupting existing TMS elements' communication with the TMC that will result in the elements being nonoperational or off line. Notify the Engineer at least 72 hours before starting excavation.

Traffic monitoring stations and their associated communication systems, which were verified to be operational during the pre-construction operational status check, must remain operational on freeway/highway mainline, except:

1. For a duration of up to 15 days on any continuous segment of the freeway/highway longer than 3 miles
2. For a duration of up to 60 days on any continuous segment of the freeway/highway shorter than 3 miles

If the construction activities require existing detection systems to be nonoperational or off line for a longer time period or the spacing between traffic monitoring stations is more than the specified criteria, and temporary or portable detection operations are not shown, provide provisions for temporary or portable detection operations. You must receive authorization on the type of detection and installation before installing the temporary or portable detection.

If existing TMS elements shown or identified during the pre-construction operational status check, except traffic monitoring stations, are damaged or fail due to your activity, where the elements are not fully functional, immediately notify the Engineer. If the Engineer notifies you that existing TMS elements have been damaged, have failed or are not fully functional due to your activity, the damaged or failed TMS elements, excluding structure-related elements, must be repaired or replaced, at your expense, within 24 hours. For a structure-related elements, you must install temporary or portable TMS elements within 24 hours. For nonstructure-related TMS elements, the Engineer may authorize temporary or portable TMS elements for use during construction.

If fiber optic cables are damaged due to your activities, install new fiber optic cables from an original splice point or termination to an original splice point or termination, unless otherwise authorized. Fiber optic cable must be spliced at the splice vaults if available. The amount of new fiber optic cable slack in splice vaults and the number of new fiber optic cable splices must be equivalent to the amount of slack and number of splices existing before the damage. Fusion splicing is required.

Demonstrate that repaired or replaced elements operate equal to or better than the replaced equipment. If you fail to perform required repairs or replacement work, the Department may perform the repair or replacement work and the cost will be deducted from monies due.

A TMS element is considered nonoperational or off line for the duration of time that active communications with the TMC is disrupted, resulting in messages and commands not transmitted from or to the TMS element.

Provide provisions for replacing existing TMS elements within the project limits, including detection systems, that were not identified on the plans or during the pre-construction operational status check that became damaged due to your activities.

If the pre-construction operational status check identified existing TMS elements, you, the Engineer, and the Department's Traffic Operations Electrical representatives must jointly conduct a post construction operational status check of all existing TMS elements and each element's communication status with the TMC. The Department's Traffic Operations Electrical representatives will certify the TMS elements' status and provide you a copy of the certified list of the existing TMS elements within the project limits. The status list will include the operational, defined as having full functionality, and the nonoperational components. TMS elements that cease to be functional between pre and post construction status checks must be repaired at your expense.

The Engineer will authorize the schedule for final replacement, the replacement methods and the replacement elements, including element types and installation methods before repair or replacement work is performed. The final TMS elements must be new and of equal or better quality than the existing TMS elements.

If no electrical work exists on the project and no TMS elements are identified within the project limits, the pre-construction operational status check is change order work.

Furnishing and installing temporary or portable TMS elements that are not shown, but are required when an existing TMS element becomes nonoperational or off line due to construction activities, is change order work.

Furnishing and installing temporary or portable TMS elements and replacing TMS elements that are not shown nor identified during the pre-construction operational status check and were damaged by construction activities is change order work.

If you are required to submit provisions for the replacement of TMS elements that were not identified, submitting the provisions is change order work.

Add to section 86-2.05A:

Conduit installed underground must be Type 1.

Add to section 86-2.05B:

The conduit in a foundation and between a foundation and the nearest pull box must be Type 1, except service conduit for Southern California Edison must be Type 3.

Add to section 86-2.05C:

If a standard coupling cannot be used for joining Type 1 conduit, use a UL-listed threaded union coupling under section 86-2.05C, a concrete-tight split coupling, or a concrete-tight set screw coupling.

If Type 3 conduit is placed in a trench, not in the pavement or under concrete sidewalk, after the bedding material is placed and the conduit is installed, backfill the trench to not less than 4 inches above the conduit with minor concrete under section 90-2, except the concrete must contain not less than 421 pounds of cementitious material per cubic yard. Backfill the remaining trench to finished grade with backfill material.

After conductors have been installed, the ends of the conduits terminating in pull boxes, service equipment enclosures, and controller cabinets must be sealed with an authorized type of sealing compound.

At those locations where conduit is required to be installed under pavement and underground facilities designated as high priority subsurface installation under Govt Code § 4216 et seq. exist, conduit must be placed by the trenching in pavement method under section 86-2.05C.

At other locations where conduit is required to be installed under pavement and if a delay to vehicles will not exceed 5 minutes, conduit may be installed by the trenching in pavement method.

Add to end of Section 86-2.05C:

Install tunnel control building conduit by the directional boring method with the authorization of the Engineer as shown.

Notify the Engineer 2 business days before starting directional boring operations.

Include the location and the equipment to be used in the advance notice to the Engineer.

Perform directional boring operations in the presence of the Engineer unless otherwise notified by the Engineer.

Conduit installed by the directional bore method must comply with section 86-2.05A.

Install conduit to a minimum depth of 3 ft below finished grade.

The diameter of the boring tool must be 1.5 times the outside diameter of the conduit. Use only mineral slurry or wetting solution to lubricate the boring tool and to stabilize the soil surrounding the boring path. Mineral slurry or wetting solution must be water based and environmentally safe.

Comply with Section 86-5.01A(4) for disposing residue from directional boring operations.

The directional boring equipment must have directional control of the boring tool and must have an electronic tool location detection system. During operation, the directional bore equipment must be able to determine the location of the tool both horizontally and vertically.

The directional boring equipment must be equipped with a tension measuring device that indicates the amount of tension exerted on conduit during conduit pulling operations.

Provide a layout and a profile plot to the Engineer showing the location of the bore to a resolution of 6 inches.

You must have direct charge and control of the directional bore operation at all times.

Do not use slurry cement backfill where the directional boring method is used.

Reform schedule 40 conduit with a mandrel after installation.

Replace the 3rd paragraph in section 86-2.06A(2) of the RSS for section 86-2.06 with:

In a ground or sidewalk area, embed the bottom of a pull box in crushed rock.

Replace "Reserved" in section 86-2.06B of the RSS for section 86-2.06 with:

86-2.06B(1) General

86-2.06B(1)(a) Summary

Section 86-2.06B includes specifications for installing non-traffic-rated pull boxes.

86-2.06B(1)(b) Submittals

Before shipping pull boxes to the job site, submit a list of materials used to fabricate the pull boxes to METS. Include:

1. Contract number
2. Manufacturer's name
3. Manufacturer's installation instructions
4. Your contact information

Submit reports for pull boxes from an NRTL-accredited laboratory.

Before installing a pull box and cover, submit the manufacturer's replacement warranty for them.

86-2.06B(1)(c) Quality Control and Assurance

86-2.06B(1)(c)(i) Functional Testing

The pull box and cover must be tested under ANSI/SCTE 77, "Specification for Underground Enclosure Integrity."

86-2.06B(1)(c)(ii) Warranty

Provide a 2-year manufacturer's replacement warranty for the pull box and cover. The warranty period starts on the date of Contract acceptance.

Deliver replacement parts within 5 business days after you receive notification of a failed pull box, cover, or both to the Department's Maintenance Electrical Shop at:

Burbank Electrical, 602 S. Flower St, Burbank, CA 91502 (for the area north of Mountain Ave)

San Gabriel Electrical, 9153 Lower Azusa Rd, Rosemead, CA 91770

86-2.06B(2) Materials

The pull box and cover must comply with ANSI/SCTE 77, "Specification for Underground Enclosure Integrity," for tier 22 load rating and must be gray or brown.

Each pull box cover must have an electronic marker cast inside.

A pull box extension must be made of the same material as the pull box and attached to the box to maintain the minimum combined depths.

Include recesses for a hanger if a transformer or other device must be placed in a pull box.

The bolts, nuts, and washers must be a captive design.

The captive bolt must be capable of withstanding a torque from 55 to 60 ft-lb and a minimum pull-out strength of 750 lb. Perform the test with the cover in place and the bolts torqued. The pull box and cover must not be damaged while performing the test.

Hardware must be stainless steel with 18 percent chromium and 8 percent nickel content.

Galvanize ferrous metal parts under section 75-1.05.

The manufacturer's instructions must include:

1. Quantity and size of entries that can be made without degrading the strength of the pull box below the tier 22 load rating
2. Locations where side entries cannot be made
3. Acceptable method for creating the entry

The tier 22 load rating must be labeled or stenciled by the manufacturer on the inside and outside of the pull box and on the underside of the cover.

86-2.06B(3) Construction

Do not install a pull box in curb ramps or driveways.

A pull box for a post or a pole standard must be located within 5 feet of the standard. Place the pull box adjacent to the back of the curb or edge of the shoulder. If this is impractical, place the pull box in a suitable, protected, and accessible location.

For pull boxes with a tamper resistant cover adjacent to lighting standards, install a 10 Amp fuse inside the pull box and a 3 Amp fuse in the circuit in the lighting standard handhole.

Add to the RSS section 86-2.06:

86-2.06D Tamper Resistant Cover for Non-Traffic Pull Box

86-2.06D(1) General

86-2.06D(1)(a) Summary

This work includes installing tamper resistant (TR) cover on pull box

86-2.06D(1)(b) Submittals

Before shipping TR cover and accessories to the job site, submit a list of materials, contract number, manufacturer's name, and manufacturer's instructions for installation.

Submit warranty documentation before installation.

86-2.06D(1)(c) Quality Control and Assurance

86-2.06D(1)(c)(i) Warranty

Provide a 2-year replacement warranty from the manufacturer of the TR cover and accessories against any defects or failures. The effective date of the warranty is the date of final acceptance.

Provide replacement parts within 5 business days after receipt of failed parts. The Department does not pay for replacement parts. Deliver replacement parts to the following Department's Maintenance Electrical Shop:

Burbank Electrical
602 S. Flower St.
Burbank, Ca 91502

for the area north of Mountain Avenue and:

San Gabriel Electrical
9153 Lower Azusa Rd.
Rosemead, Ca 91770

for the area south of Mountain Avenue.

86-2.06D(2) Materials

Provide the following:

1 A cover welded in factory, all around to security skirt sized to encase the pull box. The cover must be of steel tread plate. Cover must be of non skid surface. The security skirt must be steel
2. Cover must be marked for the application as shown
3. L shape steel anchor rod not less than 1 inch diameter by 4 feet long for the no. 5 pull box, 1 inch diameter by 5 feet long for no. 6 pull box
4. Top cap of stainless steel, steel hex nuts and lock nuts
5. Epoxy
6. Steel plate 3/16" x 3" x 8"

TR cover and accessories must be manufactured by one of the following companies or equal:

1. Factory Direct Fastening (FDF), 1608 A North Hillhurst Ave., Los Angeles, CA 90027. Telephone (800) 942-4844.
2. ERC, Inc, 2970 E Maria, Rancho Dominguez, CA 90221. Telephone (310) 603-2970.
3. Pendarvis Manufacturing, 1808 American St., Anaheim, CA 92801. Telephone (714) 992-0950.
4. Case Automation Corp, 5920 Rickenbacker Ave., Riverside, CA. 92504. Telephone (951) 202-7088 or (951) 493-6666.

TR cover manufactured by FDF is patented and royalty payments may apply.

Stainless steel hardware must have an 18 percent chromium content and an 8 percent nickel content.

Galvanize ferrous metal parts must comply with section 75-1.05.

Provide epoxy to fill the lock nut socket space. Epoxy must conform to Loc-tite No.# E-120HP or Scotch-weld # DP460 or Devcon Plus 25 #14278 or equal.

86-2.06D(3) Construction

Salvage pull box cover if installing TR cover on existing pull box.

Top of TR cover must be flush with final grade.

Install TR cover as follows:

1. Dig 8 inch diameter by 5 feet deep hole and install L shape steel anchor rod, set center of the pull box to coincide with the anchor rod. Include a provision for drain hole for the pull box
2. Install pull box over the steel anchor rod and conduits
3. Stabilizing and aligning the anchor rod vertical and concentric with the pull box
4. Pour concrete around anchor rod and outside pull box. Concrete outside pull box must be 7 inch below finished grade and the skirt will rest on this concrete
5. Bond and ground TR cover
6. Position the TR cover to encase the pull box. Secure TR cover to the anchor rod with penta head nut
7. Add epoxy to fill the lock nut socket space
8. Fix top cap
9. Tag-weld steel plate across the socket over the top cap lock nut

86-2.06D(4) Payment

Not Used.

Add to the RSS section 86-2.06

86-2.06E Tamper Resistant Cover For Traffic Rated Pull Box

86-2.06E(1) General

86-2.06E(1)(a) Summary

This work includes installing tamper resistant (TR) cover on traffic rated pull box.

86-2.06E(1)(b) Submittals

Before shipping TR cover and accessories to the job site, submit a list of materials, contract number, manufacturer's name, and manufacturer's instructions for installation.

Submit warranty documentation prior to installation.

86-2.06E(1)(c) Quality Control and Assurance

86-2.06E(1)(c)(1) Warranty

Provide a 2-year replacement warranty from the manufacturer of the TR cover and accessories against any defects or failures. The effective date of the warranty is the date of final acceptance.

86-2.06E(2) Materials

Provide the following:

1 The TR cover must be of steel tread plate reinforced with welded steel angle iron and welded soil anchor bolt well Cover must be 1/2 inch thick minimum and of non skid surface.
2. Cover must be marked for the application as shown.
3. J shape steel anchor bolt of not less than 1 inch diameter by 4 feet long for the No. 3 1/2 and 5 pull box, and 1 inch diameter by 5 feet long for No. 6 pull box.
4. Threaded stainless steel top cap with shoulder and 1/2 inch square ratchet hole.
5. Steel hex nut and lock nut.
6. Epoxy.

TR cover and accessories must be manufactured by one of the following companies or equal:

1. Factory Direct Fastening (FDF), 1608 A North Hillhurst Ave., Los Angeles, CA 90027. Telephone (800) 942-4844.
2. ERC, Inc, 2970 E Maria, Rancho Dominguez, CA 90221. Telephone (310) 603-2970.
3. Pendarvis Manufacturing, 1808 American St., Anaheim, CA 92801. Telephone (714) 992-0950.
4. Case Automation Corp, 5920 Rickenbacker Ave., Riverside, CA. 92504. Telephone (951) 202-7088 or (951) 493-6666.

Stainless steel hardware must have an 18 percent chromium content and an 8 percent nickel content.

Galvanize ferrous metal parts must comply with section 75-1.05.

Provide epoxy for application to the stainless steel top cap and threads. Epoxy must conform to Loc-tite #E-120HP or Scotch-weld # DP460 or Devcon Plus25 #14278 or equivalent.

86-2.06E(3) Construction

Top of TR cover must be flush with final grade.

Install TR cover as follows:

1. Prepare pull box location for installation of the pull box. Locate approximate center of the pull box and stub out conduits around this location.
2 Install concrete around and under the edges of the pull box and gravel to allow for drainage.
3 Dig 6 inch diameter by 4 feet deep hole at the central location of the pull box for installation of J shape galvanized steel anchor bolt.
4 Install pull box over the J shape steel anchor bolt and conduits. By using template set the pull box in a manner so the anchor bolt is in the center of the pull box. Include a provision for drain hole for the pull box.
5. Pour concrete around anchor bolt and outside the pull box. Finish concrete around the pull box level and sloping outward for water runoff.
6 Install 1-inch grout inside the pull box.
7. Provide appropriately sized 5 feet long wire and bond TR cover to the circuit equipment grounding conductor.
8. Secure TR cover to the anchor bolt with hexagonal nut lock nut.
9. Apply epoxy to the flat mating surface and threads of the stainless steel top cap and the pull box cover.
10. Insert top cap over the pull box cover and tighten the cap to 200 foot pound torque.

86-2.06E(4) Payment

Not Used.

Add to section 86-2.08A:

Secure conductors and cables to the projecting end of the conduit in pull boxes.

Replace the table in the 2nd paragraph of section 86-2.08C with:

Insulation Thickness

Insulation type	Conductor size	Insulation thickness (mils)
USE, RHH, or RHW	no. 14 to no. 10	45
	no. 8 to no. 2	60
THW or TW	no. 14 to no. 10	30
	no. 8	45
	no. 6 to no. 2	60

Replace the 1st paragraph of section 86-2.09E with:

Splices must be insulated by "Method B."

Delete the 6th and 7th paragraphs of section 86-2.09E.

Add to section 86-2.11A:

Continuous welding of exterior seams in service equipment enclosures is not required.

Each service must be provided with up to 2 main circuit breakers that will disconnect ungrounded service entrance conductors. Where the "Main" circuit breaker consists of 2 circuit breakers as described, each of the circuit breakers must have a minimum interrupting capacity of 10,000 A, rms.

Add to section 86-5.01A(1):

Loop wire must be Type 2.

Slots must be filled hot-melt rubberized asphalt sealant.

For Type E detector loops, sides of the slot must be vertical and the minimum radius of the slot entering and leaving the circular part of the loop must be 1-1/2 inches. Slot width must be a maximum of 5/8 inch. Loop wire for circular loops must be Type 2. Slots of circular loops must be filled with hot-melt rubberized asphalt sealant.

The depth of the loop sealant above the top of the uppermost loop wire in the sawed slots must be 2 inches, minimum.

Add to section 86-5:

86-5.01E Modify Automatic Vehicle Classification Station

86-5.01E(1) General

86-5.01E(1)(a) Summary

Section 86-5.01E includes specifications for installing automatic vehicle classification station (AVC) piezo-electric sensors and inductive loop detectors.

86-5.01E(1)(b) Submittals

You must provide 1 installation, operation, and service manual for each AVC provided in the contract.

Submit warranty documentation before installation.

86-5.01E(1)(c) Quality Control and Assurance

You must demonstrate that the automatic vehicle classification system is available for use by the Department by successfully completing the acceptance test for each lane of data collection.

The acceptance test must consist of the following:

1. Loop detectors must be tested according to the procedure in section 86-2.14B.
2. Piezo-electric axle sensors must be tested as follows:
   a. Capacitance must be 20% of the sensor's data sheet as provided by the manufacturer.
   b. Dissipation factor must be less than 0.04 when measured in the 20 nF range.
   c. Resistance must be greater than 20 MegOhms.
3. A minimum of 100 per-vehicle records must be collected for each lane. Collected data must meet the following accuracy standards:
   a. Total volume = +-3% with 95% confidence level
   b. Vehicle classification = 95% accurate classification by type.
4. Correct functioning of the communications link must be verified by collecting data files from the on-site equipment with the central office host computer.
5. Continuous operation of the automatic vehicle classification system on-site equipment for 5 consecutive days. Failure of the system to record and store data for an accumulated time exceeding 3 hours during the 5 day period must be cause for the acceptance test to be repeated.
6. Testing of the automatic vehicle classification system software during the above noted 5 day period and the full working day following the 5 day period. Failure of the software to perform any application must be cause for the acceptance test to be repeated.

Unavailability of any component must constitute the failure of the system to pass the acceptance test. Failure of the host computer, its peripheral equipment or of a communication line not furnished by You to transmit data may be considered not to render the system unavailable, if you demonstrate to the satisfaction of the Engineer that the failure is not caused by any of your furnished equipment.

86-5.01E(1)(c)(i) Warranty

Provide two-year manufacturer replacement warranty from the date of installation for automated vehicle classification station against any defects or failures. Manufacturer must provide replacement components within in 5 days after receipt of failed components at no cost to the Department.

A completed form will be returned to you for each AVC certifying that the system has been fully functional on the date specified.

Warranty's address and delivery of replacement equipment to the following department maintenance electrical shop:

620 South Flower Street, Burbank CA, 91502

86-5.01E(2) Materials

86-5.01E(2)(a) General

Modify Automatic vehicle classification (AVC) station must consist, in general, of replacing piezo-electric axle sensors, epoxy grout for axle sensor installation, inductive loop detectors, conductors and incidentals.

86-5.01E(2)(a)(i) Inductive Loop Detectors

Comply with section 86-5.01A.

Loop wire must be Type 2.

86-5.01E(2)(a)(ii) Piezo-Electric Axle Sensors

Piezo-electric axle sensors must be Class II and used for vehicle classification purposes. Piezo-electric axle sensors must consist of a piezo-electric copolymer surrounded by a 1/4 inch wide x 1/16 inch thick outer brass sheath. Each sensor must be 6 feet in length and come with a minimum of 200 feet of screened coaxial transmission cable (STC) attached.

86-5.01E(2)(a)(iii) Epoxy Grout

Epoxy grout must consist of a two component, self-leveling 100% solid polyurea. The grout must have a temperature range from 20 to 180 degrees F for application. The cured grout must have the following performance characteristics:

Specification	ASTM Designation	Requirement
Tensile Strength (PSI)	D412	2,950
Elongation (%)	D412	350
100% Modulus	D412	1,620
Tear Strength (PLI)	D412	500
Hardness (Shore A)	D2240	95A
Flexibility (1/8" Mandrel)	D1737	Pass
Flashpoint (F)		>200
Taber Abrasion (mg loss) CS18 Wheel 1 kg per 1,000 cycles	D4060	25
Gel Time		2 minutes
Tack Free Time		5 minutes
Open to Traffic Time		15 minutes

86-5.01E(3) Construction

86-5.01E(3)(a) General

You must deliver the automatic vehicle classification unit, piezo-electric axle sensors and epoxy grout to Caltrans District 7 Traffic Monitoring, 620 South Flower Street, Burbank, CA 91502, telephone number (323) 849-5028 for certification and testing at least 10 working days prior to installation:

86-5.01E(3)(b) Piezo-Electric Axle Sensors

The sensors are installed in an array of one inductive loop detector and two axle sensors per lane. A detailed installation procedure is provided in the project plans. The exact location of the inductive loop detector/piezo-electric axle sensor arrays will be determined by the Engineer.

Sawed pavement slots containing screened transmission cable must be filled with epoxy sealant conforming to section 95-2.09.

Shoulder termination stub-outs for loop detector wire and screened transmission cable must conform to Standard Plan ES-5D, "Electrical Systems (Detectors)". In PCC pavement, the roadway end of the conduit must be installed by core drilling.

86-5.01E(3)(c) Screened Transmission Cable

Splices in screened transmission cable runs must only be made in the first pull box on the outside shoulder using the following procedure:

1. Strip outer jacket off transmission cable, being careful not to cut braided shield.
2. Unbraid shield and neatly twist together.
3. Strip core wires and crimp together using butt splice. Fill butt splice with solder.
4. Crimp shield wires together using butt splice. Fill with solder.
5. Completely cover the splice area with electrical insulating coating and allow drying.
6. Insulate core and shield splices separately using 2 half-lapped layers of synthetic oil resistant, self fusing rubber electrical tape.
7. Overwrap entire splice with 3 half-lapped layers of PVC electrical tape. Cover entire splice with electrical insulating coating and allow to dry.

86-5.01E(4) Payment

Not Used

Add to section 86-5:

86-5.01F TEMPORARY MICROWAVE VEHICLE DETECTION SYSTEM

86-5.01F(1) General

86-5.01F(1)(a) Summary

This work consists of furnishing and installing a temporary microwave vehicle detection system (MVDS). Section 86-5.01F includes specifications for installing temporary microwave vehicle detection system.

86-5.01F(1)(b) Submittals

A list of materials that you propose to install for the MVDS, together with the drawings and other data, must be submitted under section 86-1.04. Additionally, the following must be provided before the completion of the Contract:

1. Certificate of Compliance - Submit a certificate of compliance for MVDS.
2. Site Analysis Report - Prior to MVDS installation, You must review each detection site and provide a written analysis recommending the optimum sensor placement for complying with the performance requirements of this special provision. The analysis must be reviewed and approved by the MVDS manufacturer.
3. Lane Configuration - The documentation must include a diagram that illustrates how the microwave beam is covering the traffic lanes as well as the MVDS connector pins or wire terminals that correspond to the respective lanes. The lanes must be identified by direction (i.e., NB, SB, EB, WB) and in order with lane one being the lane nearest to the center of the roadway.
4. Mounting and Wiring Information - You must provide to the Engineer for authorization 1 set of detailed diagrams showing wiring and service connections for each MVDS. The authorized diagrams must be covered separately on each side with clear self-adhesive plastic and placed in a heavy-duty plastic envelope. The envelope must be attached securely to the inside of the cabinet door or at a location ordered.
5. Communication Protocol - The MVDS communication protocol must be open and must be freely available for use in the public domain. You must provide documentation that defines the complete MVDS communication protocol. The documentation consists of a message structure organization, data packet length, and all information necessary to make use of the messages.
6. Remote Programming - You must provide all information and software necessary for operating the system from a remote Windows 2000-based or newer PC. This information and software must include at a minimum the capability to calibrate, tune, align, and program the MVDS and be provided on a CD compatible with Windows 2000-based or newer PC. The information must be formatted such that the files can be matched with the equipment being calibrated or aligned. This documentation must contain files that allow for replacement equipment to be loaded with the same configuration.
7. MVDS Accuracy Analysis - You must be responsible for conducting MVDS Performance Testing and must submit an MVDS accuracy analysis that complies with the requirements of the special provisions within 15 days of MVDS testing. The original video recordings as well as DVD or CD copies of the video images covering the analysis periods must be included.
8. Acceptance Testing Documentation - You must provide a test plan including the time and the period of the testing to be authorized. The test plan must be organized to allow the Engineer to perform acceptance testing by using the documentation and without assistance from you. You must collect and submit the data to be certified. If requested, the data must be collected in the presence of the Engineer.
9. Acceptance Testing Schedule - You must submit a testing schedule for authorization 15 days prior to acceptance testing of the MVDS. If the testing period extends beyond the normal working shift or if you fail to provide the necessary material for the testing within 1 hour of the scheduled testing start time, the Engineer may cancel the testing for the day.

86-5.01F(1)(c) Quality Control and Assurance

86-5.01F(1)(c)(i)

MVDSs must simultaneously provide vehicle detection data in the form of vehicle presence, volumes, counts, speed, classification, and occupancy for a minimum of 8 lanes of traffic and must comply with the performance requirements of the special provisions. MVDSs must provide a separate zone per lane and detect vehicles as close as 9.8 feet and as far as 197 feet from the MVDS sensor. MVDSs must monitor traffic lanes in the presence of barrier railings, guard railings, and other obstacles.

86-5.01F(1)(c)(ii)

MVDSs must comply with the following detection performance criteria when installed at a minimum of 9.8 feet from the nearest lane and at a minimum height of 16.4 feet above the roadway detection zone:

1. Average 5-minute volumes for all lanes combined with better than 95-percent accuracy compared to vehicles observed in video images for the same period for any 15-minute period selected by the Engineer.
2. Average 30-second volumes in every lane with better than 90-percent accuracy compared to vehicles observed in video images for the same period for any 5-minute period selected by the Engineer.
3. Average 30-second speed in any lane with better than 95-percent accuracy for any 5-minute period selected by the Engineer.
4. Average 5-minute occupancy for any lane with better than 85-percent accuracy for any 15-minute period selected by the Engineer.
5. Count accuracy, when compared to vehicles observed in video images for the same period, must be not less than 90 percent for any lane and not less than 95 percent for all lanes combined.
6. Average 15-minute classification according to user-defined criteria with better than 90-percent accuracy compared to vehicles observed in video images for the same period. Vehicle or length classification must be provided for categories of small car, average car, mid size car, long car, and extra-long car that are user definable by either length parameters, minimum length to maximum length for the category, or by a multiple of length of the average car.
7. You must provide the criteria for speed and volume acceptance testing for authorization. You must also provide speed and volume date for verification by the Engineer.

86-5.01F(1)(c)(iii)

The accuracy of each MVDS unit must be determined and documented so each unit may be authorized or rejected separately by the Engineer. Failure to submit the materials at the conclusion of testing invalidates the test. The recorded media serves as acceptance evidence and must not be used for calibration. The calibration must have been completed prior to testing and verification.

The MVDS must automatically restore normal operation following a power failure within 3 minutes and not require manual intervention. The MVDS must maintain the configuration and calibration information in nonvolatile memory and retain the information while powered off for at least 90 days.

The MVDS must be configurable for 30-second to 24-hour polling cycles and store vehicle count, speed, classification, and occupancy data in 10-second to not less than 15-minute intervals.

The MVDS must be tested and in standard production for a minimum of 3 months. You must not install any MVDS older than 6 months from the scheduled start date of the MVDS installation as indicated by date codes or serial numbers of electronic circuit assemblies.

The MVDS system and all supporting equipment must be designed to operate continuously in an outdoor traffic monitoring and control environment.You must assure that the MVDS will not cause harmful interference to radio communication in the area of the installation as required by 47 CFR 15. The MVDS units must be installed such that each unit operates independently and does not interfere with other MVDS units or other equipment in the vicinity.

You are responsible for site visits and analysis of each proposed pole location to assure that the detector placement will comply with the manufacturer's published installation instructions and the performance required in the special provisions. You must confirm detector placement with the manufacturer before performing work at the MVDS location. Whenever the manufacturer’s analysis requires a change in the proposed pole location, You must arrange a meeting with the manufacturer and the Engineer to select a new pole location.

You must not proceed with any MVDS installation until the pole location is authorized.

You is responsible for the compatibility of components and for making necessary calibration adjustments to deliver the performance required in the special provisions. You must provide equipment required to set up, calibrate, verify performance, and maintain the MVDS.

You must provide programming software needed to support the MVDS. The software must be installed in the appropriate equipment and used for the acceptance testing.

86-5.01F(1)(iv) Testing

Accuracy of the MVDS system must be verified by comparing the MVDS vehicle counts to recorded video image counts for the same period. The video camera must be located and oriented so that traffic is visible in all lanes. Video images must be time stamped and analysis periods recorded to a DVD or CD media for viewing on a PC. The video field of view must totally encompass the area in which vehicles are detected. You must provide a means for synchronizing the test starting and ending times or provide software that displays time stamped MVDS data along with the video images of the moving vehicles. You must provide the Engineer with the original recording medium and documentation that supports the accuracy analysis and make a copy of these materials for its own use.

The accuracy test must take place during a complex traffic period as specified by the Engineer. The following video recording and analysis options that depend on the available traffic conditions are acceptable; however the heaviest expected traffic conditions should be used, if possible:

1. The minimum recording period must be 30 minutes when the recording includes congested traffic (vehicles traveling at less than 20 mph for five or more minutes in any lane).
2. The minimum recording period must be 45 minutes when the traffic flow exceeds 1500 vehicles per hour in any lane during the test period.
3. The minimum recording period must be 60 minutes when the flow is less than 1500 vehicles per hour in every lane.

The analysis must be based on a minimum of 100 detected vehicles in every lane and cover the same time period for all lanes. The time period within the selected video will be selected by the Engineer. The total vehicle count for every lane must be used and include the first and last partial vehicles for each lane. Errors in the start and finish of the MVDS and manual counts are included in the performance criterion specified in the special provisions.

MVDS unit count must be compared to the vehicle counts under these traffic conditions. Vehicles licensed for use on State roads must be counted by the MVDS. The data accuracy must be determined by the formula 100{1-[(TC-MC)/TC]}, where TC=traffic count derived from the media recording, MC=MVDS-reported count over the same period of time, and the resulting fraction is expressed as an absolute value.

The Engineer will review the accuracy data findings and accept or reject the results within 15 days. Determination of vehicle anomalies or unusual occurrences will be decided by the Engineer. Data or counts that are not accepted by the Engineer must be considered errors and count against the MVDS unit's calibration. If the Engineer finds that the MVDS does not comply with the performance requirements, You must recalibrate and retest the unit and resubmit new test data within 10 days. Following 3 failed attempts, You must replace the MVDS detector with a new unit.

In addition to the accuracy analysis performed by You, You must provide equipment, software, documentation, support equipment, and any other materials, personnel, and devices that may be required for acceptance testing by the Engineer. You must notify the Engineer 15 days before the MVDS unit is ready for acceptance testing. Testing must be scheduled to be accomplished before the end of the normal work shift.

86-5.01F(1)(v) Technical Requirements

MVDSs must be FCC certified under 47 CFR 15 for low power, unlicensed, continuous radio transmitter operation. The MVDS must comply with FCC regulations for all specified operating conditions and over the expected life of the MVDS.

All electronic assemblies must comply with the specifications in chapters 1 and 5 of the TEES.

86-5.01F(2) MATERIALS

86-5.01F(2)(a) General

Temporary Microwave Vehicle Detection System consists, in general, of installing amicrowave vehicle detection system(MVDS) on wood pole (sensor unit and include all required mounting hardware, power supplies, surge suppression, cables, connectors, and wiring, conductors and incidentals, as shown in the plans, as specified in the Standard Specifications and these Special Provisions, and as directed by the Engineer.

The MVDS sensor must include, as a minimum, a directional microwave transmitter, antenna, microwave receiver, processor, memory, and communication interface.

The MVDS must have an Ethernet communication port that supports the National Transportation Communication for ITS Protocol (NTCIP). The MVDS communication protocol must be nonproprietary and openly specified and available for use in the public domain. The MVDS must be addressable and must download count, speed, and occupancy data when polled by the transportation management center computer. Speed must be configurable in U.S. customary or metric units. The MVDS must support unit setup from a serial console port on the MVDS unit. The console port protocol must support sensor unit setup from a local Windows 2000 or newer compatible laptop or from a remote location with a desktop computer and standard phone modem.

Connect MVDS sensor contact outputs to Model 170E/2070 controller to emulate inductive loops and comply with the following:

1. The MVDS sensors must be connected to a microwave sensor interface card (MSIF) installed in the input file of a Department-furnished Model 170E or Model 2070 controller cabinet.
2. Each detection zone must provide an optically isolated relay contact pair that follows the presence of vehicles in every traffic lane and sends signals to the controller with the accuracy stated in the special provisions.
3. The MSIF must have indications for power, communication, and the real-time operation of each detection contact output.

You must install an MVDS termination assembly (MTA) as shown. The MTA must be a single circuit board or a set of modular DIN-rail rack-mounted assemblies. The MTA must provide screw cable-lug terminations for all MVDS units with clearance for routing the cables and labels to identify the connected MVDS detectors. The MVDS unit must be connected in the order shown or as ordered. Each lane detection zone must have an LED that indicates vehicle presence with a minimum viewing angle of 50 degrees and visible from 5 feet in daylight. Power must be routed from the MTA to each MVDS unit. Each MVDS power connection must have fast-blow AGC type fuse or a resettable circuit breaker such that the loss of power to any single MVDS due to overcurrent does not limit the operation of the other connected MVDS. The fuses or breakers must be easily accessible and replaceable or resetable without requiring tools or removing cables, connectors, or other terminations. The MTA must have DB9 male connectors for every connected MVDS unit for TIA-232 communications and a local laptop computer for performing setup and diagnostics as well as remote communications. The connectors must be labeled and provide internal routing of data between the DB9 connectors and the MVDS terminal strips. You must supply the cable and connectors for connecting the communication modem.

MVDSs must be user programmable in the field via the MVDS unit console port with a Windows 2000 or newer compatible laptop computer. You must provide software, firmware, and equipment to set up, calibrate, and operate the unit. MVDS software must observe the vehicular traffic and automatically place detection lanes and set the sensor sensitivity.MVDS sensor unit must not weigh more than 11 pounds. The MVDS must operate over a temperature range from –30 to +70 degrees C, with up to 95 percent relative humidity. The MVDS sensor enclosure must be weatherproof with a NEMA 3R rating and the sensor mounted and directed perpendicular to the flow of traffic lanes at the locations shown.

The power supply or transformer must comply with or exceed the following minimum requirements:

	Power supply	Transformer
Power cord	Standard 120V(ac), 3 prong cord, at least 40 inches in length (may be added by Contractor)	Standard 120V(ac), 3 prong cord, at least 40 inches in length (may be added by Contractor)
Type	Switching mode type	Class 2
Rated power	Twice (2x) full system load	Twice (2x) full system load
Operating temperature	From –35 °C to +74 °C	From –35 °C to +74 °C
Operating humidity range	From 5 to 95 percent	From 5 to 95 percent
Input voltage	From 90 to 135 V(ac)	From 90 to 135 V(ac)
Input frequency	60 +/- 1 Hz	60 +/- 1 Hz
Inrush current	Cold start, 25 A max. at 115 V	N/A
Output voltage	As required by the MVDS	As required by the MVDS
Overload protection	From 105 to 150 percent in output pulsing mode	Power limited at >150 percent
Over voltage protection	From 115 percent to 135 percent of rated output voltage	N/A
Setup, rise, hold up	800ms, 50ms,15ms at 115 V(ac)	N/A
Withstand voltage	I/P-0/P:3kV, I/P-FG:1.5kV, for 60 sec.	I/P-0/P:3kV, I/P-FG:1.5kV, for 60 sec
Working temperature	Not to exceed 70 °C@30% load	Not to exceed 70 °C@30% load
Safety standards	UL 1012, and UL 60950	UL 1585
EMC standards	FCC Part 15	N/A

86-5.01F(3) CONSTRUCTION

86-5.01F(3)(a) General

MVDS sensors must be wired with a connectorized cable harness. Cables must run continuously without splices between the sensor and controller cabinet and terminate in labeled terminal blocks identified with the purpose served. The connector must be a standard mil type and rated plug. The cable must have the number of conductors specified by the MVDS manufacturer to support the number of detection zones shown plus spares for 2 future zones with an overall shield and copper drain wire. Conductors must be stranded copper equal to or exceeding the minimum strands and wire dimensions specified by the MVDS manufacturer for the wiring distance involved and covered with a minimum 12 mils polyvinyl chloride (PVC) insulation rated for 300 V at 105 degrees C. The outer jacket must be chrome PVC with minimum thickness of 53 mils and the outside diameter of the cable must not exceed 3/4 inch. A minimum of 6.5-foot slack of MVDS cable must be coiled at the bottom of the controller cabinet. Slack in other cabinets must be as shown or as ordered.

The MSIF must be inserted into the controller input file slots using the edge connector to obtain limited 24 V(dc) power and to provide contact closure outputs. No rewiring to the Model 170E or Model 2070 cabinet must be allowed. The MSIF must comply with the specifications in chapter 1 as well as sections 5.2.8, 5.2.8.1, 5.2.8.2, 5.4.1, 5.4.5, 5.4.6, 5.5.1, 5.5.5, and 5.5.6 of TEES.

MVDS sensor unit power supplies or transformers must be vertically mounted on a standard DIN-rail rack using standard mounting hardware. You must wire the MVDS power conductors to DIN-rail rack-mounted terminal blocks in the controller cabinet as ordered. The serial data communication output conductors must be terminated at MTA and continue for a minimum of 9.8 feet to a DB9F connector for setup and diagnostic access. The contact pair output conductors must be terminated at MTA and continue to controller cabinet terminal blocks for input to MSIF using detector designations shown on the plans. The ends of unused and spare conductors must be coiled and taped to prevent accidental contact to other circuits. Conductors inside the cabinet must be labeled for the functions as shown on the authorized detailed diagrams.

Field terminated circuits must include transient protection that complies with IEEE Standard 587-1980 Category C.

86-5.01F(4) Payment

Not Used

Replace section 86-6.02 with:

86-6.02 LED LUMINAIRES

86-6.02A General

86-6.02A(1) Summary

Section 86-6.02 includes specifications for installing LED luminaires.

86-6.02A(2) Definitions

CALiPER: Commercially Available LED Product Evaluation and Reporting. A U.S. DOE program that individually tests and provides unbiased information on the performance of commercially-available LED luminaires and lights.

correlated color temperature: Absolute temperature in kelvin of a blackbody whose chromaticity most nearly resembles that of the light source.

house side lumens: Lumens from a luminaire directed to light up areas between the fixture and the pole, such as sidewalks at intersection or areas off the shoulders on freeways.

International Electrotechnical Commission (IEC): Organization that prepares and publishes international standards for all electrical, electronic, and related technologies.

junction temperature: Temperature of the electronic junction of the LED device. The junction temperature is critical in determining photometric performance, estimating operational life, and preventing catastrophic failure of the LED.

L70: Extrapolated life in hours of the luminaire when the luminous output depreciates 30 percent from initial values.

LM-79: Test method from the Illumination Engineering Society of North America specifying test conditions, measurements, and report format for testing solid state lighting devices, including LED luminaires.

LM-80: Test method from the Illumination Engineering Society of North America specifying test conditions, measurements, and report format for testing and estimating the long-term performance of LEDs for general lighting purposes.

National Voluntary Laboratory Accreditation Program (NVLAP): U.S. DOE program that accredits independent testing laboratories.

power factor: Ratio of the real power component to the complex power component.

street side lumens: Lumens from a luminaire directed to light up areas between the fixture and the roadway, such as traveled ways and freeway lanes.

surge protection device (SPD): Subsystem or component that protects the unit against short-duration voltage and current surges.

total harmonic distortion: Ratio of the rms value of the sum of the squared individual harmonic amplitudes to the rms value of the fundamental frequency of a complex waveform.

86-6.02A(3) Submittals

Submit a sample luminaire to METS for testing after the manufacturer's testing is completed. Include the manufacturer's test data.

Product submittals must include:

1. LED luminaire checklist.
2. Product specification sheets, including:
   2.1. Maximum power in watts.
   2.2. Maximum designed junction temperature.
   2.3. Heat sink area in square inches.
   2.4. Designed junction to ambient thermal resistance calculation with thermal resistance components clearly defined.
   2.5. L70 in hours when extrapolated for the average nighttime operating temperature.
3. LM-79 and LM-80 compliant test reports from a CALiPER-qualified or NVLAP-approved testing laboratory for the specific model submitted.
4. Photometric file based on LM-79 test report.
5. Initial and depreciated isofootcandle diagrams showing the specified minimum illuminance for the particular application. The diagrams must be calibrated to feet and show a 40 by 40 foot grid. The diagrams must be calibrated to the mounting height specified for that particular application. The depreciated isofootcandle diagrams must be calculated at the minimum operational life.
6. Test report showing SPD performance as tested under ANSI/IEEE C62.41.2 and ANSI/IEEE C62.45.
7. Test report showing mechanical vibration test results as tested under California Test 611 or equal.
8. Data sheets from the LED manufacturer that include information on life expectancy based on junction temperature.
9. Data sheets from the power supply manufacturer that include life expectancy information.

Submit documentation of a production QA performed by the luminaire manufacturer that:

1. Ensures the minimum specified performance level
2. Includes a documented process for resolving problems

Submit the QA documentation as an informational submittal.

Submit the manufacturer's warranty documentation as an informational submittal before installing LED luminaires.

86-6.02A(4) Quality Control and Assurance

86-6.02A(4)(a) General

The Department may test random samples of the luminaires under section 86-2.14A. The Department tests luminaires under California Test 678 and may test any parameters specified in section 86-6.01.

Fit 1 sample luminaire with a thermistor or thermocouple temperature sensor. A temperature sensor must be mounted on the:

1. LED solder pad as close to the LED as possible
2. Power supply case
3. Light bar or modular system as close to the center of the module as possible

Other configurations must have at least 5 sensors per luminaire. The Engineer provides advice on sensor location. Thermocouples must be either Type K or C. Thermistors must be a negative-temperature-coefficient type with a nominal resistance of 20 kΩ. Use the appropriate thermocouple wire. The leads must be a minimum of 6 feet. Submit documentation with the test unit describing the type of sensor used.

Before performing any testing, energize the sample luminaires for a minimum of 24 hours at 100 percent on-time duty cycle and a temperature of +70 degrees F.

Depreciate the luminaire lighting's performance for the minimum operating life by using the LED manufacturer's data or the data from the LM-80 test report, whichever results in a higher lumen depreciation.

Failure of the luminaire that renders the unit noncompliant with section 86-6.02 specifications is cause for rejection.

86-6.02A(4)(b) Warranty

Provide a 7-year manufacturer's warranty against any defects or failures. The warranty period begins on the date of Contract acceptance. Furnish a replacement luminaire within 10 days after receipt of the failed luminaire. The Department does not pay for the replacement. Deliver replacement luminaires to the Department's Maintenance Electrical Shop at:

Burbank Electrical 602 S. Flower St, Burbank, CA 91502 (for the area north of Mountain Ave)

San Gabriel Electrical 9153 Lower Azusa Rd, Rosemead, CA 91770 (for the area south of Mountain Ave)

86-6.02B Materials

86-6.02B(1) General

The luminaire must include an assembly that uses LEDs as the light source. The assembly must include a housing, an LED array, and an electronic driver. The luminaire must:

1. Be UL listed under UL 1598 for luminaires in wet locations or an equivalent standard from a recognized testing laboratory
2. Have a minimum operational life of 63,000 hours
3. Operate at an average operating time of 11.5 hours per night
4. Be designed to operate at an average nighttime operating temperature of 70 degrees F
5. Have an operating temperature range from -40 to +130 degrees F
6. Be defined by the following applications:

Application	Replaces
Roadway 1	200 W high-pressure sodium luminaire mounted at 34 ft
Roadway 2	310 W high-pressure sodium luminaire mounted at 40 ft
Roadway 3	310 W high-pressure sodium luminaire mounted at 40 ft with back side control
Roadway 4	400 W high-pressure sodium luminaire mounted at 40 ft

The individual LEDs must be connected such that a catastrophic loss or a failure of 1 LED does not result in the loss of more than 20 percent of the luminous output of the luminaire.

86-6.02B(2) Luminaire Identification

Each luminaire must have the following identification permanently marked inside the unit and outside of its packaging box:

1. Manufacturer's name
2. Trademark
3. Model number
4. Serial number
5. Month and year of manufacture
6. Lot number
7. Contract number
8. Rated voltage
9. Rated wattage
10. Rated power in VA

86-6.02B(3) Electrical Requirements

The luminaire must operate from a 60 ± 3 Hz AC power source. The fluctuations of line voltage must have no visible effect on the luminous output. The operating voltage may range from 120 to 480 V(ac). The luminaire must operate over the entire voltage range or the voltage range must be selected from either of the following options:

1. Luminaire must operate over a voltage range of 95 to 277 V(ac). The operating voltages for this option are 120 V(ac) and 240 V(ac).
2. Luminaire must operate over a voltage range of 347 to 480 V(ac). The operating voltage for this option is 480 V(ac).

The power factor of the luminaire must be 0.90 or greater. The total harmonic distortion, current, and voltage induced into an AC power line by a luminaire must not exceed 20 percent. The maximum power consumption allowed for the luminaire must be as shown in the following table:

Application	Maximum consumption (watts)
Roadway 1	165
Roadway 2	235
Roadway 3	235
Roadway 4	300

86-6.02B(4) Surge Suppression and Electromagnetic Interference

The luminaire's on-board circuitry must include an SPD to withstand high repetition noise transients caused by utility line switching, nearby lightning strikes, and other interferences. The SPD must protect the luminaire from damage and failure due to transient voltages and currents as defined in Tables 1 and 4 of ANSI/IEEE C64.41.2 for location category C-High. The SPD must comply with UL 1449. The SPD must be tested under ANSI/IEEE C62.45 based on ANSI/IEEE C62.41.2 definitions for standard and optional waveforms for location category C-High.

The luminaires and associated on-board circuitry must comply with the Class A emission limits under 47 CFR 15, subpart B, for the emission of electronic noise.

86-6.02B(5) Compatibility

The luminaire must be operationally compatible with currently-used lighting control systems and photoelectric controls.

86-6.02B(6) Photometric Requirements

The luminaire must maintain a minimum illuminance level throughout the minimum operating life. The L70 of the luminaire must be the minimum operating life or greater. The measurements must be calibrated to standard photopic calibrations. The minimum maintained illuminance values measured at a point must be as shown in the following table:

Application	Mounting height (ft)	Minimum maintained illuminance (fc)	Light pattern figure (isofootcandle curve)
Roadway 1	34	0.15	Pattern defined by an ellipse with the equation: $$\frac{x^2}{(82)^2} + \frac{(y-20)^2}{(52)^2} = 1$$ where: x = direction longitudinal to the roadway y = direction transverse to the roadway and the luminaire is offset from the center of the pattern by 20 feet to the house side of the pattern.
Roadway 2	40	0.2	Pattern defined by an ellipse with the equation: $$\frac{x^2}{(82)^2} + \frac{(y-20)^2}{(52)^2} = 1$$ where: x = direction longitudinal to the roadway y = direction transverse to the roadway and the luminaire is offset from the center of the pattern by 20 feet to the house side of the pattern.
Roadway 3	40	0.2	Pattern defined by an ellipse with the equation: $$\frac{x^2}{(82)^2} + \frac{(y-20)^2}{(52)^2} = 1$$ for $y \geq 0$ (street side) where: x = direction longitudinal to the roadway y = direction transverse to the roadway and the luminaire is offset from the center of the pattern by 20 feet to the house side of the pattern.
Roadway 4	40	0.2	Pattern defined by an ellipse with the equation: $$\frac{x^2}{(92)^2} + \frac{(y-23)^2}{(55)^2} = 1$$ where: x = direction longitudinal to the roadway y = direction transverse to the roadway and the luminaire is offset from the center of the pattern by 23 feet to the house side of the pattern.

The luminaire must have a correlated color temperature range from 3,500 to 6,500 K. The color rendering index must be 65 or greater.

The luminaire must not allow more than:

1. 10 percent of the rated lumens to project above 80 degrees from vertical
2. 2.5 percent of the rated lumens to project above 90 degrees from vertical

86-6.02B(7) Thermal Management

The passive thermal management of the heat generated by the LEDs must have enough capacity to ensure proper operation of the luminaire over the minimum operation life. The LED maximum junction temperature for the minimum operation life must not exceed 221 degrees F.

The junction-to-ambient thermal resistance must be 95 degrees F per watt or less. The use of fans or other mechanical devices is not allowed. The heat sink material must be aluminum or other material of equal or lower thermal resistance.

The luminaire must contain circuitry that automatically reduces the power to the LEDs so the maximum junction temperature is not exceeded when the ambient outside temperature is 100 degrees F or greater.

86-6.02B(8) Physical and Mechanical Requirements

The luminaire must:

1. Be a single, self-contained device not requiring job-site assembly for installation
2. Have an integral power supply
3. Weigh no more than 35 lb
4. Have a maximum-effective projected area of 1.4 sq ft when viewed from either side or end
5. Have a housing color that matches color number from 26152 to 26440, from 36231 to 36375, or 36440 of FED-STD-595.

The housing must be fabricated from materials designed to withstand a 3,000-hour salt spray test under ASTM B 117. All aluminum used in housings and brackets must be made of a marine-grade alloy with less than 0.2 percent copper. All exposed aluminum must be anodized.

Each refractor or lens must be made from UV-inhibited high-impact plastic such as acrylic or polycarbonate or heat- and impact-resistant glass and be resistant to scratching. Polymeric materials except lenses of enclosures containing either the power supply or electronic components of the luminaire must be made of UL94VO flame retardant materials. The housing's paint must comply with section 86-2.16. A chromate conversion undercoating must be used underneath a thermoplastic polyester powder coat.

Provide each housing with a slip fitter capable of mounting on a 2-inch pipe tenon. This slip fitter must fit on mast arms with outside diameters from 1-5/8 to 2-3/8 inches. The slip fitter must be capable of being adjusted a minimum of ±5 degrees from the axis of the tenon in a minimum of 5 steps: +5, +2.5, 0, -2.5, -5. The clamping brackets of the slip fitter must not bottom out on the housing bosses when adjusted within the designed angular range. No part of the slip fitter's mounting brackets must develop a permanent set in excess of 1/32 inch when the bracket's two or four 3/8-inch-diameter cap screws are tightened to 10 ft-lb. Two sets of cap screws may be furnished to allow the slip fitter to be mounted on the pipe tenon in the acceptable range without the cap screws bottoming out in the threaded holes. The cap screws and the clamping brackets must be made of corrosion-resistant materials or treated to prevent galvanic reactions and be compatible with the luminaire housing and the mast arm.

The LED luminaire must be assembled and manufactured such that its internal components are adequately supported to withstand mechanical shock and vibration from high winds and other sources. When tested under California Test 611, the luminaire to be mounted horizontally on the mast arm must be capable of withstanding the following cyclic loading for a minimum of 2 million cycles without failure of any luminaire part:

Cyclic Loading

Plane	Power supply	Minimum peak acceleration level
Vertical	Installed	3.0 g peak-to-peak sinusoidal loading (same as 1.5 g peak)
Horizontal[a]	Installed	1.5 g peak-to-peak sinusoidal loading (same as 0.75 g peak)

[a]Perpendicular to the direction of the mast arm

The housing must be designed to prevent the buildup of water on top of the housing. Exposed heat sink fins must be oriented to allow water to freely run off of the luminaire and carry dust and other accumulated debris away from the unit. The optical assembly of the luminaire must be protected against dust and moisture intrusion to at least an ANSI/IEC rating of IP66. The power supply enclosure must be protected to at least an ANSI/IEC rating of IP43.

Furnish each mounted luminaire with an ANSI C136.10-compliant, locking-type photocontrol receptacle and a raintight shorting cap. The receptacle must comply with section 86-6.11A.

Furnish each mounted luminaire with an ANSI C136.41-compliant, locking-type photocontrol receptacle with dimming connections and a raintight shorting cap. The receptacle must comply with section 86-6.11A.

When the components are mounted on a down-opening door, the door must be hinged and secured to the luminaire housing separately from the refractor or flat lens frame. The door must be secured to the housing such that accidental opening is prevented. A safety cable must mechanically connect the door to the housing.

Field wires connected to the luminaire must terminate on a barrier-type terminal block secured to the housing. The terminal screws must be captive and equipped with wire grips for conductors up to no. 6. Each terminal position must be clearly identified.

The power supply must be rated for outdoor operation and have at least an ANSI/IEC rating of IP65.

The power supply must be rated for a minimum operational life equal to the minimum operational life of the luminaire or greater.

The power supply case temperature must have a self rise of 77 degrees F or less above ambient temperature in free air with no additional heat sinks.

The power supply must have 2 leads to accept standard 0-10 V(dc). The dimming control must be compatible with IEC 60929. If the control leads are open or the analog control signal is lost, the circuit must default to 100-percent power.

Conductors and terminals must be identified.

86-6.02C Construction

Not Used

86-6.02D Payment

Not Used

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For each stockpile of steel slag aggregate, moisture tests must be made at least once each week. The time covered by tests that show a moisture content of 6 percent or less is not included in the aging time.

Notify METS and the Engineer upon completion of each stockpile and the start of controlled aging and upon completion of controlled aging. Do not add aggregate to a stockpile unless a new aging period is started.

Steel slag used for imported borrow must be weathered for at least 3 months.

Each delivery of aggregate containing steel slag for AS or Class 2 AB must include a delivery tag for each load. The tag must identify the lot by the stockpile number, slag aging location, and stockpile completion and controlled aging start date.

You may blend air-cooled iron blast furnace slag or natural aggregate in proper combinations with steel slag aggregate to produce the specified gradings.

California Test 202 is modified by California Test 105 whenever the difference in sp gr between the coarse and fine portions of the aggregate or between the blends of different aggregates is 0.2 or more.

For slag used as aggregate in HMA, the Kc factor requirements in California Test 303 do not apply.

If steel slag aggregates are used to produce HMA, no other aggregates may be used in the mixture except that up to 50 percent of the material passing the no. 4 sieve may consist of iron blast furnace slag aggregates, natural aggregates, or a combination of these. If iron blast furnace aggregates, natural aggregates, or a combination of these are used in the mixture, each aggregate type must be fed to the drier at a uniform rate. Maintain the feed rate of each aggregate type within 10 percent of the amount set. Provide adequate means for controlling and checking the feeder accuracy.

Store steel slag aggregate separately from iron blast furnace slag aggregate. Store each slag aggregate type separately from natural aggregate.

For HMA produced from steel slag aggregates, iron blast furnace slag aggregates, natural aggregates, or any combination of these, the same aggregate must be used throughout any one layer. Once an aggregate type is selected, do not change it without authorization.

Aggregate containing slag must comply with the applicable quality requirements for the bid items in which the aggregate is used.

87-2.03 CONSTRUCTION

Do not place aggregate produced from slag within 1 foot of a non–cathodically protected pipe or structure unless the aggregate is incorporated in concrete pavement, in HMA, or in treated base.

Do not place slag aggregate used for embankments within 18 inches of finished slope lines measured normal to the plane of the slope.

Whenever slag aggregate is used for imported borrow, place a layer of topsoil at least 24 inches thick after compaction over the slag aggregate in highway planting areas.

87-2.04 PAYMENT

The Department reduces the payment quantity of HMA if:

1. Steel slag aggregates are used to produce HMA
2. The sp gr of a compacted stabilometer test specimen is in excess of 2.40

The Department prepares the stabilometer test specimen under California Test 304 and determines the sp gr of the specimen under Method C of California Test 308.

The Department determines the HMA payment quantity by multiplying the quantity of HMA placed in the work by 2.40 and dividing the result by the sp gr of the compacted stabilometer test specimen. The Department applies this quantity reduction as often as necessary to ensure accurate results.

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88 GEOSYNTHETICS

Add to section 88-1.02B:

Filter fabric for retaining walls must be Class A.

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90 CONCRETE

Add to section 90-2.02B:

You may use rice hull ash as an SCM. Rice hull ash must comply with AASHTO M 321 and the chemical and physical requirements shown in the following tables:

Chemical property	Requirement (percent)
Silicon dioxide (SiO_2)[a]	90 min
Loss on ignition	5.0 max
Total alkalies as Na_2O equivalent	3.0 max

Physical property	Requirement
Particle size distribution Less than 45 microns Less than 10 microns	 95 percent 50 percent
Strength activity index with portland cement[b] 7 days 28 days	 95 percent (min percent of control) 110 percent (min percent of control)
Expansion at 16 days when testing project materials under ASTM C 1567[c]	0.10 percent max
Surface area when testing by nitrogen adsorption under ASTM D 5604	40.0 m^2/g min

[a]SiO_2 in crystalline form must not exceed 1.0 percent.
[b]When tested under AASHTO M 307 for strength activity testing of silica fume.
[c]In the test mix, Type II or V portland cement must be replaced with at least 12 percent rice hull ash by weight.

For the purpose of calculating the equations for the cementitious material specifications, consider rice hull ash to be represented by the variable *UF*.

Replace section 90-8 with:

90-8 ROLLER COMPACTED CONCRETE

90-8.01 GENERAL

90-8.01A Summary

Section 90-8 includes specifications for furnishing roller compacted concrete (RCC.)

90-8.01B Definitions

Not Used

90-8.01C Submittals

Not Used

90-8.01D Quality Control and Assurance

90-8.01D(1) General

Fabricate compressive strength test specimens under ASTM C 1435.

Paragraph 2 of section 90-1.01D(2) does not apply to RCC.

Prequalify RCC under section 90-1.01D(5)(b).

90-8.01D(2) Shrinkage

Test RCC under ASTM C 341 on samples that have been cut from compacted RCC modified as follows:

1. Remove specimens from the in-place compacted RCC between 24 hours and 48 hours age.
2. Transport and store specimens in a water bath.
3. Trim specimens to a 4 inch by 4 inch cross section and the required length for a 10 inch gage length.
4. Use length measurement at 7 days age as the base length.
5. Store in air.
6 Take subsequent readings at 7, 14, 21, and 28 days drying.

Test two specimens from the end of one RCC placement for each week that RCC is placed on more than 3 days.

90-8.01D(3) Compressive Strength

The fifth paragraph of section 90-1.01D(5)(a) does nota apply to RCC. If the compressive strength is 5000 psi or more, 42 days are allowed to attain the specified strength.

90-8.02 MATERIALS

90-8.02A General

If the minimum cementitious material content specified is less than 425 lb/cu yd, use the following equation in place of Equation 2 in 90-1.02(B)3:

$$(425 - MSCM - PC) \geq 0$$

If the total alkali content of the cement used is less than 2 pounds per cubic yard of concrete, paragraph 3 of section 90-1.02(B)3 does not apply to RCC.

90-8.02B Aggregate Gradings

90-8.02B(1) General

Section 90-1.02C(4) does not apply to RCC.

Furnish aggregate divided into two or more sizes. The grading of each of the sizes is at your discretion. Control the gradation of each size with a minimum of 4 sieves. Maintain the gradation on each sieve within 10 percentage points of your target for each sieve.

90-8.02B(2) Combined Aggregate Grading

Combined aggregate must be graded within the limits shown in the following table:

Sieve size	Percentage passing				
	1/2 inch max	3/4 inch max	1 inch max	1-1/2 inch max	2 inch max
2-1/2 inch	--	--	--	--	100
2 inch	--	--	--	100	90–100
1-1/2 inch	--	--	100	87–100	78–97
1 inch	--	100	82–100	71–91	--
3/4 inch	100	87–100	72–91	--	56–76
1/2 inch	81–100	71–91	--	50–71	--
3/8 inch	71–91	--	50–71	--	39–59
No. 4	49–70	42–63	--	30–49	27–46
No. 8	33–54	--	23–42	--	--
No. 16	--	18–36	--	13–28	--
No. 30	12–28	--	8–22	--	6–18
No. 50	--	5–19	--	3–15	--
No. 100	3–16	1–13	1–12	1–11	1–10
No. 200	0–10	0–9	0–8	0–7	0–7

90-8.03 CONSTRUCTION

90-8.03A General

Not Used

90-8.03B Mixing and Transporting RCC

90-8.03B(1) General

Mix and transport concrete under 90-1.02G. You may use batch mixing or horizontal shaft mixing. Section 90-1.02G(5) and 90-1.02G(6) do not apply.

Protect concrete hauled in open-top vehicles from rain or evaporation with retractable covers.

Discharge RCC material within 45 minutes after the addition of the cement to the aggregates. Deposit the RCC material directly into the hopper of the paver or into a secondary material distribution system that deposits the material into the paver hopper.

90-8.03B(2) Horizontal Shaft Mixing

A horizontal shaft mixing plant must be a central plant type with a twin-shaft mixer, capable of either batch or continuous mixing, equipped with synchronized metering devices and feeders to maintain the correct proportions of aggregate, cement, SCM, and water. Plants must utilize:

1. Controlled aggregate feed rate. Use aggregate bins with a feed rate controlled by a variable speed belt, or an operable gate calibrated to accurately deliver any specified quantity of material. The feed rate from each aggregate bin must be readily adjustable to change aggregate proportions. Feed rate controls must maintain the established proportions of aggregate from each stockpile bin when the combined aggregate delivery is increased or decreased.
2. Plant scales. Plant scales must conform to Section 90-1.02F(3), “Proportioning Devices.”
3. Cement and SCM storage. Supply separate and independent storage silos for cement and SCM. Each silo must have a sign at each fill inlet clearly identifying the material that is in the silo that is easily read even when completely coated with dust. Make the sign from a durable material, with minimum two-inch high by ¼-inch wide letters that are raised, indented, or cut.
4. Preblended portland cement and SCM. If you preblend portland cement and SCM at the batch plant, demonstrate, with a testing plan, the ability to successfully produce a uniform blended material meeting the mix design requirements. Perform testing on at least a daily basis to ensure both uniformity and proper quantities.
5. Cement and SCM feed unit. Provide a satisfactory means of dispensing portland cement and SCM, volumetrically or by weight, to ensure a uniform and accurate quantity of cementitious material enters the mixer.
6. Water control unit. Use a water control unit capable of measuring the required amount of water for the approved mix by weight or volume. Ensure that the unit is equipped with an accurate metering device. Vary the amount of water to be used only with the Engineer's approval.

7. Gob hopper. For continuous operating mixers, provide a gob hopper attached to the end of the final discharge belt to temporarily hold the roller compacted concrete material in order to allow the plant to operate continuously.

90-8.04 PAYMENT

Not Used

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Abbreviations

Abbreviation	Meaning
AAMA	American Architectural Manufacturers' Association
ADAAG	ADA Accessibility Guidelines for Buildings and Facilities
AGA	American Gas Association
AITC	American Institute of Timber Construction
ALSC	American Lumber Standard Committee
AMCA	Air Movement and Control Association International
APA	Engineered Wood Association
AHRI	Air-Conditioning, Heating, and Refrigeration Institute
ASHRAE	American Society of Heating, Refrigerating and Air-Conditioning Engineers
BIA	Brick Industry Association
CEC	California Electrical Code
CMC	California Mechanical Code
CPC	California Plumbing Code
CRRC	Cool Roof Rating Council
CSA	Canadian Standards Association
ESO	Electrical Safety Orders
FM	FM Global
FS	Federal Specification
GA	Gypsum Association
GANA	Glass Association of North America
IGMA	Insulating Glass Manufacturers Alliance
ISO	International Organization for Standardization
NAAMM	National Association of Architectural Metal Manufacturers
PEI	Porcelain Enamel Institute
RIS	Redwood Inspection Service
SMACNA	Sheet Metal and Air Conditioning Contractors' National Association
TCNA	Tile Council of North America
TPI	Truss Plate Institute
WCLB	Grade stamp issued by West Coast Lumber Inspection Bureau
WI	Woodwork Institute
WWPA	Western Wood Products Association

1.3 DEFINITIONS (Not Used)

1.4 COORDINATION WITH THE DEPARTMENT

A. The Department will be working at or near the job site. Coordinate activities with the Department to avoid delays.
B. Comply with security policies of the Department facility.
C. Submit a request for authorization before interrupting any service for the purpose of making or breaking a connection. Include in the request the proposed time necessary to complete the work. Allow 5 days for the review of each request.
D. You may obtain electrical power and water from existing Department electrical power and water outlets on the job site for Contract operations at no cost to you. The Engineer determines which outlets you may use. You must not modify outlets.
E. Do not use Department telephones.

1.5 SUBMITTALS

A. In addition to specified submittals, submit any other submittal the Engineer requests.
B. Within 50 days of Contract approval, submit building construction work action submittals, including:
   1. Shop drawings
   2. Material lists
   3. Product and descriptive data
   4. Samples

C. Submit at least 5 sets or samples for each item. Except for samples, the Department returns 2 copies that show an authorized date or a request for correction and resubmittal.
D. Submit the schedule of values within 20 days of Contract approval. Submit at least 2 sets.
E. Each shop drawing sheet must be at least 11 by 17 inches and at most 24 by 36 inches.
F. Each material list must include the name of manufacturer, catalog number, size, capacity, finish, all pertinent ratings, and identification symbols described.
G. Submit building construction work submittals to OSD, Documents Unit. Notify the Engineer of the submittal. Include the date and contents of the submittal in the notification.
H. Allow 20 days for the review.
I. Dispose of samples not incorporated in the work.
J. Submit 3 copies of the following items as informational submittals:
   1. Part lists and service instructions packaged with or accompanying the equipment
   2. Operating and maintenance instructions
   3. Manufacturer's warranties
   4. Qualification data

1.6 QUALITY CONTROL AND ASSURANCE

Not Used

1.7 SCHEDULE OF VALUES

A. Section 9-1.16B does not apply.
B. Divide the schedule of values into sections representing the cost of each separate building or structure. Do not include work that is not part of the building or structure, such as excavation, grading, curbs, gutters, sidewalks, paving, sewer and storm drainage, or utility distribution lines, in the building or structure cost. Include this work in a section titled "General Work."
C. List indirect costs and bond premiums as separate line items of work.
D. Identify the sections representing each building or structure as to the building or structure they represent and break them down to show the corresponding value of each craft, trade, or other significant portion of the work. Provide a subtotal for each section.
E. Obtain authorization of a schedule of values before you perform work shown on the schedule. The Department does not process a progress payment for building work without an authorized schedule of values.
F. The sum of the items listed in the schedule of values must equal the contract lump sum price for building work. Do not list overhead and profit. Include bond premiums and other such items in the mobilization bid item for the entire project.

1.8 UTILITY CONNECTIONS

A. Make arrangements and obtain PLACs required for the extension of and connection to each utility service. For extensions not furnished by the utility, furnish the extensions and install any intermediate equipment required by the servce utilities.
B. The costs incurred by you for the following items is change order work:
   1. Utility permits, licenses, connection charges, and excess length charges
   2. Extensions of utilities beyond the limits shown
   3. Furnishing and installing any intermediate equipment required by the servce utilities

1.9 SANITARY FACILITIES

A. During toilet room renovation or other periods when Department sanitary facilities are not operational, furnish the following for Department forces:
   1. Wash facilities
   2. Drinking water fixtures
   3. At least 2 temporary toilet units
B. Furnish separate temporary toilet units for your personnel.
C. Temporary toilet units must be (1) single-occupant units of the chemical type, (2) properly vented, and (3) fully enclosed with a glass-fiber-reinforced polyester shell or similar nonabsorbent material.

D. Perform periodic flushing, waste removal, and cleaning of temporary toilet units. Maintain units in a clean and sanitary condition, including a supply of toilet paper, toilet seat covers, and paper towels.

1.10 AS-BUILT DRAWINGS

A. Prepare and maintain 1 set of as-built drawings using an unaltered set of original project plans, to show all as-constructed information, including:
   1. Any plan clarifications or *Change Order* changes
   2. Locations of any underground utilities
   3. Location, size, type, and manufacturer of major products or components used in the work
B. Neatly prepare as-built drawings as follows:
   1. Place markings on the project record drawings using red ink or red pencil.
   2. Do not eradicate or write over original figures.
   3. Line out superseded material.
   4. Submit additional drawings if the required information cannot be clearly shown on the original set of project plans. The additional drawings must be at least 11 by 17 inches and at most 24 by 36 inches.
   5. Sign and date each sheet verifying that all as-built information shown on the drawings is correct.
C. Review the as-built drawings monthly with the Engineer during the progress of the work to assure that all changes and other required information are being recorded.
D. Before completion of the work, request a review of the as-built drawings to determine the completeness and adequacy of them. If the as-built drawings are unacceptable, you must inspect, measure, and survey the work as necessary to record the required additional information.

PART 2 - PRODUCTS (Not Used)

PART 3 - EXECUTION

1.1 INSPECTION

A. Any work that will be covered or not visible in the completed work must be inspected and accepted by the Engineer before progress of work conceals portions to be inspected. Notify the Engineer at least 3 business days before needing inspection.

99-01050 FIELD ENGINEERING

99-01050A General

99-01050A(1) Summary

This work includes administrative and procedural requirements for field engineering services to be performed by the Contractor.

99-01050A(2) Definitions

Not Used

99-01050A(3) Submittals

Not Used

99-01050A(4) Quality Control and Assurance

Lines and Grades:

Such stakes or marks will be set by the Engineer as he determines to be necessary to establish the lines and grades required for the completion of the work shown and as described. In general, these will consist of the primary vertical and horizontal control points.

Stakes and marks set by the Engineer must be carefully preserved. In case such stakes and marks are destroyed or damaged they will be replaced at the Engineer's earliest convenience. You will be charged for the cost of necessary replacement or restoration of such stakes and marks which in the judgment of the Engineer were carelessly or willfully destroyed or damaged by your operations. This charge will be deducted from any moneys due or to become due to you.

All other stakes or marks required to establish the lines and grades required for the completion of the work must be the responsibility of you.

Existing Utilities and Equipment:

The existence and location of underground and other utilities and construction indicated as existing are not guaranteed. Before beginning sitework, you must investigate and verify the existence and location of underground utilities and other construction.

Prior to construction, you must verify the location and invert elevation at points of connection of sanitary and septic sewers, storm sewer, and water or fire service piping.

99-01050B Materials

Not Used

99-01050C Construction

Surveys for Layout and Performance:

You must perform all surveys for layout and performance, reduce field notes, and make all necessary calculations and drawings necessary to carry out the work.

You must locate and layout site improvements, and other work requiring field engineering services, including pavements, stakes for grading, fill and topsoil placement, utility slopes and invert elevations by instrumentation and similar appropriate means.

Batter boards must be located and laid out for structures, building foundations, column grids and locations, floor levels and, control lines and levels required for mechanical and electrical work.

Survey Accuracy and Tolerances:

The tolerances generally applicable in setting survey stakes for foundations, slabs, and underground work must not exceed the following:

Survey Stakes or Markers	Tolerance
Rough grading or excavation	0.10-foot
Trimming or preparation of subgrade for roadways	0.05-foot
Roadway surfacing, steel or concrete pipe	0.02-foot
Structures or building construction	0.01-foot

Such tolerance must not supersede stricter tolerances required by the plans or special provisions, and must not otherwise relieve you of responsibility for measurements in compliance therein.

99-01050D Payment

Not Used

99-02110 CLEARING AND GRUBBING

99-02110A General

99-02110A(1) Summary

Scope: This work consists of removing all objectionable material from the building site.

Clearing and grubbing must be performed in advance of any other grading or construction operations.

The area to be cleared and grubbed must be within the building work construction area.

99-02110A(2) Definitions

Not Used

99-02110A(3) Submittals

Not Used

99-02110A(4) Quality Control and Assurance

Not Used

99-02110A(5) Site Conditions

Traffic: Clearing and grubbing must be conducted to ensure minimum interference with roads, street, walks, or other occupied areas.

Protection of Existing Landscaping and Trees: Existing landscaping and trees which are to remain in place must be protected from injury or damage. Existing trees must be protected with a temporary fence around the drip line.

99-02110B Materials

Not Used

99-02110C Construction

99-02110C(1) Site Clearing

Remove trees, shrubs, grass, and other vegetation, concrete and masonry, improvements, or obstructions interfering with the new construction.

Trees to be removed must be grubbed to a depth of not less than 2 feet below finished grade.

99-02110C(2) Removal of Waste Material

Hauling: When hauling is done over highways or city streets, and when directed by the Engineer, the loads must be trimmed and all material removed from shelf areas of the vehicles.

Disposal: Trees, shrubs, grass, weeds and other vegetation, debrisand any obstructions above or below the ground surface that interfere with the building work, must be removed and disposed of.

99-02110D Payment

Not Used

99-02210 ROUGH GRADING

99-02210A General

99-02210A(1) Summary

Scope: This work consists of rough grading the job site.

Rough grading consists of excavation or removal of above grade material regardless of character and subsurface condition; filling of all holes, swales, embankments, and low points to the elevation shown or described; and the preparation of basement material for the placing of other material thereon and the establishment of the grading plane.

99-02210A(2) Definitions

Not Used

99-02210A(3) Submittals

Not Used

99-02210A(4) Quality Control and Assurance

Not Used

99-02210B Materials

Fill Material:

> Material from the excavation that is suitable for the required compaction may be used for filling holes, swales and low points. Fill material must be free of organic material. Rocks and lumps must be well distributed with sufficient earth or other fine matrix material to produce a dense, compacted fill that is suitable for the construction and load support intended.

The Contractor must furnish suitable borrow material to offset any material deficiencies developed from grading work.

99-02210C Construction

99-02210C(1) Excavation

Care must be exercised to avoid disturbing material below and beyond the limits of excavation. When excavation is carried beyond the limits shown or described such excavation must be replaced in kind and compacted at your expense.

Limits of the excavation must allow for adequate working space for installing materials and as required for safety of personnel. Such working space excavation must be replaced in kind and compacted at your expense.

Excess and waste materials from the excavation becomes your property and must be disposed of.

99-02210C(2) Fill

Subgrade Preparation: Preparation of subgrade material for placing other material thereon must include fine grading, compaction, reworking as necessary, and preparation of cut, or fill upon which base materials, surfacing, or slabs are to be placed. The upper 8 inches of the subgrade must have the same compaction as the fill to be placed over it.

Placing: When footings are to be constructed in fill, the fill must be constructed to the grading plane required for the building construction prior to excavating for the footings. Fill must be placed and compacted in layers. The loose thickness of each layer before compaction must not exceed 6 inches.

Water must be added to the fill material as needed for compaction.

99-02210C(3) Compaction

Relative compaction must be determined under California Test 216 or 231.

Relative Compaction (95 percent):

In fill relative compaction of not less than 95 percent must be obtained for a minimum depth of 2.5 feet below finished grade for the width of the paved areas plus 3 feet on each side thereof.

The prism of fill directly underneath the building foundation and sloping downward at 1:1 must be compacted to 95 percent.

Relative Compaction (90 percent): Relative compaction of not less than 90 percent must be obtained in all fill except as specified above.

99-02210C(4) Field Quality Control

Testing and Inspection: The State will conduct compaction tests during the earthwork operations.

99-02210D Payment

Not Used

99-02220 EARTHWORK FOR BUILDING WORK

99-02220A General

99-02220A(1) Summary

Scope: This work consists of performing earthwork for building work.

Earthwork for building work consists of structure excavation and structure backfill. Structure excavation must include excavation for footings, foundations, walls, slabs, tanks, drywells, manholes, oil/water separators, clarifiers, and trenches. Structure backfill must include backfilling under slabs; backfilling under and around footings; backfilling for walls, backfilling for pipes and conduits; backfilling holes resulting from removal of existing facilities. In addition to structure excavation and structure backfill, earthwork for building work must include any other earthwork, not mentioned, but necessary to complete the building work.

99-02220A(2) Definitions

Not Used

99-02220A(3) Submittals

Samples: Submit samples of sand, pea gravel, or crushed stone, weighing not less than 25 pounds.

99-02220A(4) Quality Control and Assurance

Not Used

99-02220A(5) Site Conditions

Existing Underground Piping and Conduit: The location of existing underground piping and conduit is based on the best records available. Before beginning work, you must accurately locate the piping and conduit involved in the work. If the location of the existing piping or conduit deviates from the location shown by more than 5 feet, or, if no elevations are indicated and the piping or conduit is more than 3 feet below grade, the cost of the additional excavation, backfill, piping or conduit, and removal and replacement of concrete, if any, will be change order work.

Existing Surfaced or Planted Areas:

Existing surfaced or planted areas that are removed, broken, or damaged by the Contractor's operations must be restored to their original condition except as otherwise shown or described.

Restoration materials must be equal to or better than the original materials. Surfacing must be replaced to match the material thickness, grades, and finish of the adjacent surrounding surfaces.

99-02220B Materials

Structure Backfill: Structure and trench backfill must be free of organic and other deleterious material and must be suitable for the required compaction. Gravel without sand matrix must not be used except as free draining granular material beneath slabs and footings.

Sand: Sand must be clean, washed sand, free from clay or organic material graded such that 100 percent passes the 1/4-inch sieve, 90 percent to 100 percent passes the No. 4 sieve and not more than 5 percent passes the No. 200 sieve size.

Pea Gravel (Naturally Rounded):

Pea gravel (naturally rounded) must be clean, washed, dry density of not less than 95 pounds per cubic foot, free from clay or organic material and must comply with the following grading as determined by California Test 202:

Sieve or Screen Size	Percentage Passing
3/4"	100
1/2"	90-100
3/8"	40-70
No. 4	0-15
No. 8	0-3

Pea gravel must comply with the following requirements:

Test	California Test No.	Test Requirements
Durability Index	229	35 Min.

Crushed Stone:

Crushed stone must be clean, washed, dry density of not less than 95 pounds per cubic foot, crushed stone or crushed gravel with an angular particle size not less than 1/8 inch or more than 1/2 inch.

Sieve or Screen Size	Percentage Passing
1/2"	100
3/8"	85-100
No. 4	10-30
No. 8	0-3

Crushed stone must comply with the following requirements:

Test	California Test No.	Test Requirements
Durability Index	229	35 Min.

99-02220C Construction

99-02220C(1) Preparation and Restoration

Sawcutting: Prior to excavation or trenching, existing surfacing must be removed to saw cut lines, or to existing wood dividers or expansion joints, if any. The saw cut must be to a neat line and have a depth not less than one inch.

Restoration: Surfacing must be replaced to match the thickness, grades and finish of the adjacent surrounding surfaces.

99-02220C(2) Structure Excavation

Unless otherwise noted, all excavation for building work must be classified as structure excavation.

Footing Excavation:

The bottom of excavation must not be disturbed. The contractor must excavate by hand to the final grade. The bottom of concrete footings must be poured against undisturbed material. Unless otherwise noted, compaction of the bottom of footing excavation is not required unless the material is disturbed. The footing depths shown must be changed to suit field conditions when directed by the Engineer. Solid rock at or near required depths must not be disturbed. Unsuitable material must be excavated down to firm bearing as directed by the Engineer. Work and materials required because of excavation in excess of the depths shown, when such excavation has been ordered by the Engineer, will be change order work.

Excavate to the elevations and dimensions within a tolerance of ±1/2 inch. Limits of the excavation must allow for adequate working space for installing materials and as required for safety of personnel. Such working space excavation must be replaced in kind and compacted at the Contractor's expense.

Overdepth excavation for footings must be backfilled with concrete or such other material recommended by the Contractor and authorized by the Engineer. Relative compaction must be not less than 95 percent.

At locations and to the limits shown, material below the bottom of the foundation or footing must be removed and replaced with select backfill under the placing and compacting requirements for backfill.

Excavation for Pipes and Conduits:

Pipes or conduits in the same trench must have a minimum clear distance between pipes or conduits of 6 inches. Pipes or conduits must have not less than 2-1/2 feet of cover from top of pipes or conduits to finished grade unless otherwise shown or described.

Trenching must be of sufficient depth to permit placing a minimum depth of 4 inches of compacted sand under all pipes and conduits.

Excavation adjacent to trees must be performed by hand methods where necessary to avoid injury to trees and roots. Roots 2 inches in diameter and larger must be protected with heavy burlap. Roots smaller than 2 inches in diameter adjacent to trees must be hand trimmed. Cuts through roots 1/2 inch in diameter and larger must be sealed with tree trimmers' asphaltic emulsion. If trenches remain open more than 24 hours, the side of the trench adjacent to the tree must be shaded with burlap and kept damp. Materials must not be stockpiled within the drip line of trees.

Dewatering: Excavations must be kept clear of standing water. Water must be removed by pumping if necessary. Water removed from excavation must be carried away from the building site and disposed of.

99-02220C(3) Structure Backfilling

Unless otherwise noted, all backfill for building work must be classified as structure backfill. Backfill must be placed and compacted in horizontal layers, not more than 6 inches thick prior to compaction, and to the lines and grades shown or to original ground.

Structure Backfill: After structures are in place and forms are removed, wood and other debris must be removed from excavations before placing structure backfill.

Backfilling Pipes and Conduits:

Backfill placed under pipe and conduits must be compacted sand, 4 inches minimum depth. Backfill material placed to a level 6 inches above tops of pipes and conduits must be sand or fine earth and particles must not exceed 1/2 inch in greatest dimension. For wrapped, coated, or plastic pipe or conduits, sand must be used for backfill. Backfill material placed higher than 6 inches above tops of pipes or conduits must consist of material free of stones or lumps exceeding 4 inches in greatest dimension except:

1. The top 12 inches of backfill under roads, walks or paving must consist of aggregate base material.
2. The top 6 inches of backfill in planted areas must consist of topsoil.

Unless otherwise shown, pipe under roads, with less than 2-1/2 feet of cover over the top of pipe, must be backfilled with concrete to a level 4 inches above the top of pipe. Concrete for backfill must be commercial quality concrete containing not less than 590 pounds of cement per cubic yard.

99-02220C(4) Compaction

Relative compaction must be determined under California Test 216 or 231.

Unless otherwise noted below, all backfill must be compacted to a minimum relative compaction of 90 percent.

Unless authorized, compaction by jetting or ponding will not be permitted.

Compact Original Ground: Original ground surface under fill with surfacing of concrete and asphalt concrete must be compacted to a relative compaction of not less than 95 percent for a minimum depth of 6 inches.

Subgrade Preparation:

Preparation of subgrade material for placing aggregate base, surfacing, or slabs thereon must include fine grading, compaction, reworking as necessary. The upper 6 inches of the subgrade must have the same compaction as the fill to be placed over it.

The prism of backfill directly underneath the building foundation and sloping downward at 1:1 must be compacted to 95 percent.

Structure Backfill: Structure backfill must be compacted to not less than 95 percent relative compaction.

Select Backfill:

Select backfill must be compacted to not less than 95 percent relative compaction.

A relative compaction of not less than 95 percent must be obtained for a minimum depth of 6 inches below the bottom of the excavation before placing select backfill.

Trench Backfill: Trench backfill placed beneath slabs or paved areas must be compacted to a relative compaction of not less than 95 percent.

99-02220C(5) Disposal

Surplus Material: Surplus material from the excavation must be disposed of at the site of the work as directed by the Engineer.

99-02220C(6) Field Quality Control

Inspection: When the excavation is substantially completed to grade, you must notify the Engineer. No concrete must be placed until the foundation has been authorized by the Engineer.

Testing: The Department will conduct compaction tests during the backfilling and compacting operations.

99-02220D Payment

Not Used

99-02236 FREE DRAINING GRANULAR MATERIAL

99-02236A General

99-02236A(1) Summary

Scope: This work consists of furnishing and placing free draining granular material beneath slabs.

99-02236A(2) Definitions

Not Used

99-02236A(3) Submittals

Not Used

99-02236A(4) Quality Control and Assurance

Not Used

99-02236B Materials

Free Draining Granular Material: Free draining granular material must be clean, hard, durable, free-draining rock. The material gradation must be such that all passes the one-inch screen, and not more than 10 percent passes the No. 4 sieve as determined by California Test 202. Granular material must be free from organic material, clay balls, or other deleterious substances.

99-02236C Construction

Free draining granular material must be placed, spread, and consolidated by tamping or vibrating.

99-02236D Payment

Not Used

99-3 CONCRETE AND REINFORCEMENT

99-03300 CAST-IN-PLACE CONCRETE

99-03300A General

99-03300A(1) Summary

Scope: This work consists of constructing cast-in-place concrete facilities.

Concrete:

Except for concrete used for minor work, concrete must comply with section 90. The minimum required compressive strength must be as described or 3,600 psi at 28 days, whichever is greater.

Concrete for minor work must comply with section 90-2.

Reinforcement: Reinforcement must comply with section 52, except you may use deformed bars complying with ASTM A 615/A 615M, Grade 60.

99-03300A(2) Definitions

Not Used

99-03300A(3) Submittals

Product Data:

Manufacturer's descriptive data, installation and use instructions for admixtures, expansion joint material, vapor barrier, curing compound, hardener, and sealer must be submitted.

Descriptive data must be delivered to the Engineer at the job site.

Concrete Mix Designs: Submit copies of concrete mix designs.

Certificates of Compliance: Submit a certificate of compliance when required.

99-03300A(4) Quality Control and Assurance

Not Used

99-03300B Materials

99-03300B(1) Concrete Mixes

The amount of cementitious material used per cubic yard of concrete for each building element must comply with the following:

Type	Cementitious Material Content (Pounds/CY)
Concrete (Structural Work): Footings, foundation walls, floor slabs, building frame members, building walls	590 min.[a,]
Concrete (Sewer Structures): For sewer structures, vehicle washracks and mudrinse slabs	658 min.[b]
Concrete (Minor Work): For concrete curbs, sidewalks, driveways, gutter depressions, new door openingsand collars	505 min.

Notes:
[a]For concrete designated by compressive strength, the maximum amount of cementitious material must be 800 pounds per cubic yard.
[b]Concrete must be air entrained under section 90-1.02E. The air content at time of mixing and prior to placing must be 6 ± 1-1/2 percent.

99-03300B(2) Colored Concrete

Not Used

99-03300B(3) Form Materials

Forms for Exposed Finish Concrete:

Forms for exposed surfaces must be plywood, metal or other panel type materials. Plywood must be not less than 5/8 inch thick and without scars, dents, and delaminations. Forms must be furnished in largest practical pieces to minimize number of joints.

Plywood must comply with the requirements of U. S. Product Standard PS-1 for Exterior B-B (Concrete Form) Class I.

Forms for edges of slabs must be nominal 2-inch solid stock lumber, plywood, or metal forms.

Forms for Unexposed Finish Concrete: Forms for unexposed finish concrete surfaces must be plywood, lumber, metal, or other acceptable material.

Forms for Cylindrical Columns or Supports: Forms for cylindrical columns must be metal, fiberglass reinforced plastic, paper, or fiber tubes. Paper or fiber tubes must be constructed of laminated plies using water-resistant adhesive with wax-impregnated exterior for protection against weather or moisture.

Form Ties: Form ties must be factory fabricated, removable or snapoff metal ties for use as necessary to prevent spreading of forms during concrete placement.

Form Oil: Form oil must be commercial quality form oil which will permit the ready release of the forms and will not discolor the concrete.

99-03300B(4) Reinforcement

Not Used

99-03300B(5) Epoxy

Epoxy must be furnished as 2 components which must be mixed together at the site of the work.

Epoxy Resin Adhesive: Epoxy resin adhesive must comply with State of California Specification No. 8040-21M-08 or other epoxy suitable for bonding new concrete to old.

Epoxy Mortars: Epoxy mortar and epoxy mortar surface treatment must consist of a commercial quality, trowelable mixture consisting of epoxy and sand. Epoxy must have a pull-off strength of not less than 1,000 psi and a 90-percent cure in 24 hours. Epoxy must be of the type that requires no primer as a bonding agent.

Sand:

Sand for use in epoxy mortars must be clean and must have a moisture content of not more than 0.50-percent when tested under California Test 226.

Sand for epoxy mortar surface treatment must be graded such that 100-percent passes the No. 100 sieve.

99-03300B(6) Related Materials

Anchor Bolts and Anchor Rods, Nuts and Washers:

Headed and Unheaded Anchor Bolts and Anchor Rods: Comply with ASTM F 1554. Use Grade 36 unless a higher grade is shown.

Nuts: Comply with ASTM A 563.

Washers:

1. Washers bearing on wood surfaces must be commercial quality.
2. Washers bearing on steel surfaces must comply with ASTM F 436, Type 1.
3. Plate washers must comply with ASTM A 36/A 36M.

Exposed anchor bolts and anchor rods, nuts and washers must be hot-dipped galvanized.

Expansion Joint Material: Expansion joint material must be commercial quality asphalt impregnated pressed fiber sheets, 1/2-inch minimum thickness.

Vapor Barrier: Vapor barrier must be not less than 15 mils thick and must comply with the requirements of ASTM E 1745, Grade A. Tape for overlapped seams must be as recommended by the manufacturer of the vapor barrier.

Bond Breaker: Bond breaker must be Type I asphalt saturated organic felt or such other material authorized by the Engineer.

Nonskid Abrasive Aggregate: Nonskid abrasive aggregate must be commercial quality aluminum oxide, silicon carbide, or almandite garnet grit particles; screen size 12-30 or 14-36.

Type A Control Joints: Type A control joints must be commercial quality, preformed, T-shaped plastic strips with detachable top flange.

Keyed Construction Joint Forms: Keyed construction joint forms must be commercial quality, galvanized metal or plastic, factory fabricated construction joint forms. Forms must produce a rabbeted key type joint.

Divider and Edger Strips: Divider and edger strips must be foundation grade redwood.

Mortar: Mortar must consist of one part cement to 2 parts clean sand and only enough water to permit placing and packing.

Curing Compound: Curing compound must be curing compound no. 6.

Concrete Hardener: Concrete hardener must be commercial quality water borne penetrating type magnesium fluosilicate, zinc fluosilicate or combination thereof.

Splash Block: Splash blocks must be precast concrete splash blocks with depressed runoff trough. Splash blocks must be 12 by 24 by 3-1/2 inch in size unless otherwise shown.

Nonshrink Grout:

Nonshrink grout must be metallic for concealed areas, nonmetallic for exposed areas.

Grout must be factory packaged, nonstaining, noncorrosive, nongaseous grout complying with ASTM C 1107; free of oxidizing catalysts and inorganic accelerators, used as dry or damp pack, or mixed to a 20-second flow (CRD C621), without segregation or bleeding at any temperature between 45 deg F and 90 deg F.

Working time of grout must be 30 minutes or more.

99-03300C Construction

99-03300C(1) Preparation

Existing Concrete Construction:

Where fresh concrete joins existing or previously placed concrete or masonry, the contact surfaces of the existing or previously placed material must be roughened, cleaned, flushed with water and allowed to dry to a surface dry condition immediately prior to placing the fresh concrete. The roughened surface must be no smoother than a wood trowelled surface. Cleaning of the contact surfaces must remove laitance, curing compounds, debris, dirt and such other substances or materials which would prevent bonding of the fresh concrete.

Abrasive blast methods must be used to clean horizontal construction joints to the extent that clean aggregate is exposed.

Exposed reinforcing steel located at the contact surfaces which is to be encased in the fresh concrete must be cleaned to remove any substance or material that would prevent bonding of the fresh concrete.

Forms:

Forms must be mortar tight, true to the dimensions, lines, and grades shown, securely fastened and supported, and of adequate rigidity to prevent distortion during placing of concrete.

Forms for exposed surfaces must be constructed with triangular fillets not less than 3/4 by 3/4 inch attached so as to prevent mortar runs and to produce smooth straight chamfers at all sharp edges of the concrete.

Form fasteners must be removable without chipping, spalling, heating or otherwise damaging the concrete surface. Form ties must be removed to a depth of at least one inch below the surface of the concrete.

The inside surfaces of forms must be cleaned of all dirt, mortar and foreign material. Forms must be thoroughly coated with form oil prior to use.

Forms must not be stripped until at least 40 hours after placing concrete, except soffit forms and supports must not be released or removed until at least 10 days after placing concrete.

Anchorages and embedded items must be placed and rigidly secured at their planned locations prior to placing concrete.

Reglets or embedded flashing must be installed on concrete forms before the concrete is placed.

Redwood dividers must have 16d galvanized nails partially driven into both vertical faces at 18 inches on center.

Vapor Barrier:

Vapor barrier must be installed under the manufacturer's instructions and must be protected with a 3-inch layer of clean uncompacted sand cover.

Placing Reinforcement:

If authorized, you may use plastic supports to hold reinforcement in position.

Set wire ties with ends directed into concrete, away from exposed concrete surfaces.

Install welded wire reinforcement in longest practicable lengths on bar supports spaced to minimize sagging. Lap edges and ends of adjoining sheets at least one mesh spacing. Offset laps of adjoining sheet widths to prevent continuous laps in either direction. Lace overlaps with wire.

Ground Bar: A continuous reinforcing steel bar must be installed in the building foundation at the location shown for the electrical ground bar. The use of epoxy coated reinforcing bar is not permitted. The end of the ground bar must extend beyond the concrete surface and must be protected from damage by construction operations.

Hydronic Tubing:

Hydronic tubing must be securely fastened to the bar reinforcing using nylon ties.

The hydronic heating system must be fully tested prior to placing concrete.

99-03300C(2) Placing Concrete

Concrete must be placed under section 51-1.03D.

Concrete must be deposited and consolidated in a continuous operation within limits of construction joints, until the placing of the panel or section is completed.

When concrete is to be placed in large areas requiring more than two pours, concrete must be placed in alternate long strips between construction joints and the final slab infilled.

99-03300C(3) Colored Concrete

Not Used

99-03300C(4) Finishing Concrete Surfaces

Finishing Unformed Surfaces:

Slabs must be placed full thickness to finish elevation and leveled to screeds by use of long straightedges. The screeds must be set to grade at approximately 6-foot centers. After leveling, screeds must be removed and the surface must be floated with wooden floats.

Type A control joint strips must be inserted into the floated concrete so that the bottom of the top flange is flush with the finish elevation. Strips must be standard manufactured lengths and must be placed on an approximate straight line. The top flange of the strips must be removed after the concrete has set and cured.

The floated surface must be trowelled with steel trowels. Troweling must form a dense, smooth and true finish. Walkways, pedestrian ramps, stairs and outdoor slabs for pedestrian traffic must be given a non-slip broom finish unless a different finish is described.

The application of cement dust coat will not be permitted.

Steel trowel finish and broom finish will not be required for slabs to receive exposed aggregate finish nor for slabs to be covered with ceramic tile.

Concrete floor surfaces to receive ceramic tile must be floated to grade and then, before final set of the concrete, the floated surfaces must be roughened with stiff bristled brushes or rakes.

Finished surfaces of floor slabs must not deviate more than 1/8 inch from the lower edge of a 10-foot long straight edge.

Finishing Formed Surfaces:

Formed concrete surfaces must be finished by filling holes or depressions in the surface, repairing all rock pockets, and removing fins. All surfaces of formed concrete exposed to view must have stains and discolorations removed, unsightly bulges removed, and all areas which do not exhibit the required smooth, even surface of uniform texture and appearance must be sanded with power sanders or other authorized abrasive means until smooth, even surfaces of uniform texture and appearance are obtained.

Cement mortar, patching and finishing materials used to finish exposed surfaces of concrete must closely match the color of surrounding surfaces.

Nonskid Abrasive Aggregate Finish: Where shown, walkways must receive a nonskid abrasive aggregate (grit) finish. The grit must be applied uniformly at the rate of not less than 0.3 pound per square foot and tamped into the floated concrete surface while the concrete is plastic. The grit must be buried about 0.7 diameter of each particle into the concrete.

99-03300C(5) Curing Concrete

Freshly placed concrete must be protected from premature drying and excessive cold or hot temperatures.

Floor slabs must be cured by the water method as specified for structures. Initial curing of floor slabs must start as soon as free water has disappeared from the concrete surface.

Concrete surfaces, other than floor slabs, must be cured by the forms-in-place method or the water method as specified for structures.

Concrete curbs, sidewalks, collars, and gutter depressions may be cured by the curing compound method.

99-03300C(6) Protecting Concrete

Vehicles, equipment, or concentrated loads weighing more than 300 pounds individually and material stockpiles weighing more than 50 pounds per square foot will not be permitted on the concrete within 10 days after placing.

99-03300C(7) Special Treatments

Concrete Hardener:

Chemical concrete hardener must be applied to the floor surfaces shown, prior to the application of concrete sealer. Surfaces must be clean and dry before the application of hardener.

The solution must be applied under the manufacturer's instructions.

After the hardener has dried, the surface must be mopped with water to remove encrusted salts.

Concrete Sealer: Concrete sealer must be applied to the concrete surfaces designated on the plans under the manufacturer's instructions for heavy duty use. The sealer must be applied to dry concrete surfaces.

Epoxy Resin Adhesive: Epoxy resin adhesive must be applied to concrete surfaces shown. Epoxy resin adhesive must be mixed and applied under the manufacturer's instructions.

Epoxy Mortars:

> Epoxy for use as a binder in epoxy mortars must be thoroughly mixed together before the aggregate is added, and unless otherwise specified, the mix proportions must consist of one part binder to approximately 4 parts of aggregate, by volume.
>
> All surfaces against which epoxy mortars are to be applied must be free of rust, paint, grease, asphalt, and loose or deleterious material.

99-03300D Payment

Not Used

99-4 MASONRY

99-04221 PREFACED MASONRY UNITS

99-04221A General

99-04221A(1) Summary

Scope: This work consists of furnishing and installing prefaced concrete masonry units.

The requirements of this special provision are in addition to the applicable requirements for block work, reinforcing, grouting and other details applicable to masonry construction.

99-04221A(2) Definitions

Not Used

99-04221A(3) Submittals

Product Data: Submit manufacturer’s descriptive data and installation instructions for prefaced masonry units.

Certificate of Compliance: Submit certificate of compliance for prefaced masonry units.

99-04221A(4) Quality Control and Assurance

Single Source Responsibility: Obtain masonry units of uniform texture, color, and manufacture from a single source.

Construct sample panel 48 by 48 inch for Engineer's authorization of colors, color blending, textures, tooling, and aesthetic qualities of workmanship.

99-04221A(5) Delivery, Storage, and Handling

Delivery: Masonry units must be delivered on covered pallets with individual, glazed faces protected from damage.

Storage: Masonry units must be stored in a dry space, protected from damage and weather. Pallets must not be stacked. Excessive handling prior to installation must be avoided.

99-04221B Materials

Acceptable Manufacturers: Subject to contract requirements, prefaced concrete masonry units must be Spectra Industries, Inc., Spectra-Glaze; Trenwyth Industries, Inc., Astra-Glaze-SW-+; Premier Block Corporation, Premier Glazed; or equal.

Unit Construction: Provide lightweight or medium weight hollow concrete masonry units that comply with ASTM C90.

Fire Resistance: For fire-rated construction, provide units rated by testing under ASTM E 119, by UL rating under UL 263 or UL 618, by equivalent masonry thickness, or by other acceptable means to demonstrate required fire-resistance rating.

Sound Absorption: Provide sound-reducing masonry units where indicated, having a noise reduction coefficient (NRC) of not less than 0.5 and a sound transmission class (STC) of not less than 50.

Glazed Face:

Glazed face must be a factory applied compound of resin, resin and inert filler, or cement and inert filler, to produce a smooth resinous tile facing. Glazed face after curing must comply with ASTM C 744.

Glazed face must be the color shown.

Special shapes (bullnose, cove, etc.) must be provided as shown.

Mortar: If recommended by prefaced masonry unit manufacturer, use mortar containing a liquid integral water repellent admixture approved by masonry unit manufacturer.

99-04221C Construction

99-04221C(1) General

Not Used

99-04221C(2) Masonry Unit Installation

Prefaced masonry units must be aligned faces plumb and level to comply with acceptable masonry practice.

Mortar joints must be 1/4 inch on exposed surfaces allowing for a standard 3/8-inch joint between unglazed portions of the masonry units.

When shown, joints to be sealed must be raked to a depth of 3/8 inch and the wall must be pointed, floated, gunned or bagged with an authorized fine textured sanitary grout containing hardening and/or waterproofing agents.

Joints must be tooled slightly concave with glass or other non-staining tool, to join rounded block edge. Factory scores must be filled and tooled at the same time as the field joints.

99-04221C(3) Cleaning

Excess mortar must be removed as the work progresses.

Final cleaning of the glazed wall must comply with the facing manufacturer's instructions.

The finished work must be protected from spillage or other damage.

99-04221D Payment

Not Used

99-04230 CONCRETE MASONRY UNITS

99-04230A General

99-04230A(1) Summary

This work includes:

1. Concrete masonry units
2. Mortar and grout
3. Bar reinforcement
4. Ties and anchors

99-04230A(2) Related Work

Water Repellent Coating: Comply with section 99-07182.

99-04230A(3) Definitions

CMU: Concrete masonry unit.

99-04230A(4) Performance Requirements

Masonry Compressive Strength: Provide masonry that develops the compressive strength (f'm) shown at 28 days.

99-04230A(5) Submittals

Product Data: Submit manufacturer's descriptive data for each type of masonry unit, accessory, and manufactured product.

Samples: Submit two sample CMUs for each color and architectural finish.

Certificates of Compliance: Submit certificate of compliance for CMUs, aggregate for grout, and ready-mixed grout.

Grout Mix Design: Submit a grout mix design for each grout mix proposed for use. Submit a revised grout mix design for any proposed change to the proportions of an authorized grout mix.

Shop Drawings: Submit calculations and shop drawings for temporary supports of masonry lintels. Design and construct temporary supports to provide the necessary rigidity and to support loads that will be applied. Shop drawings and calculations must be sealed and signed by an engineer who is registered as a civil engineer in the State.

Qualification Data: Submit qualification data for proposed testing laboratory.

Special Inspector Final Report: The special inspector must submit a final signed report to the Engineer and to you stating whether the work requiring special inspection, to the best of the inspector's knowledge, complies with the authorized plans, specifications, and the applicable workmanship provisions of these special provisions and the 24 CA Code of Regs Pt 2.

Field Quality Control Plan:

Submit a written Field Quality Control Plan that identifies the inspector, the testing laboratory, and the procedures to be used. The plan must comply with these specifications and the CBC.

Designate a Masonry Quality Control Manager (MQCM) in the Field Quality Control Plan. The MQCM must be directly responsible to you for the quality of masonry, including materials and workmanship, performed by you and your subcontractors.

The MQCM must be the sole individual responsible to you for submitting, receiving, and approving all correspondence, required submittals, and reports to and from the Engineer.

The MQCM must not be employed or compensated by any subcontractor, or by other persons or entities hired by subcontractors, who will provide other services or materials for the project. The MQCM may be your employee.

99-04230A(6) Quality Control and Assurance

Masonry Preconstruction Testing:

Employ and pay all costs for the services of a testing laboratory experienced in performing preconstruction masonry tests.

The testing laboratory must comply with ASTM E 329.

Perform preconstruction testing to determine masonry compressive strength for each grout mix to be used under one of the following 24 CA Code of Regs Pt 2 test methods:

1. § 2105.2.2.1, except the grout must comply with § 2105.2.2.1.2, "Concrete Masonry," 3.3.2.
2. § 2105.2.2.2. The grout compressive strength must also be tested under ASTM C 1019 and be equal to or greater than the masonry compressive strength, but not less than 2,000 psi.

The grout compressive strength at 28 days must be 85 percent or more of the minimum required grout compressive strength.

The testing laboratory must report test results to the Engineer and you, on the same day the tests are made.

Do not perform masonry work until the required masonry and grout compressive strengths are attained.

Single Source Responsibility:

For each product required, obtain exposed CMUs of a uniform color and architectural finish from a single source and from a single manufacturer.

Obtain mortar ingredients of a uniform quality, including color for exposed masonry, from a single manufacturer for each cementitious component and from a single source or producer for each aggregate.

99-04230A(7) Delivery, Storage and Handling

Delivery: Deliver masonry materials to the project in an undamaged condition.

Storage:

Store CMUs on elevated platforms in a dry location. If CMUs are not stored in an enclosed location, cover tops and sides of stacks with waterproof sheeting, securely tied. If CMUs become wet, do not install until they are dry.

Store cementitious materials on elevated platforms and in a dry location. Do not use cementitious materials that have become damp.

Store aggregates where grading and other required characteristics can be maintained and contamination avoided.

Store masonry accessories, including metal items, to prevent corrosion and accumulation of dirt and oil.

Handling: Prevent damage, contamination, corrosion, or other deterioration to masonry materials when handling.

99-04230(8) Project Conditions

Protection of Masonry: Cover partially completed masonry, tops of walls, projections, and sills with waterproof sheeting when construction is not in progress.

Stain Prevention:

Prevent grout, mortar, and soil from staining the face of masonry to be left exposed or painted. Immediately remove grout, mortar, and soil that come in contact with such masonry.

Protect the base of walls from rain-splashed mud and from mortar splatter by spreading coverings on the ground and over wall surfaces.

Protect sills, ledges, and projections from mortar droppings. Protect surfaces of window and door frames, and other similar products with painted and integral finishes, from mortar droppings.

99-04230B Materials

99-04230B(1) Concrete Masonry Units

Concrete Masonry Units:

Provide CMUs of nominal size, color, and architectural finish as shown. Provide hollow, load bearing, light weight or medium weight, open-ended CMUs that comply with ASTM C 90.

Provide special shapes for lintels, corners, jambs, sash, control joints, headers, bonding, and other special conditions.

99-04230B(2) Masonry Lintels

Masonry Lintels: Construct built-in-place masonry lintels from bond beam CMUs, with reinforcing bars placed as shown.

99-04230B(3) Mortar and Grout Materials

Mortar Materials:

Mortar: Comply with ASTM C 270.

Cement: Comply with ASTM C 150, Type II, low alkali portland cement.

Lime: Comply with ASTM C 207, Type S.

Aggregate: Comply with ASTM C 144, with maximum 10 percent passing the No. 100 sieve.

Coloring for Mortar: Chemically inert, fade resistant mineral oxide or synthetic type.

Premixed Mortar: A premixed packaged blend containing only cement, lime, and sand, with or without color, that requires only water to prepare for use as masonry mortar. Packages of premix must bear the manufacturer's name, brand, contents, weight, and color identification.

Grout Materials:

Grout must comply with ASTM C 476, coarse grout, except for grout proportions. Grout proportions must comply with "Grout Mixes" in section 99-04230B(6).

Cementitious material must comply with section 90-1.02B.

Aggregate: Comply with ASTM C 404, except 100 percent of the coarse aggregate must pass the 3/8-inch sieve. Soundness loss must not exceed 10 percent when tested under California Test 214.

99-04230B(4) Reinforcement

Bar Reinforcement: Comply with ASTM A 615/A 615 M, Grade 60, or ASTM A 706/A 706 M.

99-04230B(5) Ties and Anchors

Anchors, Ties, Angles, and Metal Lath: Commercial quality and galvanized.

Anchor Bolts and Anchor Rods, Nuts and Washers:

Headed and Unheaded Anchor Bolts and Anchor rods: Comply with ASTM F 1554. Use Grade 36 unless a higher grade is shown.

Nuts: Comply with ASTM A 563.

Washers: Comply with ASTM F 436.

99-04230B(6) Mortar and Grout Mixes

Dry Pack Mortar: One part portland cement to not over 3 parts of clean sand, mixed with a minimum amount of water for hydration and packing.

Mortar Mix: Proportion by loose volume with one part cement, 1/4 to 1/2 part hydrated lime, and 2-1/4 to 3 parts aggregate. Tint with coloring to match the masonry units.

Premixed Mortar: Packages of premixed mortar must consist of one part cement, 1/4 to 1/2 part hydrated lime, and 2-1/4 to 3 parts aggregate, and no other ingredients.

Grout Mixes:

Grout proportions must be determined by one of the following methods:

1. Proportioned by volume complying with ASTM C 476, Table 1.
2. Specified compressive strength complying with "Grout Mix Design" in section 99-04230A(5).

Mix with sufficient water to produce a mix consistency suitable for pumping without segregation. Provide grout with a slump of 8 to 11 inches.

Nonshrink Grout: Comply with ASTM C 1107, match color with the mortar.

Aggregate: Measure in a damp, loose condition.

99-04230C Construction

99-04230C(1) Installation

Comply with 24 CA Code of Regs Pt 2 § 2104. Comply with tolerances specified in § 2104 unless otherwise shown.

Lay masonry units in running bond, unless otherwise shown.

Roughen, clean, and lightly wet the contact surfaces of existing material where fresh masonry joins concrete or masonry. The roughened surface must be at least as rough as a wood troweled surface. Remove laitance, curing compounds, debris, dirt, and any substance which decreases bond to the fresh masonry.

Erect masonry only when the ambient air temperature is above 40 degrees F.

If masonry is erected when the ambient air temperature exceeds 100 degrees F, keep surfaces of masonry moist with water for at least 24 hours. Uniformly apply water with a fog spray at time intervals required to keep the surfaces moist, but no more than 3 hours between applications.

Firmly secure anchors, bolts, dowels, reglets, and other miscellaneous items to be cast into the wall in place before grout is poured.

Laying Masonry Units:

1. Lay CMUs dry.
2. Keep cells dry during laying of units in inclement weather by covering incomplete walls. Do not use wooden boards and planks as covering materials. Extend covering down each side of masonry walls approximately 2 feet.
3. Keep chases free from debris and mortar.
4. Use bond beam units with an opening at each cross web at horizontal reinforcing bars.
5. Make cuts with a masonry saw to neat and true lines. Do not use blocks with excessive cracking or chipping of the finished surfaces exposed to view.
6. Leave openings to inset outlet boxes, access control keypads, intrusion detection sensors, and similar components in to masonry, with concealed wiring.

Bar Reinforcement:

Position bar reinforcement as shown and securely hold in position with either wire ties or spacing devices near the ends of bars and at intervals not exceeding 192 bar diameters. Wire must be 16-gauge or heavier. Do not use wooden, aluminum, or plastic spacing devices. Tolerances for the placement of vertical reinforcement in walls and flexural elements must be ±1/2 inch. Tolerances for longitudinal reinforcement in walls must be ±2 inches.

The minimum spacing for splices in vertical reinforcement for masonry walls must be 4 feet plus lap.

Do not place bar reinforcement in the plane of mortar joints.

Mortar:

Mortar joints must be approximately 3/8 inch thick. Lay units with all head and bed joints filled solidly with mortar for the full width of masonry unit shell. Shove head joints tight. Exposed joints must be concave and tooled smooth, unless otherwise shown.

Mortar that has been mixed more than one hour must not be retempered.

Preserve the unobstructed vertical continuity of the grout during mortar placement in joints. Any overhanging mortar projecting more than 1/2 inch, or other obstruction or debris must be removed from the inside of cells.

Pointing:

During the tooling of joints, enlarge voids and holes, except weep holes, and completely fill with mortar. Point up joints, including corners, openings, and adjacent construction, to provide a neat, uniform appearance. Prepare joints for sealant application as shown.

Remove splashes, stains, and spots on the faces of the masonry exposed to view.

99-04230C(2) Lintels

Temporarily support formed-in-place lintels for a minimum of 15 days after the wall has been completed.

99-04230C(3) Grouting

Place CMUs full height of the masonry wall before grouting, or place CMUs full height of the grout pour before grouting.

For grout pours over 5 feet in height, provide cleanouts in the bottom course at every cell containing vertical reinforcement, at a maximum spacing of 32 inches. Each cleanout must be at least 3 inches square. After cell inspection, seal cleanouts before filling with grout.

Place grout in a continuous pour in grout lifts. Grout lifts must not exceed 5 feet. The interruption between placing successive grout lifts must be no more than one hour. If the interruption is more than one hour, another grout pour must be used.

Solidly fill cells with grout. For each grout lift, consolidate grout in the cells at the time of placement by vibrating and reconsolidated after excess moisture has been absorbed but before plasticity is lost.

Between grout pours, form a horizontal construction joint by stopping the grout a minimum of 1-1/2 inches below the top of the last course, except if the joint is at a bond beam, the joint must be 1/2 inch below the top of the bond beam unit or at the top of the wall.

99-04230C(4) Field Quality Control

You must employ a special inspector and a testing laboratory to perform inspections and structural tests of masonry to verify the masonry construction complies with 24 CA Code of Regs Pt 2 § 1704 and § 2105.

Masonry special inspection personnel or testing laboratories used in the work must not be employed or compensated by any subcontractor, or by other persons or entities hired by subcontractors, who will provide other services or materials for the project.

Special Inspector:

The special inspector must be, as a minimum, an ICC certified Structural Masonry Special Inspector. The special inspector must perform the inspections required under § 1704.5.

The special inspector must prepare a "Daily Field Report" providing information regarding the specific operations witnessed, including placing of CMUs and bar reinforcing, grouting, fabrication of test specimens, and other observations of importance to the work.

A "Daily Field Report" is required for each day that the special inspector is on the job site. Deliver a copy of these reports to the Engineer on the day following the preparation.

Testing:

Perform testing of masonry for every 5,000 square feet of CMU wall area, or portion thereof.

Determine masonry compressive strength under "Masonry Preconstruction Testing" in section 99-04230A(6).

Any work not meeting the requirements of 24 CA Code of Regs Pt 2 § 2105 must be redone and retested. Sampling, inspecting, reworking, and retesting of material will be done at your expense.

99-04230C(5) Repairs and Protection

Remove and replace CMUs that are loose, chipped, broken, stained, do not match adjoining units, or are otherwise damaged. Install new units in fresh mortar to match adjoining units, and point joints to eliminate evidence of replacement.

Protect completed masonry from freezing for a period of at least 5 days.

99-04230D Payment

Not Used

99-5 METALS

99-05500 BUILDING MISCELLANEOUS METAL

99-05500A General

99-05500A(1) Summary

Scope: This work consists of fabricating and installing building miscellaneous metal.

Building miscellaneous metal consists of the following:

1. Metal Clips
2. Anchor Bolts

Including all anchors, fastenings, hardware, accessories, and other supplementary parts necessary to complete the work.

99-05500A(2) References

Codes and Standards: Welding of steel must comply with AWS D 1.1, "Structural Welding Code - Steel" and D 1.3, “Structural Welding Code - Sheet Steel.”

99-05500A(3) Definitions

Not Used

99-05500A(4) Submittals

Product Data: Submit manufacturer's specifications, anchor details, and installation instructions for products used in miscellaneous metal fabrications.

Shop Drawings: Shop drawings of fabricated items must be submitted.

99-05500A(5) Quality Control and Assurance

Shop Assembly: Preassemble items in shop to the greatest extent possible to minimize field splicing and assembly. Disassemble units only as necessary for shipping and handling limitations. Clearly mark all units for reassembly and installation.

Inspection and Tests: Materials and fabrication procedures must be subject to inspection and tests by the Engineer, in mill, shop, and field.

99-05500B Materials

99-05500B(1) General

Steel Bars, Plates, and Hot-rolled Shapes: Steel bars, plates, and hot-rolled shapes must comply with ASTM A 36/A 36M.

Galvanized Sheet Steel: Galvanized sheet steel must comply with ASTM A 653/A 653M. Galvanizing must be G60.

Checkered Floor Plates: Checkered floor plates must be commercial quality steel with standard raised pattern.

Pipe: Pipe must be commercial quality standard steel pipe.

Hollow Structural Sections: Hollow structural sections must comply with ASTM A 500/A 500M, Grade B, or A 501.

Bolts, Studs, Threaded Rods, Nuts, and Washers:

Bolts, studs, and threaded rods for general application must comply with ASTM A 307 or F 1554, Grade 36.

Nuts must comply with ASTM A 563.

Washers bearing on wood surfaces must be commercial quality. Washers bearing on steel surfaces must comply with ASTM F 844 or F 436.

Fittings: Brackets, bolt, threaded studs, nuts, washers, and other fittings for railings and handrailings must be commercial quality pipe and fittings.

Expansion Anchors: Expansion anchors must be ICC approved for the purpose intended, integral stud type anchor or internally threaded type with independent stud, hex nut, and washer.

Powder Driven Anchors: Powder driven anchors must be plated, spring steel alloy drive pin or threaded stud type anchors for use in concrete or steel. Spring steel must comply with ASTM A 227, Class 1. The diameter, length, and type of shank and the number and type of washer must be as recommended by the manufacturer for the types and thickness of material being anchored or fastened.

Resin Capsule Anchors: Stud anchors for resin capsule anchors must comply with ASTM A 307 or F 1554, Grade 36, threaded steel rod with hex nut and washer and sealed glass capsule or cartridge containing an adhesive composed of unsaturated polyester resin and benzol peroxide coated quartz sand. Resin capsule must be Hilti; Molly; or equal.

Drainage Grates: Drainage grates must be fabricated from steel bars as specified herein; ductile iron castings complying with ASTM A 536, Grade 65-45-12; or carbon steel castings complying with ASTM A 27, Grade 65-35.

Mortar: Mortar must consist of one part cement, measured by volume, to 2 parts clean sand and only enough water to permit placing and packing.

99-05500B(2) Shop Fabrication

Workmanship and Finish:

Workmanship and finish must be equal to the best general practice in modern shops.

Miscellaneous metal must be clean and free from loose mill scale, flake rust and rust pitting, and must be well formed and finished to shape and size with sharp lines and angles. Bends from shearing or punching must be straightened.

The thickness of metal and details of assembly and support must give ample strength and stiffness.

Built-up parts must be true to line and without sharp bends, twists, and kinks. Exposed ends and edges of metal must be milled or ground smooth, with corners slightly rounded.

Joints exposed to the weather must be made up to exclude water.

Galvanizing: Items indicated on the plans to be galvanized must be hot-dip galvanized after fabrication. The weight of galvanized coating must be at least 1-1/2 ounces per square foot of surface area, except drainage grates must have at least 2 ounces per square foot of surface area.

Painting: Building miscellaneous metal items that are not galvanized must be cleaned and coated with 1 prime coat prior to erection under section 99-09900. After erection, surfaces must be coated with a second prime coat, and finish coats when specified, to comply with the requirements specified under section 99-09900.

Loose Bearing and Leveling Plates: Loose bearing and leveling plates must be provided for steel items bearing on masonry or concrete construction, made flat, free from warps or twists, and of required thickness and bearing area. Plates must be drilled to receive anchor bolts. Galvanize after fabrication.

Drainage Pipes, Frames and Grates:

Drain piping must have connections sealed watertight.

Drainage grates must have end bars of the same cross section as support bars. Connections between end bars and support bars of structural steel must be welded all around.

Drainage frames must be angles and plates as shown.

Drainage grates and frames must be match marked.

Steel Pipe Railings and Handrailings:

Pipe handrailing must consist of handrailing elements supported by metal brackets (wall type) or handrailing elements supported by tubular steel posts (post type).

Ends of railing pipe must be closed, except for a 1/8-inch diameter weep hole at the low point.

All corners on railings must be rounded. Simple and compound curves must be formed by bending pipe in jigs to produce uniform curvature; maintain cylindrical cross-section of pipe throughout the bend without buckling, twisting or otherwise deforming exposed surfaces of the pipe.

Wall brackets, end closures, flanges, miscellaneous fitting and anchors must be provided for interconnections of pipe and attachment of railings and handrails to other work. Inserts and other anchorage devices must be provided for connecting railings and handrails to concrete or masonry.

Steel railing must be galvanized after fabrication. After galvanizing, all elements of the railing must be free of fins, abrasions, rough or sharp edges, and other surface defects and must not be kinked, twisted, or bent.

99-05500C Construction

99-05500C(1) General

Anchorages:

Anchorage devices and fasteners must be provided for securing miscellaneous metal in-place construction; including threaded fasteners for concrete and masonry inserts, toggle bolts, through bolts, lag bolts, wood screws, and other connectors.

Cutting, drilling, and fitting must be performed as required for installation of miscellaneous metal fabrications. Work is to set accurately in location, alignment and elevation, plumb, level, true and free of rack, measured from established lines and levels.

Loose Leveling and Bearing Plates: Plates must be set on wedges or other adjustable devices. Anchor bolts must be snug tightened after the plates have been positioned and plumbed. Mortar must be packed solidly between bearing surfaces and plates to ensure that no voids remain.

Steel Pipe Railings and Handrailings:

Railings must be adjusted prior to anchoring to ensure matching alignment at abutting joints. Secure posts and railing ends to building construction.

Resin capsule anchors must not to be used for anchoring railings and handrailings.

Powder Driven Anchors: Powder driven anchors must be installed with low velocity powder actuated equipment to comply with the manufacturer's instructions and State and Federal OSHA regulations.

Resin Capsule Anchors: Resin capsule anchors must be installed in compliance with the manufacturer's instructions.

Bolted connections not otherwise specified or shown on drawings must be snug-tightened.

99-05500C(2) Damaged Surfaces

Galvanized surfaces that are abraded or damaged must be repaired by thoroughly wire brushing the damaged areas and removing all loose and cracked coating. The clean areas must then be painted with 2 spot applications of a coating complying with the requirements in the Detailed Performance Standards of the Master Painters Institute (MPI) and listed on MPI List Number 18, Primer, Zinc Rich, Organic, and meeting the requirements under section 99-09900.

99-05500D Payment

Not Used

99-6 WOOD AND PLASTICS

99-06100 ROUGH CARPENTRY

99-06100A General

99-06100A(1) Summary

Scope: This work must consist of furnishing and installing materials and performing rough carpentry work including wood framing, furring, and sheathing.

Rough carpentry includes carpentry work not specified as part of other sections and which is generally not exposed.

99-06100A(2) Definitions

Not Used

99-06100A(3) Submittals

Product Data: Manufacturer's material data and installation instructions must be submitted for gypsum sheathing, framing hardware, and underlayments.

Wood Treatment Data:

Chemical treatment manufacturer's instructions must be submitted for the handling, sorting, installation, and finishing of treated materials.

For each type of preservative treatment used, certification by treating plant must include type of preservative solution and pressure process used, net amount of preservative retained and conformance with the applicable standards of the American Wood Protection Association.

For each type of fire-retardant treatment, include certification by treating plant that the treated material complies with the applicable standards and other requirements.

99-06100A(4) Quality Control and Assurance

Not Used

99-06100A(5) Delivery, Handling, and Storage

Delivery and Storage: Materials must be kept under cover and dry. All materials must be protected from exposure to weather and contact with damp or wet surfaces with blocking and stickers. All lumber, plywood and other panels must be stacked in such a manner to provide air circulation within and around the stacks.

99-06100B Materials

99-06100B(1) Lumber

Lumber must be manufactured to comply with PS 20, "American Softwood Lumber Standard," and with applicable grading rules of inspection.

Softwood lumber must be quality grade stamped or must be accompanied by a certificate of inspection. Inspection certificates or grade stamps must indicate compliance with the grading requirements of WWPA, WCLIB, RIS, or other approved lumber inspection agencies.

All lumber used must be nominal sized and dressed S4S unless otherwise described.

Framing lumber must be solid stock lumber, Douglas Fir-Larch, and the grades indicated under WCLIB or WWPA rules. Moisture content must not exceed 19 percent and must be grade stamped "S-Dry."

99-06100B(2) Dimension Lumber

Except as otherwise shown, lumber must have the following grades.

Vertical Framing Lumber:

Vertical framing lumber, nominal 2 by 2 inch through 4 by 4 inch, must be Construction grade or better.

Vertical framing lumber, nominal 2 by 6 inch through 4 by 6 inch, must be No. 2 or better.

Horizontal Framing Lumber:

Horizontal framing lumber, nominal 2 by 4 inch and wider, including joists and rafters, must be No. 2 or better.

Horizontal framing lumber, nominal 4 by 4 inch and wider, including joist and rafters, must be No. 1 or better.

Exposed Framing Lumber: Exposed framing lumber which is not concealed and is to receive a stain or natural finish must be the same grade and species as indicated for structural framing and hand selected for appearance.

Miscellaneous Lumber:

Miscellaneous lumber for support or attachment of other work including rooftop equipment curbs and support bases, cant strips, bucks, nailers, blocking, furring, grounds, stripping and similar members must be not less than No. 2 or better.

Lumber in contact with concrete or masonry construction must be pressure treated Douglas Fir-Larch.

99-06100B(3) Timbers

Timbers (Nominal 5 inches or Thicker): Timbers must be No. 1 or better.

99-06100B(4) PlywoodPanels

Plywoodpanels must comply with Voluntary Product Standard PS 1, "Structural Plywood," or its predecessor, " Construction and Industrial Plywood."

Plywood panels must be Group 1 unless otherwise noted.

Each plywood panel must be factory marked with APA or other trademark evidencing compliance with grade requirements.

Structural Plywood Wall Sheathing: Structural plywood wall sheathing for walls must be APA RATED SHEATHING, Exposure 1. Thickness and grade must be as shown.

Structural Plywood Roof Sheathing:

Structure plywood roof sheathing must be APA RATED SHEATHING, Exposure 1. Span rating, thickness and grade must be as shown.

Structure plywood roof sheathing in exposed overhangs must be APA RATED SHEATHING, A-C, Exterior, Group 1. Thickness must be the same as the remainder of the sheathing.

Plywood Decking: Plywood decking must be APA RATED STURD-I-FLOOR, Exposure 1, with tongue-and-groove edges. Span rating and thickness must be as shown.

99-06100B(5) Miscellaneous Materials

Rough Carpentry Hardware:

Steel plates and rolled sections must be mild, weldable steel, complying with AISI grades 1016 through 1030 except 1017.

Nails, screws, bolts, nuts, washers must be commercial quality. Exposed fasteners must be hot dipped galvanized or stainless steel. Fasteners for use with preservative treated wood must be hot dip galvanized.

Joist hangers, clips and other standard framing hardware must be ICC approved, commercial quality, galvanized sheet steel or hot dipped galvanized, of the size shown.

Expansion anchors and powder driven anchors must comply with section 99-05500.

Nails: Nails must comply with ASTM F 1667. "Common" nails must comply with the following table:

Nail Size	Length (inches)	Diameter (inches)
8d	2½	0.131
10d	3	0.148
16d	3½	0.162

Building Paper: Building paper must be kraft type waterproofing building paper, Type I (No. 15) asphalt saturated roofing felt or high density, bonded polyethylene fiber building paper.

Adhesive: Adhesive for plywood glue-nailed systems must comply with APA Specification: AFG-01.

99-06100B(6) Wood Treatment By Pressure Process

Preservative Treatment:

Preservative treatment must be copper napthenate, pentachlorophenol or water-borne arsenicals (ACA, CCA or ACZA).

The following items must be treated:

Wood cants, nailers, curbs, equipment support bases, blocking, stripping and similar members in connection with roofing, flashing, vapor barriers and waterproofing.
Wood sills, sleepers, blocking, furring and other similar members in contact with concrete or masonry.
All holes, daps and cut ends of treated lumber must be thoroughly swabbed with 2 applications of copper napthenate.

Fire Retardant Treatment: Fire retardant treatment must be paintable, odorless fire retardant preservative applied by pressure treating methods.

99-06100C Construction

Wood Framing:

Wood framing must comply with Chapter 23 of the California Building Code.

Framing members must be of sizes and spacing shown. Unless otherwise shown, structural members must not be spliced between supports.

Wood framing must be accurately cut and assembled to provide closely fitted members. Framing must be erected true to the lines and grades shown and must be rigidly secured in place as shown and as required by recognized standards. Bracing must be placed wherever necessary to support all loads on the structure during erection.

The size and spacing of fasteners and the edge distance for nails must be as shown.

Nailing schedule must be as shown and must comply with the California Building Code.

Wall coverings exposed to the weather must have a backing of building paper applied weatherboard fashion to the framing or sheathing. Backing must be lapped 2 inches at horizontal joints, 6 inches at vertical joints and 12 inches at building corners.

Stair Framing:

Stair framing members must be of the size and spacing shown.

Stringers must be notched to receive treads, risers and supports. Effective depth remaining must be not less than 3-1/2 inches.

Plywood Panels:

Plywood panels must be attached to the framing as described. All structural plywood sheathing (both roof and wall) must be nailed with "Common" nails.

Plywood decking must be glued and nailed to the framing system.

Plywood sheathing must be nailed to the framing system and must be continuous over 2 or more supports. Roof and floor panels must be installed with the long dimension across the supports, with end joints staggered 4 feet. Wall sheathing must have all edges blocked. Spacing between panels must be 1/8 inch.

99-06100D Payment

Not Used

99-06180 GLUED LAMINATED MEMBERS

99-06180A General

99-06180A(1) Summary

Scope: This work consists of erecting pre-engineered, factory fabricated glued laminated members, including beams, headers and laminated decking.

99-06180A(2) Definitions

Not Used

99-06180A(3) Submittals

Product Data: Submit manufacturer's data, specifications and installation instructions for lumber, adhesives, fabrication process, preservative and fire-retardant treatment, accessories and protection.

Shop Drawings:

Submit shop drawings for glued laminated members.

Shop drawings must include erection drawings, if required, and a location plan which shows the position and identification of each glued laminated member.

Certificates of Compliance: Submit a certificate of compliance for glued laminated members.

99-06180A(4) Quality Control and Assurance

Codes and Standards:

Glued laminated members, including beams and headers, must comply with ANSI/AITC A190.1, "Structural Glued Laminated Timber."

Glued laminated decking must comply with American Lumber Standard Committee Glued Lumber Policy and Voluntary Standard PS 20.

Factory Marks:

Glued laminated structural members must be stamped with a APA EWS or similar mark which indicates that the member conforms to the requirements of ANSI/AITC A190.1.

Such marks must be placed on surfaces that will not be exposed in the completed work.

99-06180A(5) Delivery, Storage, and Handling

Protection:

Water resistant wrapping on glued laminated members must remain in place until units are erected.

Laminated members that are to be stored prior to erection must be stored on blocks well off the ground with individual member separated for air circulation. Wrapping must remain intact, lower side of wrapping must be slit or punctured to permit drainage of water which may accumulate.

99-06180B Materials

99-06180B(1) Glued Laminated Members

Lumber:

Glued laminated members must be engineered, stress rated, factory laminated structural members with adhesive for wet use. Unless otherwise shown, structural glued laminated timber members must be Combination 24F-V8 DF/DF for all cantilever beams and Combination 24F-V4 DF/DF for simple beam spans under AITC 117, "Standard Specifications for Structural Glued Laminated Timber of Softwood Species."

Exposed members must be of Architectural Grade and non-exposed members must be of Industrial Grade complying with AITC 110.

Penetrating Sealers: Penetrating sealers must be the manufacturers standard translucent penetrating sealer which will not interfere with the application of wood stain and transparent finish or paint finish as shown.

Connectors, Anchors, Accessories:

Steel plates and rolled sections must be mild, weldable steel, complying with ASTM A 36.

Nails, screws, bolts, nuts, washers must be commercial quality. Fasteners for galvanized hardware must be hot-dip galvanized.

Joist hangers, clips and other standard framing hardware must be commercial quality, galvanized sheet steel or hot dipped zinc coated, manufacturer's standard units for timber sizes indicated.

Expansion anchors and powder driven anchors must be ICC approved for the purpose intended.

99-06180B(2) Shop Fabrication

Glue laminated members must comply with ANSI/AITC A190.1 as indicated.

Members must be shop-cut for connections and connecting hardware to greatest extent feasible, including drilling of bolt holes.

Members must have location placement identification marks or symbols which correspond to the authorized location plan and must have stamps or marks which indicate the top of each member.

Camber: Unless otherwise shown, the camber must be the manufacturer's standard camber, but must not exceed a 2,000-foot radius.

Preservative Treatment: The entire surface of the members, including ends, must be sealed with a penetrating sealer immediately following manufacture.

99-06180B(3) Glued Laminated Decking

Glued laminated decking must be sized as shown.

Glued laminated decking must be Douglas Fir-Larch; Appearance Grade; smooth sanded face; edge pattern must be standard vee.

Factory Finishing: Finish must be manufacturer's standard "dry appearance" clear, penetrating acrylic stain-and-sealer, over dried and resistant to mildew and fungus.

Factory Applied Protection: Before shipping, decking units must be wrapped with manufacturer's standard, opaque durable, water-resistant, plastic coated paper covering with water resistant seams.

Small members of uniform size may be bundle wrapped. Protective slip sheets must be inserted between finished surfaces where factory finishes have been provided.

99-06180C Construction

Miscellaneous steel connectors, anchors and accessories must be installed as shown.

Members must be erected so that a close fit and neat appearance of joints and structure as a whole will not be impaired.

Padded or non-marring slings must be used when hoisting members. Corners must be protected with wood blocking.

Expansion spaces must be maintained as shown and instructed by the decking manufacturer.

Decking must be nailed as indicated or under the manufacturer's instructions for nailing wood decking.

Temporary waterproof covering must be provided to protect exposed decking if required prior to application of finish roofing.

99-06180D Payment

Not Used

99-7 THERMAL AND MOISTURE PROTECTION

99-07182 WATER REPELLENT COATING

99-07182A General

99-07182A(1) Summary

Scope: This work consists of applying water repellent coating to concrete or masonry surfaces.

The water repellent coating must be applied to all exterior concrete or masonry surfaces and exposed aggregate surfaces as shown.

99-07182A(2) Definitions

Not Used

99-07182A(3) Submittals

Product Data: Manufacturer's descriptive data, application instructions and general instructions for water repellents must be submitted.

Certificates of Compliance: Submit a certificate of compliance with each shipment of water repellent coating material.

99-07182A(4) Quality Control and Assurance

Codes and Standards: Water repellent coatings must comply with all rules and regulations concerning air pollution in the State of California.

99-07182B Materials

Water Repellent Coating: Water repellent coating must be clear, colorless, water-based sealer not containing silane, recommended for use on concrete and concrete unit masonry in vertical applications.

Compatibility: Verify compatibility between water repellent coating and anti-graffiti coating when an anti-graffiti coating is specified in section 99-9.

99-07182C Construction

Preparation: All surfaces to receive water repellent coating must be dry and cleaned by removing contaminants that block pores of the surface. Cleaning methods must be those recommended by the water repellent manufacturer.

Application:

The water repellent solution must be applied under the manufacturer's instructions

The time period between applications of water repellent coating must be not less than 24 hours.

Protection: Surfaces of other materials surrounding or near the surfaces to receive the water repellent coating must be protected from overspray or spillage from the waterproofing operation. Water repellent coating applied to surfaces not intended to be waterproofed must be removed and the surfaces restored to their original condition.

99-07182D Payment

Not Used

99-07210 INSULATION (GENERAL)

99-07210A General

99-07210A(1) Summary

Scope: This work consists of installing insulation. Insulation includes related materials such as substrate boards, underlayments, vapor retarders, and cover boards.

Insulation materials must be compatible with existing or new materials incorporated in the building.

99-07210A(2) Definitions

Not Used

99-07210A(3) Submittals

Product Data:

A list of materials, manufacturer's descriptive data, location schedule, and time schedule must be submitted.

The list of materials to be used must include the trade name, manufacturer's name, smoke developed and flame spread classification, resistance rating and thickness for the insulation materials and accessories.

Schedules:

A location schedule and time schedule must be submitted.

The location schedule must show where each material is to be installed.

The Contractor must provide the Engineer at the job site with an accurate time schedule of the areas of the building to be insulated each day. The time schedule must be submitted 3 working days in advance of the work.

Samples: Samples of insulation material must be submitted to the Engineer at the job site.

99-07210A(4) Quality Control and Assurance

Codes and Standards: All insulating materials must be certified to comply with the California Quality Standards for Insulating Materials and must be listed in the Department of Consumer Affairs publication "Consumer Guide and Directory of Certified Insulation Material."

99-07210A(5) Delivery, Storage, and Handling

Insulating materials must be delivered to the job site and stored in a safe dry location with labels intact and legible.

Insulating materials must be protected from physical damage and from becoming wet or soiled.

In the event of damage, materials must be repaired or replaced.

99-07210B Materials

Not Used

99-07210C Construction

Not Used

99-07210D Payment

Not Used

99-07212 BATT AND BLANKET INSULATION

99-07212A General

99-07212A(1) Summary

Scope: This work consists of installing batt or blanket insulation.

Batt insulation includes faced and unfaced batts in walls and ceilings, acoustical batts for sound control, and exposed batt or blanket insulation for ceilings and walls.

99-07212A(2) Definitions

Not Used

99-07212A(3) Submittals

Not Used

99-07212A(4) Quality Control and Assurance

Laminator's Qualifications:

Laminator for bonding polyethylene vapor-retarder to insulating batts must be approved by the insulation manufacturer.

The name of the laminator must be submitted with the Product Data.

Codes and Standards:

All batt or blanket insulation, including facings such as vapor barriers, must have a flame-spread rating not to exceed 25 and a smoke density not to exceed 450 when tested under UBC Standard No. 8-1.

The flame-spread and smoke density limitations do not apply to facings on batt insulation installed between ceiling joists, or in roof-ceiling or wall cavities, provided the facing is installed in substantial contact with the surface of the ceiling or wall finish.

99-07212B Materials

99-07212B(1) Insulating Materials

Fiberglass batts must be thermal insulation produced by combining glass fibers with thermosetting resins to comply with ASTM C 665.

Ceiling Insulation: Ceiling insulation must be R-19 fiberglass batts with paper-laminate vapor-retarder membrane on one face. Insulation must comply with ASTM C 665, Type II, Class C.

99-07212B(2) Vapor Retarders

Polyethylene Vapor-retarder: Polyethylene vapor-retarder must be factory-applied, 3 mils, white polyethylene film, a blend of fiberglass and polyester yarn reinforcement, and metallized polyester film laminated with a flame resistant adhesive, and a Class I flame-spread classification.

99-07212B(3) Auxiliary Insulation Materials

Insulation Tape: Insulation tape must be that recommended by the insulation manufacturer.

Insulation Adhesive: Insulation adhesive must be the type recommended by the insulation manufacturer and complying with the requirements for fire resistance and VOC content.

Impaling Pins: Impaling pins must be self-adhering wire pins with sheet metal retaining clips and protective rubber tips. Adhesive for pins must be that recommended by the pin manufacturer.

Line Wire: Line wire must be commercial quality 20-gage galvanized steel wire.

99-07212B(4) Shop Fabrication

Polyethylene must be factory laminated to fiberglass batts or blankets by an applicator approved by the manufacturer of the batts or blankets.

99-07212C Construction

The vapor retarder on faced batts must be toward the interior and must be fastened to provide a sealed retarder. Punctures and holes in the retarder must be repaired.

Unless otherwise described, insulation must be kept at minimum 3 inches clear of lighting fixtures and heat producing electrical appliances and equipment.

Installing Batt Type Insulation: Insulation batts must be installed to completely fill the space between framing members. Apply a single layer of insulation of required thickness, unless otherwise shown or required to make up total thickness. Installation must comply with the manufacturer's instructions and these special provisions.

When line wire is shown, blankets must be supported with line wire spaced at 16 inches on center.

Overlapping joints must be sealed with insulation adhesives as instructed by vapor retarder manufacturer's printed directions. Butt joints and fastener penetrations must be sealed with insulation tape of the type recommended by the vapor retarder manufacturer. Joints at pipes, conduits, electrical boxes and similar items penetrating the vapor retarder must be sealed.

99-07212D Payment

Not Used

99-07411 METAL ROOFING

99-07411A General

99-07411A(1) Summary

Scope: This work consists of installing preformed metal roofing.

Metal roofing system consists of underlayment, prefinished metal roof panels, gutters, downspouts, concealed fasteners, sealants, and other accessories and components required for a complete, securely fastened, and weathertight installation.

99-07411A(2) System Description

Design Requirements: The roofing system must comply with the wind design requirements for uplift in Chapter 16 of the CBC for the wind speed and exposure shown.

The roofing system must meet the requirements for a structural standing seam metal panel roof system under ASTM E 1514.

99-07411A(3) Definitions

Not Used

99-07411A(4) Submittals

Product Data:

Manufacturer's technical product data, installation instructions, and recommendations for each type of roofing material must be submitted for authorization.

Product data must include the manufacturer's name and a complete material description of all components of the metal roofing system.

Samples:

Material samples must include a 12 by 12 inches sample of the roofing panel for each color to be installed and a sample of each anchor clip and fastening device.

Shop Drawings:

Shop drawings showing the layout and details of the metal roofing must be submitted.

Shop drawings must show the shape, size, thickness, and method of attachment for each component used in the work; the layout and spacing of fasteners; details of connections and closures; and details for expansion joints and weathertight joints.

Design calculations for the fastening system with the substrate shown must be submitted to verify compliance with the design requirements.

Shop drawings and calculations must be sealed and signed by an engineer who is registered as a civil engineer in the State.

Certificates of Compliance: Submit a certificates of compliance for the metal roofing system.

99-07411A(5) Quality Control and Assurance

Not Used

99-07411A(6) Delivery, Storage, and Handling

Delivery and Handling: Panels must be protected against damage and discoloration.

Storage: Panels must be stored above ground, with one end elevated for drainage and protected against standing water and condensation between adjacent surfaces.

99-07411B Materials

99-07411B(1) Sheet Material

Base Metal: Base metal must be cold formed, 0.028 inch nominal (24-gage), galvanized sheet steel complying with ASTM A 653/A 653M, Grade 33 [230] with G90 [Z275] coating, except where a higher strength is required for performance, extra smooth; or cold formed aluminum-zinc alloy-coated, commercial quality, sheet steel complying with ASTM A 792/A 792M, Grade 40 [275] with AZ55 [AZM 165], coating extra smooth.

Configuration: Metal roofing system must be batten roofing system. Battens may be snap-on or integral to the roofing panel. Panel edges at batten must stand a minimum of one inch high. Battens must be a minimum of 1-3/4 inches high and spaced not less than 12 inches nor more than 18 inches on center.

Configuration: Metal roofing system must be a standing seam system with standing rib a minimum of 1-3/4 inches high and spaced not less than 12 inches nor more than 18 inches on center.

Configuration: Metal roofing system must have primary vertical ribs spaced at not less than 8 inches nor more than 12 inches, with 2 intermediate ribs located between the primary ribs. The depth of the primary ribs must be between 3/4 inch and 1 5/8 inches in depth.

99-07411B(2) Metal Finishes

Coatings must be applied before or after forming and fabricating panels, as required for maximum coating performance capability.

Colors or color matches must be as shown or, if not otherwise shown, must be as selected by the Architect from the manufacturer's standard color palette.

Fluoropolymer Coating:

Finish must be the manufacturer's standard 70 percent polyvinylidene fluoride (Kynar or Hylar) coating with a baked on primer (0.2-mil) and a finish coat of 0.8-mil nominal for a total dry film thickness of approximately 1.0-mil nominal. Coating must comply with requirements of AAMA 621.

Interior finish must consist of a 0.15-mil epoxy primer and a backer coat.

99-07411B(3) Miscellaneous Metal Shapes

Flashings, Gutters, and Downspouts: Flashings, gutters, and downspouts must be formed from the same material, gage and in the same finish as the roofing panels.

Perforated Soffit: Perforated soffit must be formed from the same material, gage and in the same finish as the roof panels.

99-07411B(4) Miscellaneous Materials

Fastener Clips: Fastener clips must be noncorrosive ferrous metal fasteners as recommended by the metal roofing system manufacturer to resist the design loads.

Fasteners: Fasteners must be as recommended by the metal roofing system manufacturer. Sheet metal screws must not be used except to fasten trim and flashings.

Underlayment: Underlayment must be as recommended by the metal roofing system manufacturer, but not less than 30-pound minimum asphalt impregnated fiber glass mat roofing felt.

Red Rosin Sheet: Red rosin sheet must be commercial quality rosin-sized sheathing paper suitable for use as a slip sheet.

Sealant and Sealant Tape: Sealant and sealant tape must be as recommended by the roofing manufacturer.

Closures: Closures must be rubber, neoprene, closed cell plastic or prefinished metal.

99-07411B(5) Shop Fabrication

Unless otherwise shown, or specified herein, roof panels must be fabricated in continuous lengths for the length of the roof, from ridge or peak to eave, except such length must not exceed the manufacturer's maximum production length. Flashings must be fabricated in the longest practical lengths.

Roofing panels must be factory formed. Field formed panels are not acceptable.

99-07411C Construction

99-07411C(1) Installation

Underlayment: The roof and fascia panels must be installed over underlayment. Underlayment must be laid parallel to the eaves, shingle fashion with 6-inch edge laps and 12-inch end laps and must be fastened as instructed by the metal roofing system manufacturer.

Roof Panels:

The roof system must be installed and fastened complying with the details shown and the authorized shop drawings. Cutting and fitting must present a neat and true appearance with exposed burrs removed. Openings through roof panels must be cut square and must be reinforced as instructed by the metal roofing system manufacturer.

Roof panels must be adjusted in place and properly aligned for the detailed conditions before fastening. Panels must not be warped, bowed or twisted. The surface finish on the panels must not be cracked, blemished or otherwise damaged.

Gaskets, joint fillers, sealants and sealing tape must be installed where indicated on the authorized drawings or as required for weatherproof performance of panel systems.

Fasteners must be concealed and must not be driven through roof panels or batten covers.

Miscellaneous Metal Shapes:

Trim, fascia, flashings, gutters, downspouts, scuppers, caps, and other prefinished metal work must be positioned to the correct alignment for each detailed condition. Metal work must be securely attached to backing using fasteners at the spacing shown on authorized shop drawings. Prefinished metal to be installed over concrete, masonry or plaster must be back-coated with asphaltic paint as instructed by the metal roofing system manufacturer.

Roof panels, trim, gutters, and other prefinished metal that are marred, punctured, incorrectly bent, or incorrectly installed will be considered damaged and must be replaced with undamaged units.

Gutters must be fabricated by the metal roofing system manufacturer to the shape and lengths shown. Expansion joints must comply with the manufacturer's instructions and to SMACNA "Architectural Sheet Metal Manual."

The metal roofing system must be installed weathertight. Closures must be tight fitting and must be provided at the ends of panels, at the boundary of the roof, and as indicated on the authorized shop drawings.

99-07411C(2) Clean Up and Close Out

Clean up:

Adjacent surfaces must be protected during the roofing system installation and sealant work. Excess sealant must be removed as the installation progresses.

Roof panels, molding, trim, and other prefinished metal surfaces must be cleaned after installation as instructed by the manufacturer. Exposed cuts must be touched-up with a matching durable primer and paint as instructed by the metal roofing system manufacturer.

Touch up: Damaged paint surfaces must be touched up by using an air dry touch up paint supplied by the metal roofing system manufacturer. Only a small brush must be used for touching up. No spraying of touch up paint is to be performed.

Damaged Units: Panels and other components of the work which have been damaged or have deteriorated beyond successful repair must be removed and replaced.

99-07411D Payment

Not Used

99-07920 SEALANTS

99-07920A General

99-07920A(1) Summary

Scope: This work consists of applying sealants which are required for this project, but not described elsewhere.

Related Work: Pourable polyurethane joint sealant for joints in concrete decks must comply with “Joint Sealant.”

99-07920A(2) Definitions

Not Used

99-07920A(3) Submittals

Product Data: Manufacturer’s descriptive data and installation instructions for all sealants must be submitted.

Samples: Color samples of all sealants must be submitted. Unless otherwise shown, colors will be selected by the Engineer from the manufacturer’s standard colors.

Compatibility and Adhesion Test Reports:

Submit evidence that materials forming joint substrates and joint sealant backings have been tested for compatibility with and adhesion to joint sealants.

Submit interpretation of test results and written recommendations for primers and substrate preparation needed for adhesion.

Certificates of Compliance: Submit a certificates of compliance for the sealants.

99-07920A(4) Quality Control and Assurance

Preconstruction Field Adhesion Testing: Before installing sealants, field test adhesion to joint substrates:

Locate test joints where indicated by Engineer.

Conduct field tests for each type of sealant and joint substrate. Test method: Hand pull method under the sealant manufacturer's instructions.

99-07920B Materials

All sealants, primers and accessories must be non-staining to adjacent exposed surfaces. Products having similar applications and usage must be of the same type and same manufacturer. Gun consistency compound must be used unless otherwise required by the job conditions.

Nonstaining: Products that have undergone testing under ASTM C 1248 or ASTM C 510 and have not stained porous substrates.

Compatibility: Provide joint sealants, backings, and related materials compatible with one another and with joint substrates under conditions of service and application as demonstrated by sealant manufacturer based on testing and field experience.

Acrylic Sealant: Acrylic sealant must be one compound, solvent release acrylic sealant.

Polyurethane Sealant: Multicomponent, nonsag, capable of 50 percent extension and contraction without failure, complying with ASTM C 920. Provide BASF, Sika, Tremco, or equal.

Butyl Sealant: Butyl sealant must be single-component, solvent-release, polyisobutylene sealant complying with ASTM C 1311.

Silicone Sealant: Silicone sealant must be one component, low modulus, non-acid curing building sealant complying with ASTM C 920 and formulated for reduced dirt pickup. Sealant must be tack-free in one hour, must not sag or flow, must be ozone resistant and capable of 100 percent extension and 50 percent contraction without failure. Provide BASF Sonneborn Sonolastic 150, Dow Corning 756 SMS Building Sealant, GE Silicones SilPruf NB SCS 9000, or equal.

Mildew Resistant Silicone Sealant: One component, sanitary type, mildew resistant, formulated with fungicide, intended for damp areas and complying with ASTM C 920. Provide Pecora 898, GE Sealants SCS 1700, Dow Corning 786, or equal.

Acoustical Sealant: Single component, latex, ASTM C 834, nondrying, nonhardening, nonsag, nonstaining, acoustically tested under ASTM E 90, paintable by acrylic or alkyd paints. Provide USG Sheetrock, Pecora AC-20, Owens Corning QuietZone, or equal.

Polysulfide Sealant: Polysulfide sealant must be a two-part, non sag polysulfide base, synthetic rubber sealant formulated from liquid polysulfide polymer.

Backer Rod: ASTM C 1330, Type C (closed-cell material with a surface skin) or Type B (consisting of both open- and closed-cell material) as recommended by manufacturer for application, of size and density to control sealant depth; polyurethane or polyethylene as recommended by sealant manufacturer. Backer rod must be sized such that it must be compressed between 25 percent and 75 percent of its uncompressed diameter during installation in the joint.

Bond Breaker Tape: Polyethylene tape or other plastic tape recommended by sealant manufacturer for preventing sealant from adhering to rigid, inflexible joint filler materials or joint surfaces at back of joint.

Primer: Material recommended by joint sealant manufacturer where required for adhesion of sealant to joint substrates indicated under anticipated service conditions, as determined from preconstruction joint sealant substrate tests and field tests.

Neoprene: Neoprene must comply with the requirements of ASTM C 542.

99-07920C Construction

Unless otherwise shown, sealants must be applied under the manufacturer’s instructions and ASTM C 1193.

When silicone sealants (or mildew-resistant silicone sealants) are used in locations where painting is required, use sealants formulated to accept paint satisfactorily and demonstrated to do so in preconstruction mockups, or sealants tinted to match adjoining painted surfaces.

Sealants must be applied in a continuous operation for the full length of the joint. Immediately following the application of the sealant, the sealant must be tooled smooth using a tool similar to that used to produce concave masonry joints. Following tooling, the sealant must remain undisturbed for not less than 48 hours.

99-07920D Payment

Not Used

99-8 DOORS AND WINDOWS

99-08100 STEEL DOORS AND FRAMES

99-08100A General

99-08100A(1) Summary

This work consists of installing steel doors and frames.

99-08100A(2) Definitions

ANSI/SDI: American National Standards Institute/Steel Door Institute.

ANSI/NAAMM-HMMA: American National Standards Institute/National Association of Architectural Metal Manufacturers-Hollow Metal Manufacturers Association.

99-08100A(3) Submittals

Product Data: Submit for all products. Include the following:

1. Material descriptions
2. Core descriptions
3. Finishes
4. Construction details

Shop Drawings: Include the following:

1. Elevations of each door design
2. Details of doors, including vertical and horizontal edge details and metal thicknesses
3. Frame details for each frame type, including dimensioned profiles and metal thicknesses
4. Locations of reinforcement and preparations for hardware
5. Details of each different wall opening condition
6. Details of anchorages, joints, field splices, and connections
7. Details of accessories

Door Schedule: Submit a schedule of steel doors and frames using same reference numbers for details and openings shown. Include a description of the type, location and size of each door and frame. Coordinate with door hardware schedule.

Certificates of Compliance: Furnish a certificate of compliance for gate and door.

99-08100A(4) Quality Control and Assurance

Single Source Responsibility: Obtain steel doors and frames from single manufacturer.

Steel Doors and Frames: Fabricate steel doors and frames under ANSI/SDI A250.8 or ANSI/NAAMM-HMMA 861.

Hardware Reinforcement: Fabricate hardware reinforcement under ANSI/SDI A250.6 with reinforcing plates from same material as door face sheets.

99-08100A(5) Delivery, Storage, and Handling

Deliver steel doors palletized, wrapped, or crated to provide protection during transit and job site storage. Do not use nonvented plastic. Furnish additional protection to prevent damage to finish.

Deliver welded frames with two removable spreader bars across bottom of frames, tack welded to jambs and mullions.

Store steel doors and frames under cover at the job site. Place in stacks of five units maximum in a vertical position with heads up, spaced by blocking, on at least 4-inch high wood blocking. Do not store in a way that traps excess humidity.

Furnish at least 1/4-inch space between each stacked door to allow air circulation.

99-08100A(6) Coordination

Coordinate installation of anchorages for steel frames. Furnish setting drawings, templates, and directions for installing anchorages, including sleeves, concrete inserts, anchor bolts, and items with integral anchors.

99-08100B Materials

99-08100B(1) General

Thickness dimensions must be minimum thickness of base metal without coatings.

Steel sheet must comply with the following:

1. Cold rolled must be commercial steel, Type B, ASTM A 1008/A 1008M
2. Hot-rolled must be commercial steel, Type B, ASTM A 1011/A 1011M; free of scale, pitting, surface defects, and pickled and oiled
3. Metallic coated must be commercial steel, Type B, ASTM A 1008/A 1008M with at least A60 metallic coating complying with ASTM A 653/A 653M
4. Stainless steel must be Type 304, ASTM A 666

Frame anchors must be commercial steel, hot dip galvanized complying with ASTM A 153/A 153M.

Inserts and fasteners must be commercial steel, hot dip galvanized complying with ASTM A 153/A 153M.

99-08100B(2) Steel Doors

99-08100B(2)(a) General

Steel doors must be at least 1-3/4 inches thick, full flush, seamless hollow metal construction unless otherwise shown. Construct doors with smooth surfaces without visible joints or seams on exposed faces, and the following:

1. Concealed stiffeners and hardware reinforcement from steel sheet, except use stainless steel to match stainless steel face sheets.
2. Furnish beveled edge, 1/8-inch in 2 inches, for single doors. Furnish round vertical edge with 2 1/8-inch radius for double doors.
3. Astragals must be full height, 1/8-inch flat bar or folded sheet strip, at least 0.053 inches thick, and same material as face sheets. Weld on the outside of the active leaf of double doors.

99-08100B(2)(b) Exterior Doors

Exterior doors must comply with ANSI/SDI A250.4, physical endurance Level A, and the following:

1. Fabricate face sheets, vertical stiffeners, and top and bottom channels from at least 0.053-inch thick metallic-coated steel sheet.
2. Fabricate the steel-stiffened core using vertical stiffeners that extend full-door height. Install stiffeners not more than 6 inches apart and spot weld to both face sheets no more than 5 inches on center. Fill spaces between stiffeners with glass-fiber insulation or mineral-fiber insulation.
3. Top and bottom channels must be continuous and spot welded to both face sheets. The top channel must be flush and the bottom channel must be inverted.
4. Include moisture vents in the bottom channel.

99-08100B(2)(c) Interior Doors

Not Used

99-08100B(3) Steel Frames

99-08100B(3)(a) General

Steel frames must comply with details shown for type and profile. Frames must be mitered corners, integral stop, and continuously welded unless otherwise shown.

Steel frames must be constructed as follows:

1. Exterior frames from metallic-coated steel sheet.
2. Borrowed-light frames from 0.053-inch-thick steel sheet.
3. Sidelight and transom frames from same thickness material as adjacent door frame.
4. Frames for openings 48 inches and less from 0.053-inch thick steel sheet.
5. Frames for openings wider than 48 inches from 0.067-inch thick steel sheet. Include at least 0.093-inch thick steel channel or angle stiffener head reinforcement.

99-08100B(3)(b) Frame Anchors

Jamb Anchors: Select one of the following methods to suit the wall type shown:

1. Masonry Type: Adjustable strap-and-stirrup or T-shaped anchors to suit frame size, at least 0.042 inch thick, with corrugated or perforated straps at least 2 inches wide by 10 inches long. For grouted frames or where shown use wire anchors at least 0.177 inch thick.

Floor Anchors: Furnish the same material as frame and at least 0.042-inch thick. Select one of the following attachment methods for the floor shown:

1. Monolithic Concrete Slab: Clip-type anchors, with two holes to receive fasteners.
2. Separate Topping Concrete Slab: Adjustable-type anchors with extension clips, allowing not less than 2-inch height adjustment. Terminate bottom of frames at finish floor surface.

99-08100B(3)(c) Stops And Moldings

Not Used

99-08100B(4) Louvers

Not Used

99-08100B(5) Accessories

Mullions and Transom Bars: Join to adjacent members by welding or rigid mechanical anchors.

Grout Guards: Form from same material as frames and at least 0.016-inch thick.

Sealants: Sealants must be ultraviolet and ozone resistant, gun grade polysulfide or polyurethane, multicomponent, complying with ASTM C 920.

Grout: Furnish grout complying with ASTM C 476, except with a maximum slump of 4 inches, as measured under ASTM C 143.

99-08100B(6) Fabrication

99-08100B(6)(a) General

Fabricate steel doors and frames to be rigid and free of defects, warp, or buckle. Accurately form metal to required sizes and profiles, with minimum radius for thickness of metal. Where practical, fit and assemble units in manufacturer's plant. To ensure proper assembly at job site, clearly identify work that cannot be permanently factory assembled before shipment.

Fabricate steel doors and frames to tolerances under SDI 117 or ANSI/NAAMM-HMMA 861.

99-08100B(6)(b) Steel Doors

Furnish overlapping astragal on one leaf of pairs of doors where required by NFPA 80 for fire-performance rating or where shown. Extend at least 3/4 inch beyond edge of door on which astragal is mounted.

Fabricate concealed stiffeners, edge channels, and hardware reinforcement from either cold or hot-rolled steel sheet.

Factory cut glazing and louver openings in doors.

99-08100B(6)(c) Steel Frames

Not Used

99-08100B(6)(d) Frame Anchors

Grout Guards: Weld guards to frame at back of hardware mortises in frames to be grouted.

Floor Anchors: Weld anchors to bottom of jambs and mullions with at least four spot welds per anchor.

Jamb Anchors: Unless otherwise shown, furnish number and spacing of anchors as follows:

1. Masonry Type: Locate anchors not more than 18 inches from top and bottom of frame. Space anchors not more than 32 inches o.c. and as follows:
   1.1. Two anchors per jamb up to 60 inches high.
   1.2. Three anchors per jamb from 60 to 90 inches high.
   1.3. Four anchors per jamb from 90 to 120 inches high.

Compression Type Anchor: Not less than two anchors in each jamb.

Postinstalled Expansion Type Anchor: Locate anchors not more than 6 inches from top and bottom of frame. Space anchors not more than 26 inches o.c.

99-08100B(6)(e) Stops and Moldings

Not Used

99-08100B(7) Shop Finishes

Apply shop primer to steel doors, frames., Use manufacturer's standard, fast-curing, lead-free and chromate-free primer complying with ANSI/SDI A250.10 acceptance criteria. Primer must be recommended by manufacturer for substrate; and compatible with field-applied coating.

99-08100C Construction

99-08100C(1) General

Examine rough-in for embedded and built-in anchors to verify actual locations before frame installation. Proceed with installation only after unsatisfactory conditions have been corrected.

99-08100C(2) Preparation

Check door frames for square, alignment, twist, and plumb before installation and adjust if necessary. Tolerances are ±1/16 inch.

Check the door frame as follows:

1. Squareness at door rabbet on a line 90 degrees from jamb perpendicular to frame head
2. Alignment at jambs on a horizontal line parallel to plane of wall
3. Twist at opposite face corners of jambs on parallel lines, and perpendicular to plane of wall
4. Plumbness at jambs on a perpendicular line from head to floor

Drill and tap doors and frames to receive nontemplated, mortised, and surface-mounted door hardware.

Doors, frames, stops, molding, louvers, and accessories must be cleaned, prepared, and painted under section 99-09900 before installation.

If grout contains an antifreezing agent, field apply a bituminous coating to the backside of frames.

99-08100C(3) Installation

99-08100C(3)(a) General

Install steel doors and frames plumb, rigid, properly aligned, and securely fastened in place. Comply with manufacturer's written instructions. Install fire rated assemblies under NFPA 80, the SFM, and the manufacturer's written instructions.

After installation, measure frames for squareness, alignment, twist, and plumbness under section 99-08100C(2). Adjust to meet tolerances as required.

Remove grout and other bonding material from exposed surfaces of steel doors and frames immediately after installation.

Install and fit fire rated assemblies under NFPA 80.

Install and fit smoke-control door assemblies under NFPA 105.

99-08100C(3)(b) Steel Frames

Set frames accurately in position, plumbed, aligned, and braced securely until permanent anchors are set. After wall construction is complete, remove spreaders and braces. Restore exposed finish by grinding, filling, and dressing, as required to make repaired area smooth, flush, and invisible on exposed faces.

Where frames are fabricated in sections because of shipping or handling limitations, field splice at accepted locations by welding face joint continuously. Grind, fill, dress, and make splices smooth, flush, and invisible on exposed faces.

Install frames with removable glazing stops located on the secure side of opening.

Install floor anchors for each jamb and mullion that extends to the floor and secure with expansion anchors.

Coordinate installation of frames to allow for solidly filling space between frame and walls with grout or mineral-fiber insulation as shown.

Solidly pack mineral-fiber insulation behind frames set in metal-stud partitions. Fill frames in masonry or concrete walls with grout. Hand trowel grout; do not pump in. Do not allow frames to be deformed or damaged by grout forces.

99-08100C(3)(c) Steel Doors

Fit steel doors accurately in frames. Shim as necessary. Clearances must be as follows:

1. Jambs and Head: 1/8 inch ±1/16 inch.
2. Between Edges of Pairs of Doors: 1/8 inch ±1/16 inch.
3. Between Bottom of Door and Top of Threshold: Maximum 3/8 inch.
4. Between Bottom of Door and Top of Finish Floor (No Threshold): Maximum 3/4 inch.

99-08100C(3)(d) Glazing

Not Used

99-08100C(4) Adjustments

Check and readjust operating hardware items immediately before final inspection. Leave work in complete and proper operating condition. Replace defective work, including steel doors and frames that are warped, bowed, or otherwise unacceptable.

99-08100C(5) Field Finish Repairs

After installation, clean field welds, bolted connections, and abraded areas of paint under SSPC-SP 2. Apply one coat of the same coating as applied for painting to the cleaned areas. Use galvanizing repair paint for metallic coated surfaces complying with manufacturer's written instructions.

99-08100D Payment

Not Used

99-08710 DOOR HARDWARE

99-08710A General

99-08710A(1) Summary

Scope: This work consists of installing mechanical door hardware for swinging doors.

99-08710A(2) Design Requirements

Hardware must be free of defects, blemishes, and excessive play. The State will provide the Solid Steel locksets for door and gate of the building. Obtain each kind of hardware from 1 manufacturer for Shrouded Hasps, steel lock box, hinges and astragals.

Furnish hardware items required to complete the work complying with performance level and design intent. Comply with the manufacturers' instructions for installation.

Furnish the manufacturer's updated item where specified item is now obsolete.

Furnish hardware with suitable fasteners to complete work.

Furnish ANSI/BHMA A156 Operational Grade 1 and Security Grade 1 for door hardware unless otherwise specified.

Maintenance Tools: Furnish a complete set of specialized tools for continued adjustment, maintenance, removal, and replacement of door hardware.

99-08710A(3) Definitions

Not Used

99-08710A(4) Submittals

Product Data: Submit for all products. Include the following:

1. Manufacturer's technical information and catalog cuts for each door hardware item. Include style, function or type, grades, size, and finish.
2. Fasteners and other pertinent information.
3. Explanation of abbreviations, symbols, and codes contained in schedules.
4. ANSI/BHMA certification.
5. Installation details for door hardware.

Shop Drawings:

Submit locations of door hardware sets, cross-referenced to drawings, both on floor plans and in door schedule. Include identification number, location, hand, fire and material of each door and frame.

Door Hardware Schedule: . Coordinate final door hardware schedule with doors, frames, and related work to ensure proper size, style, thickness, hand, function, and finish of door hardware.

Closeout Documents:

Include closeout documents in the "Maintenance and Operations Manual" before completion of the work. Submit 1 copy of PDF files on CD or DVD.

Closeout documents must include the following:

1. Index.
2. Parts list.
3. Operating instructions.
4. Maintenance instructions.

Incomplete or inadequate documentation will be returned for correction and resubmittal.

99-08710A(5) Quality Control and Assurance

99-08710A(5)(a) General

Not Used

99-08710A(5)(b) Regulatory Requirements

Door hardware and installation must comply with 24 CA Code of Regs Pt 2 and the following table:

Door hardware item	ANSI/BHMA Standard
Full mortise hinges	ANSI/BHMA A156.1
Lock cylinders, single cylinder deadbolts	ANSI/BHMA A156.5
Push plates, pull plates, kick plates, and mop plates	ANSI/BHMA A156.6
Door gasketing, automatic door bottoms, door shoes with rain drip, door sweeps, door top weatherstrips, and overhead door drips	ANSI/BHMA A156.22
Keying systems	ANSI/BHMA A156.28
Hardware preparation in steel doors and steel frames	ANSI/BHMA A156.115

99-08710A(5)(c) Certificates

Not Used

99-08710B Materials

99-08710B(1) General

Furnish door hardware sets for each door as described. Provide steel lock box and locking pin as shown

The State will provide the lockset manufactured by Pacific Lock Model 400 XIC with State No. 2 key.

Plans show direction of swing or hand of each door leaf. Furnish each item of hardware for proper door movement.

99-08710B(2) Hinges

Hinges must be full mortise, five knuckle, ball bearing construction and comply with the following:

1. Heavy Weight Hinges:
   1.1. Exterior: As shown.

99-08710B(3) Mechanical Locks and Latches

99-08710B(3)(a) General

Lock Backset: 2-3/4 inches, unless otherwise described.

Strike: Manufacturer's steel box as shown.

99-08710B(3)(b) Cylindrical Locksets

The lockset is provided by the State and installed by the Contractor.

99-08710B(3)(c) Mortise Locksets

Not Used

99-08710B(3)(d) Auxiliary Locks

Not Used

99-08710B(3)(e) Lock Cylinders

Keys must be nickel silver and same manufacturer as locking devices. Furnish 2 change keys per temporary core. Furnish 2 blank keys per permanent core. Stamp change key bows and blank key bows "State of California" and "Do Not Duplicate."

99-08710B(4) Electric Strikes

Not Used

99-08710B(5) Electromechanical Locks

Not Used

99-08710B(6) Flush Bolts

Not Used

99-08710B(7) Accessories For Pairs Of Doors

Coordinators: Type 21A, flush bolts, manufacturer's coordinated channel or filler, active-leaf, hold-open lever and inactive-leaf release trigger; fabricated from steel with nylon-coated strike plates. Include built-in, adjustable safety release.

99-08710B(8) Surface Closer

Door Closers: Surface mounted, aluminum cover, non handed, field adjustable sizes 1 through 6, parallel arm set with hold open and stop. Include separate adjusting valves for closing, latching speed, and backcheck. Use drop brackets at narrow head rails.

99-08710B(9) Exit Devices

Not Used

99-08710B(10) Operating Trim

Push Plates and Pull Plates: Beveled edges, stainless steel, and size 16 by 4 inches. Push plate must be Type J301. Pull plate must be Type J405, with one-inch diameter round pull and 1 1/2-inch standoffs on 8-inch centers.

99-08710B(11) Protective Trim Unit

Kickplates and Mop Plates: Beveled edges, stainless steel, countersunk screw holes, width 2 inches less than door width for single doors, and 1-inch less than door width each for door pairs. Kickplate must be Type J102, 12 inches tall. Mop plate must be Type J103, 6 inches tall.

99-08710B(12) Mechanical Stops and Holders

Floor Stops: Dome type, Type L12141 or L12161 as required, countersunk screw holes, non marring rubber bumper, and height for threshold or non threshold door frame as required.

Wall Stops and Door Mounted Stops: Wall type, 3 1/2-inch projection, Type L12011 or L12021 as required, countersunk screw holes, and non marring rubber tip.

Wall Bumpers: Wall type bumper, Type L22101 or L22201 as required, no visible screw holes, and convex rubber pad.

99-08710B(13) Door Gasketing

Automatic Door Bottoms: Heavy duty, full mortise, mill finished aluminum with silicone insert, end covers, and strike plates.

Door Shoe with Rain Drip: Mill-finished aluminum with neoprene insert, end covers, and formed rain drip.

Door Sweep: Mill-finished aluminum and neoprene.

Overhead Door Drip: Mill-finished aluminum 2-1/2 inches wide.

Door Gasketing: Bumper-type resilient inserts with retainer strips and surface applied. Perimeter seals must meet performance tests for heat, cold, air leakage, and smoke. At astragals, furnish a compression bulb resilient pressure sensitive door gasketing. Materials must be NRTL listed where used with labeled assemblies.

99-08710B(14) Thresholds

Thresholds must be factory non-slip mill-finished aluminum, nominal 6 inches wide unless otherwise shown, and full width of opening described.

Threshold bedding sealant must be weatherproof silicone sealant and adhesive.

99-08710B(15) Shop Fabrication

Manufacturer's Nameplate: Do not use products that have manufacturer's name or trade name displayed in a visible location except with required fire-rated labeling. Manufacturer's identification will be permitted on lock cylinder rims.

Base Metals: Furnish door hardware items of base metal specified, fabricated by forming method indicated, using manufacturer's standard metal alloy, composition, temper, and hardness. Furnish metals of a quality equal to or greater than that of specified door hardware items. Do not use a manufacturer's standard materials or forming methods if different from the specified standard.

Fasteners: Screws must comply with commercially recognized industry standards for application intended. Furnish Phillips oval-head screws finished to match surface of door hardware. Furnish fire-rated fasteners for labeled assemblies for the following:

1. Hinges mortised to wood doors or frames.
2. Strike plates to wood frames.
3. Closers to wood doors and frames.
4. Surface hinges to steel doors.
5. Closers to steel doors and frames.
6. Surface-mounted exit devices to steel doors and frames.
7. Spacers or sex bolts for through bolting of hollow-metal doors.

Do not use aluminum fasteners. Furnish noncorrosive fasteners for exterior door gasketing applications.

99-08710B(16) Finishes

Exterior Hardware: Standard Stainless Steel Finish 630 (US 32D), satin stainless steel. Where shown, use Standard Finish 626 (US 26D), satin chromium.

Factory Covering: Apply a strippable, temporary protective covering to exposed finishes before shipping.

99-08710C Construction

99-08710C(1) General

Doors and Frames: Doors and frames must be set square, plumb, and properly prepared before hardware installation.

99-08710C(2) Examination

Doors and Frames: Examine doors and frames for compliance with requirements for installation tolerances, labeled fire-rated door assembly construction, wall and floor construction, and other conditions affecting door hardware installation.

99-08710C(3) Installation

Furnish heavy weight hinges for exterior doors.

Hardware items must be accurately fit, securely applied, adjusted, and lubricated to comply with the manufacturer's instructions. Hardware items must operate without binding or excessive play.

Hinges must be installed at equal spacing with the end hinges not more than 9 5/8 inches from the top and bottom of the door. Kickplates and mop plates must be mounted on the push side of the doors, 1 inch up from bottom edge. Locksets on doors must be installed approximately 3 feet and 6 feet from the bottom of the door.

Thresholds must be set in a continuous bed of bedding sealant.

Mechanical stops on concrete surfaces must be attached with expansion anchoring devices. Mechanical stops mounted elsewhere must be attached with wood screws. Do not locate stops in the path of travel.

Hardware, except hinges, must be removed from surfaces to be painted before painting. Do not install surface-mounted items until finishes have been completed on substrates involved. Painting must comply with section 99-09900.

Furnish all dogging keys, closer valve keys, lock spanner wrenches, other factory furnished installation aids, instructions, and maintenance guides to the Engineer.

Install continuous weatherstripping at each edge of exterior door leaf. Seal finish must match adjacent frame color.

99-08710C(4) Lock Cylinders

Install temporary cores in all lockable doors during construction.

Furnish permanent cores and keys to the Engineer before Contract acceptance. The Department will install permanent cores.

99-08710C(5) Cleaning and Protection

Clean adjacent surfaces soiled by door hardware installation.

Clean hardware items as necessary to restore proper function and finish.

Furnish final protection and maintain conditions that ensure that door hardware is without damage or deterioration before Contract acceptance.

99-08710C(6) Adjusting

Initial Adjustment: Adjust and check each operating item of door hardware and each door to ensure proper operation or function. Replace units that cannot be adjusted to operate as intended.

99-08710C(7) Door Hardware Schedule

Not Used

99-08710D Payment

Not Used

99-9 FINISHES

99-09250 GYPSUM WALLBOARD

99-09250A General

99-09250A(1) Summary

Scope: This work consists of installing and finishing gypsum wallboard.

Where assembly fire ratings are shown, construction must provide the fire resistance under the applicable standards in the *Fire Resistance Design Manual* published by the Gypsum Association.

99-09250A(2) Definitions

Not Used

99-09250A(3) Submittals

Product Data: Submit manufacturer's descriptive data and installation instructions.

99-09250A(4) Quality Control and Assurance

Not Used

99-09250B Materials

99-09250B(1) General

Gypsum wallboard must be mold-, mildew-, and moisture-resistant. Use mold- and moisture-resistant joint tape and compound. You may use cementitious backer board.

Use mold-, moisture-, and water-resistant gypsum board as backing boards for (1) tile, (2) rigid sheet wall covering, and (3) wainscoting. You may use cementitious backer board.

Use mold- and moisture-resistant joint tape and finishing compound with mold-, moisture-, and water-resistant gypsum board.

99-09250B(2) Delivery and Storage

Materials must be delivered in original packages, containers or bundles bearing brand name, applicable standard of manufacture, and name of manufacturer or supplier and must be kept dry and fully protected from weather and direct sunlight exposure. Gypsum wallboard must be stacked flat with adequate support to prevent sagging or damage to edges, ends and surfaces.

99-09250B(3) Gypsum Wallboard

Gypsum Wallboard: Gypsum wallboard must comply with ASTM C 1396. Gypsum board must be Type X with tapered edges.

Mold-, Mildew-, and Moisture-Resistant Gypsum Board: Mold-, moisture-, and water-resistant resistant gypsum board must achieve a mold resistance rating of 10 under ASTM D 3273 and evaluated under ASTM D 3274. Furnish one of the following types:

1. Glass mat water-resistant gypsum panel with glass mat facings and water-resistant fiber-reinforced gypsum core, and complying with ASTM C 1658/C 1658M. Glass mat water-resistant gypsum panel must be Georgia-Pacific DensArmor Plus Fireguard Paperless Interior Drywall, or equal.
2. Fiber-reinforced water-resistant gypsum panel, unfaced with water-resistant core, and complying with ASTM C 1278/C 1278M. Fiber-reinforced water-resistant gypsum panel must be US Gypsum Fiberock Aqua-Tough Interior Gypsum Panel, or equal.
3. Gypsum panel with paper faces treated with an antimicrobial agent and containing core additives to add resistance to mold, mildew, and moisture and complying with ASTM C 1396/C 1396M. Gypsum panel must be National Gypsum Gold Bond XP Fire Shield Gypsum Wallboard, or equal.

Joint Tape and Joint and Finishing Compound: Joint tape and joint and finishing compound must comply with ASTM C 475.

Mold and Moisture Resistant Joint Tape and Finishing Compound: Mold and moisture resistant joint tape and finishing compound must comply with ASTM C 475. Joint tape must be glass mesh or as recommended by gypsum board manufacturer. Joint compound must be setting-type joint or as recommended by gypsum board manufacturer. Compound must achieve a mold resistance rating of 10 under ASTM D 3273 and evaluated under ASTM D 3274.

Corner Beads, Metal Trim and Control Joints: Corner beads, metal trim and control joints must be galvanized steel of standard manufacture.

Resilient Metal Channel: Resilient metal channel must be galvanized sheet steel channels of standard manufacture designed to reduce sound transmission through wood frame partitions.

Fasteners: Fasteners must be gypsum wallboard nails complying with ASTM C 514 or steel drill screws complying with ASTM C 1002.

Cementitious Backer Board: Cementitious backer board must be non-asbestos fiber-mat reinforced cementitious backer board complying with ASTM C 1325.

99-09250C Construction

Install wallboard panels on ceilings and soffits with the long dimension of the panels perpendicular to the framing members. Install wallboard panels on walls with the long dimension of the panels either parallel or perpendicular to the framing members. The direction of the panels must be the same on any 1 wall or partition assembly.

Edges of wallboard panels must be butted loosely together. All cut edges and ends must be smoothed as needed for neat fitting joints.

All edges and ends of gypsum wallboard panels must coincide with the framing members, except those edges and ends which are perpendicular to the framing members. End joints on ceilings and on the opposite side of partition assemblies must be staggered.

Gypsum wallboard panels for shear wall sheathing or fire resistive assemblies must be fastened to all framing members. Gypsum wallboard panels at other locations and gypsum wallboard finish over plywood sheathed shear walls must be fastened to all framing members except at the following locations:

At internal angles formed by ceiling and walls, first install ceiling panels with the fasteners terminating at a row 7 inches from the walls, except for walls parallel to ceiling framing. Wall panels must butt the ceiling panels. The top row of wall panel fasteners must terminate 8 inches from the ceiling.

At internal vertical angles formed by the walls, fasteners must not be installed along the edge or end of the panel that is installed first. Fasteners must be installed only along the edge or end of the panel that butts and overlaps the panel installed first.

Adhesives must not be used for securing wallboard to framing.

Except where closer spacing is shown, spacing of fasteners must not exceed (1) 7 inches for nails, (2) 12 inches for screws, and (3) 8 inches for screws at the perimeter of panels for fire resistive assemblies having metal framing.

Use Type S steel drill screws to fasten wallboard to metal framing. Use nails or Type W steel drill screws to fasten wallboard to wood framing. Screws must not be used in fire resistive assemblies unless otherwise shown.

Fasteners must be located at least 3/8 inch from wallboard panel edges and ends. Nails must penetrate into wood framing at least 1-1/8 inches. Screws must penetrate into wood framing at least 5/8 inch. All metal fasteners must be driven slightly below surface level without breaking the paper or fracturing the core.

Metal trim must be installed at all free edges of panels, where wallboard panels abut dissimilar materials and at locations shown. Corner beads must be installed at external corners. Control joints must be installed at the locations shown.

Joints in mold-, moisture-, and water-resistant resistant gypsum board must not be taped or filled and dimples at the fastener heads must not be patched. Edges of cuts and holes in backing board must be sealed with a primer or sealer that is compatible with the wall covering or wainscoting adhesive to be used.

All other joints must be filled and finished with joint tape and at least 3 coats of joint compound (1) between face panels, (2) the internal angles formed by ceiling and walls and (3) the internal vertical angles formed by walls. Tape in the corners must be folded to comply with the angle of the corner. Tape at joints and corners must be embedded in joint compound.

Dimples at nail and screw heads, dents, and voids or surface irregularities must be patched with joint compound. Each patch must consist of at least 3 coats and each coat must be applied in a different direction.

Flanges of corner beads, control joints and trim must be finished with a least 3 coats of joint compound.

Each coat of joint compound must be feathered out onto the panel surface and must be dry and lightly sanded before applying the next coat. The finished surfaces of joint compound at the panel joints, internal angles, patches and at the flanges of trim, corner beads and control joints must be flat and true to the plane of the surrounding surfaces and must be lightly sanded.

Good lighting of the work area must be provided during the final application and sanding of the joint compound.

Surfaces of wallboard to be textured must receive an orange peel texture, unless otherwise shown.

99-09250D Payment

Not Used

99-09900 PAINTING

99-09900A General

99-09900A(1) Summary

Scope: This work consists of preparing surfaces to receive coatings and applying coatings.

The coatings specified in this section are in addition to any factory finishes, shop priming, or surface treatment described.

99-09900A(2) Definitions

Detergent Wash: Removal of dirt and water-soluble chemicals by scrubbing with a solution of detergent and water, and removal of all solution and residues with clean water.

Hand Cleaning: Removal of dirt, loose rust, mill scale, excess base material, filler, aluminum oxide, chalking paint, peeling paint, or paint that is not firmly bonded to the surfaces by using hand or powered wire brushes, hand scraping tools, power grinders, or sandpaper and removal of all loose particles and dust prior to coating.

Mildew Wash: Removal of mildew by scrubbing with a solution of detergent, hypochlorite-type household bleach, and warm water, and removal of all solution and residues with clean water.

Abrasive Blasting:

Removal of loosely adhering paint, dirt, rust, mill scale, efflorescence, weak concrete, or laitance, must be by the use of airborne abrasives. Loose particles, dust, and abrasives must be removed by blasting with clean, oil-free air.

Abrasives must be limited to mineral grit, steel grit, or steel shot, and must be graded to produce the surface profile recommended in the manufacturer's data sheet.

Steam Cleaning: Removal of oil, grease, dirt, or other foreign matter by using steam generated by commercial steam cleaning equipment, from a solution of water and steam cleaning compounds, and removal of all residues and cleaning compounds with clean water.

TSP Wash: Removal of oil, grease, dirt, paint gloss, and other foreign matter by scrubbing with a solution of trisodium phosphate and warm water, and removal of all solution and residues with clean water.

Water Blasting: Removal of dirt, loose scale, chalking, or peeling paint by low-pressure water cleaning. Water blasting must be performed under SSPC-SP12 and must produce a surface cleanliness meeting SSPC-SP12-WJ4. Equipment used must have a minimum flow rate of 1.5 gpm. If a detergent solution is used, it must be biodegradable and must be removed from all surfaces with clean water.

99-09900A(3) Submittals

Product Data:

Manufacturer's descriptive data, a materials list, and color samples must be submitted.

Product descriptive data must include product description, manufacturer's instructions for product mixing, thinning, tinting, handling, site environmental requirements, product application, and drying time.

Materials list must include manufacturer's name, trade name, and product numbers for each type coating to be applied.

Samples: Submit color samples. Samples must be manufacturer's color cards, nominally 2 by 3 inches for each color of coating shown.

Certificates of Compliance: Submit certificates of compliance for products required to comply with SSPC standards.

99-09900A(4) Quality Control and Assurance

Regulatory Requirements: Coatings and applications must comply with the rules for control of VOC emissions adopted by the Air Quality Management District in the air basin in which the coatings are applied.

99-09900A(5) Site Environmental Requirements

Coatings must be applied under the environmental constraints specified in the manufacturer's instructions. These conditions must be maintained until the coating has cured and is ready for recoat.

Continuous ventilation must be provided during application of the coatings.

Adequate lighting must be provided while surfaces are being prepared for coatings and during coating applications.

99-09900A(6) Maintenance Stock

Upon completion of coating work, deliver a full one-gallon container of each type and color of finish coat to the Engineer. Containers must be tightly sealed, have the manufacturer's standard product label, and be labeled with color, texture, and room locations where used.

99-09900B Materials

99-09900B(1) General

Products for each coating system must be from a single manufacturer and must comply with the Detailed Performance Standards of the Master Painters Institute (MPI). Each product must be shown on the MPI Approved Products List unless otherwise specified.

99-09900B(2) Delivery, Storage, and Handling

Products must be delivered to the site in sealed, labeled containers and stored in a well-ventilated area at an ambient air temperature of at least 45 degrees F. Container labeling must include manufacturer's name, type of coating, trade name, color designation, drying time, and instructions for tinting, mixing, and thinning.

99-09900C Construction

99-09900C(1) Inspection

Coatings must not be applied until surface preparation has been authorized by the Engineer. Notify the Engineer at least 3 business days before application of coatings.

99-09900C(2) Surface Preparation

Prepare surfaces for coating under the coating manufacturer's instructions unless otherwise specified.

Remove hardware, cover plates, light fixture trim, and similar items before preparing surfaces for coating. Following the application of the finish coating, the removed items must be reset in their original locations.

Wood:

Lightly sand exterior surfaces no more than 24 hours before applying coatings.

Apply a sealer under the coating manufacturer's instructions to knots, sap, pitch, tar, creosote, and other bleeding substances.

After applying the prime coat, all nail holes, cracks, open joints, dents, scars, and surface irregularities must be filled, hand cleaned, and spot primed to provide smooth surfaces before applying finish coats.

Irregularities in wood surfaces to receive a clear finish must be filled and hand cleaned before applying coatings. The color of the filler must match the color of the coated wood.

Galvanized Metal:

New surfaces must be roughened by hand sanding or light abrasive blasting. Galvanizing must not be removed during cleaning or roughening.

Damaged or corroded areas must be cleaned and given 2 spot applications of a coating that complies with the Detailed Performance Standards of the MPI, and listed on MPI List "Number 18, Primer, Zinc Rich, Organic."

Steel and Other Ferrous Metals: Surface must be cleaned under SSPC-SP 1. Surface profile must be as required for the coating system specified.

Cement Plaster: New plaster must be cured at least 14 days before coating. Cracks, holes, and surface imperfections must be filled with patching plaster and hand textured to match adjacent surfaces.

Concrete and Concrete Masonry Unit: New material must be cleaned and prepared under SSPC-SP 13. Cracks and voids must be filled with cement mortar patching material. Concrete must be cured until the surface moisture is below the level specified in the coating manufacturer's instructions.

Previously Coated Surfaces:

Dirt, oil, grease, or other surface contaminants must be removed by water blasting, steam cleaning, or TSP wash. Minor surface imperfections must be filled as specified for new work. Mildew must be removed by mildew wash. Chalking paint must be removed by hand cleaning. The surfaces of existing hard or glossy coatings must be abraded to dull the finish by hand cleaning or light abrasive blasting. Abrasive blasting must not be used on wood or non-ferrous metal surfaces.

Chipped, peeling, blistered, or loose coatings must be removed by hand cleaning, water blasting, or abrasive blasting. Bare areas must be pretreated and primed as specified for new work.

99-09900C(3) Application

Coatings must be applied under the manufacturer's instructions and at the application rates recommended by the manufacturer to achieve the dry film thickness stated in the coating technical data sheet.

Mixing, thinning and tinting must comply with the manufacturer's instructions. After thinning, the coating must comply with the regulatory requirements.

Coatings must be applied only when surfaces are dry and properly prepared.

Cleaning and painting must be scheduled so that dust and other contaminants from the cleaning process do not fall on wet, newly coated surfaces.

Materials required to be coated must have coatings applied to all exposed surfaces, including the tops and bottoms of wood and metal doors, and other surfaces not normally visible from eye level.

Surface Finish Application:

Each coat must be applied to a uniform finish. Finished surfaces must be free of surface deviations and imperfections such as skips, cloudiness, spotting, holidays, laps, brush marks, runs, sags, curtains, ropiness, improper cutting in, overspray, drips, ridges, waves, and variations in color and texture.

Each application of a multiple application finish system must closely resemble the final color coat, except each application must provide enough contrast in shade to distinguish the separate applications.

Work Required Between Applications:

Each application of material must be cured under the coating manufacturer's instructions before applying the next coating.

Enamels and clear finishes must be lightly sanded, dusted, and wiped clean between applications.

Timing of Applications: The first application of the coating system must be during the same work shift that the final surface preparation was performed. Additional coats must be applied as soon as the required drying time of the preceding coat, specified in the coating manufacturer's instructions, has been met.

Application Methods:

Coatings must be applied by brush, roller or spray. Rollers must not leave a stippled texture in the paint film. Extension handles for rollers must not be greater than 6 feet in length.

If spray methods are used, surface deviations and imperfections such as overspray, thickness deviations, lap marks, and orange peel must be considered as evidence the work is unsatisfactory and the Contractor must apply the remainder of the coating by brush or roller, as authorized by the Engineer.

Back Priming: The first application of the coating system must be applied to all wood surfaces (face, back, edges, and ends) of wood materials that are not factory coated, immediately upon delivery to the job site. Surfaces of interior finish woodwork that adjoin concrete or masonry must be coated with one application of exterior wood primer before installation.

Patches in Previously Coated Surfaces: Where patches are made on surfaces of previously coated walls or ceilings, the entire surface to corners on every side of the patch must be coated with at least 1 application of the finish coat.

Finishing Mechanical and Electrical Components:

Shop primed mechanical and electrical components must be finish coated under the coating system specified for the substrate material. Louvers, grilles, covers, and access panels on mechanical and electrical components must be removed and coated separately.

Interior surfaces of air ducts which are visible through grilles or louvers must be coated with one application of flat black enamel, to the limit of the sight line.

Conduit, piping, and other mechanical and electrical components visible in the finished work must be painted.

Both sides and all surfaces, including edges and back of wood mounting panels for electrical and telephone equipment must be finish coated before installing equipment.

99-09900C(4) Cleaning

Upon completion of all operations, the coated surfaces must be thoroughly cleaned of dust, dirt, grease, or other unsightly materials or substances.

Surfaces marred or damaged as a result of your operations must be repaired, to match the condition of the surfaces before the beginning of your operations.

99-09900C(5) Protection

Provide protective devices, such as tarps, screens or covers, as necessary to prevent damage to the work and to other property or persons from all cleaning and painting operations.

Paint or paint stains on surfaces not designated to be painted must be removed at your expense and the original surface must be restored.

99-09900C(6) Coating System

The surfaces to be coated must be as described. When a coating system is not described for a surface to be finish coated, use the coating system as specified below for the substrate material. The number of applications specified for each coating system specified is a minimum. Additional coats must be applied if necessary to obtain a uniform color, texture, appearance, or required dry film thickness.

SYSTEM 1 - CEMENT PLASTER AND CONCRETE

2 Finish Coats:

Flat: Latex, Exterior, MPI Gloss Level 1, MPI List Number 10
Semi-Gloss: Latex, Exterior, MPI Gloss Level 5, MPI List Number 11

SYSTEM 2- CONCRETE MASONRY UNIT

One Prime Coat:

Block Filler: Latex, Interior/Exterior MPI List Number 4

2 Finish Coats:

Flat: Latex, Exterior, MPI Gloss Level 1, MPI List Number 10
Semi-Gloss: Latex, Exterior, MPI Gloss Level 5, MPI List Number 11

SYSTEM 3 - GALVANIZED METAL

2 Finish Coats:

Flat: Latex, Exterior, MPI Gloss Level 1, MPI List Number 10
Eggshell-like: Light Industrial coating, Water Based, Exterior, MPI Gloss Level 3, MPI List Number 161
Semi-Gloss: Light Industrial coating, Water Based, Exterior, MPI Gloss Level 5, MPI List Number 163
Gloss: Light Industrial coating, Water Based, Exterior, MPI Gloss Level 6, MPI List Number 164

SYSTEM 4 - GYPSUM BOARD

One Prime Coat:

Primer Sealer: Latex, Interior, MPI List Number 50

2 Finish Coats:

Flat: Latex, Interior, MPI Gloss Level 1, MPI List Number 53
Velvet-like: Latex, Interior, MPI Gloss Level 2, MPI List Number 44
Semi-Gloss: Latex, Interior, MPI Gloss Level 5, MPI List Number 54
Gloss: Latex, Interior, MPI Gloss Level 6, MPI List Number 114

SYSTEM 5 - STEEL AND OTHER FERROUS METALS, NON-CORROSIVE ENVIRONMENT

VISIBLE IN FINISHED WORK:

2 Prime Coats:
Shop Primer: Coating meeting the requirements of SSPC-Paint 15
Field Primer: Rust Inhibitive, Water Based, MPI List Number 107
2 Finish Coats:
Flat: Latex, Exterior, MPI Gloss Level 1, MPI List Number 10
Eggshell-like: Light Industrial coating, Water Based, Exterior, MPI Gloss Level 3, MPI List Number 161
Semi-Gloss: Light Industrial coating, Water Based, Exterior, MPI Gloss Level 5, MPI List Number 163
Gloss: Light Industrial coating, Water Based, Exterior, MPI Gloss Level 6, MPI List Number 164

NOT VISIBLE IN FINISHED WORK:

2 Prime Coats:
Shop Primer: Coating meeting the requirements of SSPC-Paint 15
Field Primer: Rust Inhibitive, Water Based, MPI List Number 107

SYSTEM 6 - WOOD, PAINTED

1 Prime Coat:

Primer: Latex for Exterior Wood, MPI List Number 6

2 Finish Coats:

Flat: Latex, Exterior, MPI Gloss Level 1, MPI List Number 10
Low Sheen: Latex, Exterior, MPI Gloss Level 3/4, MPI List Number 15
Semi-Gloss: Latex, Exterior, MPI Gloss Level 5, MPI List Number 11
Gloss: Latex, Exterior, MPI Gloss Level 6, MPI List Number 119

99-09900C(6) Color Schedule

Colors must be as shown.

99-09900D Payment

Not Used

99-10 SPECIALTIES

Not Used

99-11 EQUIPMENT

Not Used

99-12 FURNISHINGS

Not Used

99-13 SPECIAL CONSTRUCTION

Not Used

99-14 CONVEYING SYSTEMS

Not Used

99-15 MECHANICAL

Not Used

99-16 ELECTRICAL

99-16010 TUNNELS AND CONTROL BUILDING ELECTRICAL WORK

99-16010A General

99-16010A(1) Summary

Scope: This work consists of performing electrical work for procuring power from City of Pasadena for modifying three tunnel's lighting system and installing tunnel controls equipment and main service switchboard equipment at the tunnels control building including furnishing all labor, materials, equipment and services required to construct, connect and install the complete electrical system.

Disposal of existing electrical equipment that contains hazardous materials must comply with section 14-11.10.

99-16010A(2) System Description

System layouts are generally diagrammatic and location of equipment is approximate. Exact routing of conduits and other facilities and location of equipment is to be governed by structural conditions and other obstructions, and must be coordinated with the work of other trades. Equipment requiring maintenance and inspection must be located where it is readily accessible for the performance of such maintenance and inspection.

99-16010A(3) Definitions

Not Used

99-16010A(4) Order of Work

The first order of work must be to obtain authorization of the shop submittals and order materials for the electrical work and submit request for obtaining electrical service to the City of Pasadena Water and Power. The materials must be ordered as soon as the submittals are authorized. You must furnish the Engineer with a statement from the vendor that the order for the electrical materials required for this contract has been received and accepted by the vendor and the statement must be furnished within seven days after the submittals are authorized. The statement must give the date that the electrical materials will be shipped. If you have all the necessary materials on hand, then you will not be required to furnish the vendor's statement.

The second order of work must be to coordinate with the construction of the Tunnels Control Building and install all underground conduit/conductors system and complete the installation work required for the installation of the main service switchboard, tunnels control equipment, and miscellaneous building related electrical work The second order of work must also include energizing all electric main service switchboard and tunnels control equipment.

The third order of work must be to complete the tunnel lighting modification work as described. You must not work at more than one tunnel at a time. Prior to starting lighting modification work on another tunnel, you must complete all installation work as shown inclusive of completing all punch list items work, commissioning of the tunnel and site acceptance testing. The tunnel selection to work on must be made as per the recommendations of the Engineer.

99-16010A(5) Submittals

Equipment Lists Submittal:

A list of all electrical materials and equipment to be installed, and manufacturer's descriptive data must be submitted for authorization prior to ordering of the equipment.

Manufacturer's descriptive data must include the following:

1. Complete description, tunnel light fixtures performance data and installation instructions for the materials and equipment
2. Control and wiring diagrams, rough-in dimensions, and component layout. All control and power wiring on shop drawings must be identified with wire numbers. All shop drawings must be 22 by 34 inches in dimensions or other dimensions.

Work Plan Submittal:

Submit a detailed work plan, Work plan must include;

1. A description of procedures, methods and schedules for procuring electrical service from the City of Pasadena, tunnels temporary lighting system design and placement inside each tunnel, removal of existing tunnels lighting system and installation of new tunnels lighting system, installation of miscellaneous electrical equipment such as conduits, conductors, pull boxes, installation of main service switchboard and tunnels control equipment inside the tunnels control building, and modifying existing electrical service equipment (total of three services) as shown.

Operation and Maintenance Manuals/Closeout Submittals:

Submit 3 copies of the operation and maintenance instructions with parts lists for the equipment. Submit one PDF of the complete operations and maintenance manuals.

The instructions and parts lists must be in a bound manual form and must be complete and adequate for the equipment installed. Inadequate or incomplete material will be returned for correction. Resubmit adequate and complete manuals.

Submit operation and maintenance manuals for the following equipment:

1. Tunnel light fixtures
2. Hot standby programmable logic controller
3. Light sensors/photo-diodes and photoelectric unit

4. Lighting contactors
5. Circuit breakers
6. Transformer
7. Batteries and battery chargers
8. Light dimming control modules

Training Details Submittal:

Fifteen days prior to training, submit training class details for each training session, including:

1. Course content
2. Equipment and materials available during the training

Manufacturer's Warranties:

Submit manufacturer's warranties for equipment and materials prior to contract acceptance.

99-16010A(6) Quality Control and Assurance

Regulatory Requirements: All electrical work performed and materials installed must comply with section 86-1.02, latest edition of the California Electrical Code, latest edition of the IESNA Recommended Practice for Tunnel Lighting (IESNA RP-22) and the CA Code of Regs, Title 24, Part 6, "California Energy Code."

99-16010B Materials

Not Used

99-16010C Construction

99-16010C(1) General

Not Used

99-16010C(2) Testing

After the installation work for the various systems has been completed, each electrical system must be tested in the presence of the Engineer to demonstrate that the electrical systems function properly. You must make necessary repairs, replacements, adjustments and retests at your expense.

Final inspection for the completed electrical system will take place after all the various systems have been tested.

The Engineer must be notified 15 days in advance of testing and State personnel training on the job site.

Factory Acceptance Test (FAT):

A factory acceptance test must be conducted for the tunnels control equipment inclusive of the hot standby programmable logic controller and proving compatibility of the hot standby controller with the existing Tesco's Site Glass Enterprise SCADA central monitoring system in Sacramento by making physical connection over the web, and other instruments prior to the delivery of the equipment to the site.

The testing must demonstrate the compliance of the equipment with authorized shop drawings. Testing must be conducted at equipment supplier's shop in the presence of Engineer. Notify the Engineer at least 5 days in advance prior to testing date.

Equipment must not be shipped unless you have received written authorization from the Engineer.

You must make necessary repairs, replacements, and adjustments to the system per the instruction of the engineer.

Site Acceptance Test (SAT):

The site acceptance test (SAT) must demonstrates functional test of the supplied system. You are responsible for the compatibility and adjustment of all systems and components necessary for the successful operation of the complete electrical system.

SAT for the tunnels operation must consist of at least 7 days of trouble free operation. If any systems fail to perform satisfactory during the tests, the tests must be repeated after the deficiency is corrected.

Notify the Engineer at least 5 days prior to testing.

99-16010C(3) Training

Onsite Training:

Provide 4 hours of on-site training on the use, operation, and maintenance for each of the following for not more than 6 designated Department employees:

1. PLC
2. Lighting Fixtures
3. Lighting Contactors
4. Dimming control modules
5. Light sensors/photo-diodes

Trainers for the PLC and other equipment must be an authorized representative of the manufacturer. and must have complete knowledge of PLC installation and operation.

Notify the Engineer not less than 5 days in advance of the proposed training class.

99-16010C(4) Manufacturer's Technical Support

The supplier of the PLC and other controls must provide technical assistance and guidance in operation, programming, maintenance and trouble shooting of operational problems of the entire system, for 5

year period following the SAT.

The technical support must include:

1. Telephone consultation
2. On call site consultation

Technical support must be provided at no additional cost to the Department.

99-16010C(5) Manufacturer's Warranty

All electrical equipment and material installed on this project must have replacement warranty from manufacturers against any defect for the period of 12 months from the date of acceptance of the Contract.

The manufacturer’s replacement warranty for the following equipments must be 5 years:

1. Tunnel lighting fixtures
2. Hot standby PLC
3. Lighting Contactors
4. Light sensors/photo-diodes
5. Photoelectric unit
6. Light dimming Control Modules

99-16010C(6) Identification of Equipment, Devices and Terminal Blocks

All newly installed electrical equipment, devices, and each terminal block must be identified with nameplates.

Nameplates must be made from laminated phenolic plastic with white core and black front and back.

Nameplate inscription must be in capitals letters etched through the outer layer of the nameplate material.

Nameplates must be fastened with self-tapping, cadmium plated screws or nickel-plated bolts.

99-16010C(7) Arc-Flash Hazard Warning Labels

Arc-flash hazard warning labels must be installed on all electrical equipments, enclosures and junction boxes with electric energized parts within them.

Warning labels must comply with NEC article 110.16 and 110.24.

Warning labels must comply with ANSI Z535.4.

99-16010D Payment

Not Used

99-16020 SYSTEM STUDIES

99-16020A General

99-16020A(1) Summary

Scope: The following systems studies work for the electrical system must be performed by a firm hired and paid by you. You must cooperate with the firm as required to accomplish the work. The work must include furnishing labor and material to perform the:

1. Engineering services for the electrical systems studies including short-circuit and overcurrent protective device coordination studies
2. Electrical arc flash hazard calculations and analysis, Personal Protective Equipment (PPE) and Clothing requirements, and Arc Flash Hazard Training
3. Overcurrent protective device setting for all electrical equipment and systems
4. Commissioning assistance must be provided by the firm for final coordinated systems

99-16020A(2) Type of Studies And Data Collection Requirements

Data Collection: Furnish all data as required by the firm performing the system studies. The firm performing the short-circuit and protective device coordination studies and arc flash hazard calculations and analysis must furnish you with a listing of required data immediately after the award of the contract. Expedite collection of the data to assure completion of the system studies as required for final authorization of the power distribution equipments shop drawings and prior to the release of the power distribution equipment for manufacturing. Power distribution equipment includes service equipment, integrated facility switchboard, electrical equipments and control panels. Load data utilized must include proposed loads obtained from the contract plans. Obtain required equipment data if necessary to satisfy the system study requirements.

Short Circuit And Overcurrent Protective Device Coordination Studies:

Short-circuit and overcurrent protective device coordination studies must be performed by a licensed Professional Electrical Engineer in the State that works directly for the firm manufacturing and supplying the main service switchboard and tunnels control equipment or is directly hired by the firm manufacturing and supplying the equipment and must include the:

1. Complete single-line diagram of system studied with all buses identified to report data
2. Obtain available Short circuit current from the utility inclusive of percent impedance of the utility transformer and their X/R ratio
3. Three phase and line to ground short-circuit currents at major buses extended down to system buses
4. Relay settings and protection device coordination studies which consist of:
   - 4.1 Coordinated composite time-current characteristic curves including recommended ratings and settings of all overcurrent protective devices in tabulated form; and associated calculations to demonstrate that the power system protection must be selectively coordinated by the use of devices or equipment submitted
   - 4.2. Situations where system coordination is not achievable due to overcurrent protective device limitations along with inadequate interrupting ratings must be noted
   - 4.3. Operational conditions during normal operation, alternate operations, and during emergency power conditions, and must include ground fault protective device settings
   - 4.4 Settings of overcurrent protective device must be coordinated and authorized by the Department
5. Data consisting of manufacturer's time-current characteristic curves for individual overcurrent protective devices, recommended settings of adjustable overcurrent protective devices, and recommended ratings of non-adjustable overcurrent protective devices

6. The data utilized in the study must match the data on the shop drawings and project plans and specifications.

The short circuit and overcurrent protective device coordination study must be submitted within 30 days after the power distribution equipment submittals have been received for review . The electrical equipment submittals will be reviewed but will not be authorized until the power systems studies have been received, reviewed, and-authorized.

Electrical Arc Flash Hazard Calculation and Analysis:

Electrical arc flash hazard calculation and analysis must be performed by a licensed Professional Electrical Engineer in the State and must include the:

1. Review of the tripping time for the overcurrent protective devices and associated arc energy during arc fault
2. Recommendations for equipment modifications to you prior to making shop submittals for power distribution equipments for inclusion into the final shop submittals of power distribution equipments at no cost to the State
3. Technical reports
4. PPE recommendations
5. Relay coordination properly adjusted based upon the arc flash hazard analysis
6. Arc flash hazard training by a qualified professional in the field of electric arc hazard for 4 hours on site for a maximum of 12 designated Department employees

During electrical arch flash hazard calculation and analysis, every effort such as selection of overcurrent protective devices and equipment and selection of AC magnetic trip adjustment for overcurrent protective devices must be considered and put into practice so that Category of Hazard as defined by NFPA 70E at any location under consideration is either "0 or 1." Category of hazard greater than these values will be rejected. If the firm performing system studies end up performing multiple studies to comply with these requirements, no additional compensation will be made and cost of performing all studies must be considered as part of the lump sum payment for the tunnels and control building work.

Software program such as SKM, EDSA, or EasyPower must be used in conducting arc flash hazard study. Study must be complete including proper list of actual data of the installed equipment and any assumption made in case where actual equipment data was not available. Before finalizing the study analysis provide a draft copy to the Engineer for reviews and authorization.

99-16020A(3) Definitions

Not Used

99-16020A(4) Submittals

Submittals for review and authorization: The system studies must be submitted to the Engineer prior to receiving final authorization of the service equipment, electrical equipments, and associated equipment shop drawings and prior to release of equipment drawings for manufacturing. Any shop drawings submitted for review must be 22 by 34 inches in size. If formal completion of the studies may cause delay in equipment manufacturing, authorization from the Engineer may be obtained for preliminary submittals of sufficient study data to ensure that the selection of device and characteristic will be satisfactory.

Submittals for construction: The results of the system studies must be summarized in a final report. One CD with PDF files and two bound copies of the complete final comprehensive report must be submitted. Final comprehensive report must include:

1. Report summary with analysis mythology, findings and recommendations
2. Summary of input data for Utility source, equipment and cables
3. Available fault at each equipment location with comparison to equipment rating
4. Overcurrent protective device settings (e.g. pick-up, time delay, curve)
5. Incident energy level (calories/cm2) for each equipment location and PPE
6. Overcurrent protective device coordination curves including related section of the single line diagram

Working Labels in accordance with ANSI Z535.4 which must include as a minimum the following:

1. Arc hazard boundary
2. Working distance
3. Arc flash incident energy at the working distance
4. PPE category and description of glove rating
5. Voltage rating of the equipment
6. Limited approach distance
7. Prohibitive approach distance
8. Equipment/bus name
9. Date prepared
10. Arc Flash hazard study prepare name and address

99-16020A(5) Quality Control and Assurance

The Short-circuit and overcurrent protective device coordination studies must be performed in accordance with applicable ANSI and IEEE Standards..

Electrical arc flash hazard calculation and analysis must be performed in accordance with CEC and NFPA 70E, 29 CFR 1910 Subpart S, "Electrical", IEEE 1584 and IEEE 1584a.

Licensed Professional Electrical Engineer must have at least 5 years of current experience in the design of coordinated power system protection and arc flash hazard calculations and analysis. Experience data must include at least five references for work of a magnitude comparable to this contract.

99-16020B Materials

Not Used.

99-16020C Construction

99-16020C(1) Field Settings

Adjust overcurrent protective device settings according to the recommended settings table provided by the system study report. Field adjustments of the overcurrent protective devices must be completed by the engineering service division of the power distribution equipment manufacturer. Make minor modifications to equipment as required to accomplish conformance with short circuit and overcurrent protective device coordination study and electric arc flash hazard calculations and analysis report. Notify the Engineer in writing of any required major equipment modifications, if required to comply with the system study requirements.

99-16020C(2) Field Testing

Not Used

99-16020C(3) Warranty

Following completion of system studies, field setting and the startup by the field engineering service of the equipment manufacturer, a 12 month warranty must be provided on all components manufactured by the engineering service parent manufacturing company.

99-16020D Payment

Not Used

99-16050 BASIC MATERIALS AND METHODS

99-16050A General

99-16050A(1) Summary

Scope: This work consists of furnishing and installing the basic materials for the electrical work, including conduits, conductors, fittings, and wiring devices. The basic materials must include those accessories and appurtenances, not mentioned, that are required for the installation and operation of the electrical system.

Related Work:

Roof penetrations must be flashed and sealed watertight to comply with section 99-07411.

99-16050A(2) Definitions

Not Used

99-16050A(3) Submittals

Product Data:

Submit a list of all materials and equipment to be installed and the manufacturer's descriptive data.

Manufacturer's descriptive data must include catalog cuts, complete description, performance data and installation instructions for the materials and equipment.

99-16050A(4) Quality Control and Assurance

Not Used.

99-16050B Materials

99-16050B(1) Conduits and Fittings

Rigid Steel Conduit and Fittings:

Rigid steel conduit must be Type 1 complying with section 86-2.05A.

Type 1 conduit must have steel or malleable iron fittings.

Split or three-piece couplings must be electroplated, malleable cast iron couplings.

Insulated grounding bushings must be threaded malleable cast iron body with plastic insulated throat and steel, lay-in ground lug with compression screw.

PVC Coated Rigid Steel Conduit and Fittings: PVC coated rigid steel conduit and fittings must be Type 2 complying with section 86-2.05A.

Electrical Metallic Tubing (EMT) and Fittings:

EMT must be formed of cold rolled strip steel, zinc coated, and interior lined to comply with UL Standard 797 and ANSI C 80.3.

Couplings must be electroplated, rain and concrete tight, gland compression type, steel body couplings with malleable iron nuts.

Connectors must be electroplated, rain and concrete tight, gland compression type, steel body connectors with male hub, malleable iron nut and insulated thermoplastic throat.

Rigid Non-Metallic Conduit and Fittings:

Rigid non-metallic conduit must be Type 3 complying with section 86-2.05A.

Couplings must be PVC, socket type or thread on one end and socket type on the other end as required for the particular application.

Terminal adapters for adapting PVC conduit to boxes, threaded fittings, or metallic conduit system must be PVC adapters with threads on one end and socket type on the other end.

99-16050B(2) Cables and Conductors

Cables:

Ambient light sensor/photodiode cable must be three insulated conductors shielded cable with drain wire and outer PVC jacket and be suitable for the equipment selected.

Tunnel interior light sensor/photodiode cable must be the same as ambient light sensor cable and be of the size as recommended by the system integrator based upon the distance between the programmable logic controller input modules inside the Tunnels control Building and the interior light sensors inside the tunnel.

Light intensity reference signal cable must be three insulated conductors shielded cable with drain wire and outer PVC jacket, suitable for the equipment selected, and be of the size as recommended by the system integrator based upon the distance between the programmable logic controller output modules inside the Tunnels Control Building and the Tunnel Dimming Control Panel inside the tunnel.

Tunnel light fixtures dimming control cable must be three insulated conductors shielded cable with drain wire and outer PVC jacket, suitable for the tunnel light fixture selected, and be of the size so that intensity of light of the tunnel light fixtures not to vary more than 2 percent between the nearest and farthest light fixture inside the tunnel.

Conductors:

Conductors must be stranded copper wire of the size shown. Conductors must comply with ASTM B3 and ASTM B8. Conductor size must be based on AWG, except that conductor diameter must be not less than 98 percent of the specified AWG diameter.

Conductor insulation types must be as follows:

1. Conductors in control panel enclosures must be Type MTW.
2. Conductors in wet, underground, or outdoor locations must be Type XHHW-2.
3. All conductors other than Type MTW and XHHW-2 must be Type THHN.

Wire Connections and Devices: Wire connections and devices must be pressure or compression type, except that connectors for No. 10 AWG and smaller conductors in dry locations may be preinsulated spring-pressure type.

99-16050B(3) Electrical Boxes

Outlet, Device and Junction Boxes:

Boxes for the duplex receptacles, light switches, and light fixture outlets must be galvanized steel boxes with knock-outs and must be the size and configuration best suited to the application shown. Minimum size of outlet, device, or junction boxes must be 4 inches square by 1-1/2 inches deep. Flush-mounted single device and surface mounted light fixture boxes must have four inch square single raised device covers.

Stainless steel junction boxes inside the tunnels must be as shown.

Cast boxes and weatherproof boxes must be cast iron boxes with threaded hubs complying with NEMA FB-1, and must be of the size and configuration best suited to the application shown. Minimum size of outlet, device, or junction boxes must be 4 inches square by 1-7/8 inches deep.

Cast boxes and weatherproof boxes must have cast iron covers with gaskets.

Weatherproof device boxes must have gasketed covers with gasketed hinged flaps to cover switches and receptacles.

Pull Boxes:

Pull boxes must comply with section 86-2.06.

Traffic rated pull boxes must comply with section 86-2.06C.

Electrical pull box covers and traffic rated pull box covers must be marked "ELECTRICAL."

99-16050B(4) Receptacles and Switches

Ground Fault Circuit Interrupter Receptacle, (GFCI): GFCI receptacle must be NEMA Type 5-20R, feed-through type, ivory color, 3-wire, 20-ampere, 125-volt, specification grade, duplex receptacle suitable for wiring with stranded conductors. Receptacle must detect and trip at current leakage of 5 mA and must have front mounted test and reset buttons.

Duplex Receptacle: Duplex receptacle must be NEMA Type 5-20R, 3-wire, 20-ampere, 125-volt, ivory color, specification grade duplex receptacle suitable for wiring with stranded conductors.

Single Pole Switch: Single pole switch must be 20-ampere, 120/277-volt, quiet type, specification grade, ivory color switch with silver alloy contacts. Switch must be suitable for wiring with stranded conductors.

99-16050B(5) Occupancy Sensor Switches

Not Used

99-16050B(6) Miscellaneous Materials

Warning Tape: Warning tape must be 4 inches wide and contain the printed warning "CAUTION ELECTRICAL CONDUIT" in bold 3/4-inch black letters at 30-inch intervals on bright orange or yellow background. The printed warning must be non-erasable when submerged under water and resistant to insects, acids, alkali, and other corrosive elements in the soil. The tape must have a tensile strength of not less than 155 pounds per 4-inch wide strip and must have a minimum elongation of 700 percent before breaking.

Pull Rope: Pull rope must be nylon or polypropylene with a minimum tensile strength of 1800 pounds.

Watertight Conduit Plug: Watertight conduit plug must be a hollow or solid stem expansion plug complete with inner and outer white polypropylene compression plates and red thermoplastic rubber seal. Seal material must be non-stick type rubber resistant to oils, salt, and alkaline substances.

Anchorage Devices: Anchorage devices must be corrosion resistant, toggle bolts, wood screws, bolts, machine screws, studs, expansion shields, or expansion anchors as required by the supporting device.

Electrical Supporting Devices:

Electrical supporting devices must be one hole conduit clamps with clamp backs, hot-dipped galvanized, malleable iron.

Construction channel must be 1-5/8 inches x 1-5/8 inches, 12-gage galvanized steel channel with 17/32-inch diameter bolt holes, 1-1/2 inches on center in the base of the channel.

Ground Rod: Ground rod must be a 3/4-inch (minimum) galvanized or copper clad steel rod, 10 feet long, and must conform to the requirements in NEMA GR-1.

99-16050C Construction

Conduit:

Conduits must be installed to comply with section 86-2.05C and the following:

1. All conduits must be rigid steel except as follows:
   a. EMT may be used in walls and furred spaces and for exposed work indoors above the switch height and inside the Tunnel Control Building only.
   b. PVC coated rigid steel conduit must be used for base elbows and vertical risers through concrete slabs.
   c. Rigid non-metallic conduit must be used in underground, exterior locations.
2. Rigid non-metallic conduit bends of 30 degrees or greater must be factory-made long radius sweeps. Bends less than 30 degrees must be made using an authorized heat box.
3. Locations of conduit runs must be planned in advance of the installation and coordinated with the ductwork, ceiling and wall construction in the same areas and must not unnecessarily cross other conduits or pipe, nor prevent removal of ceiling tiles or panels, nor block access to mechanical or electrical equipment.
4. Where practical, conduits must be installed in groups of parallel, vertical or horizontal runs and at elevations that avoid unnecessary offsets.
5. Exposed conduit must be installed parallel and at right angles to the building lines.
6. All raceway systems must be secured to the building structures using specified fasteners, clamps and hangers.
7. All metal conduits, fittings, and elbows in contact with soil or concrete must be wrapped with a double layer of 20-mil thick pipe wrapping tape.
8. Single conduit runs must be supported by one hole conduit clamps. Single conduit runs on walls in damp or wet locations must be installed with clamp backs to space conduit off the surface.

9. Multiple conduit runs must be supported with construction channel secured to the building structure. Conduits must be fastened to construction channel with channel compatible pipe clamps.
10. Raceways of different types must be joined using authorized couplings or transition fittings.
11. Expansion couplings must be installed where conduit crosses a expansion joint.
12. All floor and wall penetrations must be sealed watertight.

Conduit Terminations:

Rigid steel conduits must be securely fastened to cabinets, boxes and gutters using 2 locknuts and insulating metallic bushing. EMT must be securely fastened to cabinets, boxes and gutters using connectors. Conduit terminations at exposed weatherproof and cast boxes must be made watertight using hubs.

Grounding bushings with bonding jumpers must be installed on all conduits terminating at concentric knockouts and on all conduits containing service conductors, grounding electrode conductor, and conductors feeding separate buildings.

Rigid non-metallic conduit must be terminated inside the underground pull boxes with an authorized conduit bushing or fitting. All conduits must enter vertically through the bottom of pull boxes.

All future conduits terminated in underground pull boxes or left exposed indoors and outdoors must be provided with watertight conduit plugs.

Warning Tape: Warning tape must be placed over each conduit in a trench. Each warning tape must be centered over the conduit and must be placed over the 6 inch layer of sand covering the conduit.

Conductor and Cable Installation:

Conductors must not be installed in conduits until all work of any nature that may cause injury is completed. Care must be taken in pulling conductors so that insulation is not damaged. An authorized non-petroleum base and insulating type pulling compound must be used as needed.

All cables must be installed and tested to comply with manufacturer's instructions.

Splices and joints must be insulated with insulation equivalent to that of the conductor.

Six inches of slack must be provided at each outlet and device connection. If the outlet or device is not at the end of a run of conductor, connection must be made with correctly colored pigtails tapped to the runs with splices.

All pressure type connectors and lugs must be retightened after the initial set.

Splices in underground pull boxes and similar locations must comply with section 86-2.09C and section 86-2.09E.

Conductor Identification:

The neutral and equipment grounding conductors must be identified as follows:

1. Neutral conductor must have a white or natural gray insulation except that conductors No. 4 and larger may be identified by distinctive white markers such as paint or white tape at each termination.
2. Equipment grounding conductor may be bare or insulated. Insulated equipment grounding conductors must be green or green with one or more yellow stripes over its entire length. Conductors No. 4 and larger may be permanently identified by distinctive green markers such as paint or green tape at all accessible locations over the entire exposed conductor.

Ungrounded feeder and branch circuit conductors must be color coded by continuously colored insulation, except conductors No. 6 AWG or larger may be color coded by colored tape at each connection and where accessible. Except as shown, ungrounded conductor color coding must be as follows:

SYSTEM	COLOR CODE
120/208 volt-Three phase	Black, red, blue
277/480 volt-Three phase	Brown, orange, yellow

Once grounded and ungrounded insulated conductors are identified with a specific color code, that color code must be used for the entire length of the circuit.

Where more than one branch circuit enters or leaves a conduit, panel, gutter, or junction box, each conductor must be identified by its panelboard and circuit number. All control conductors including control conductors of manufacturer supplied and field wired control devices must be identified at each termination with the conductor numbers shown and shop drawings, where deemed necessary. Identification must be made with one of the following:

1. Adhesive backed paper or cloth wrap-around markers with clear, heat shrinkable tubing sealed over either type of marker.
2. Pre-printed, white, heat-shrinkable tubing.

The identifying numbers of the terminating conductors, as shown or the shop drawings, must be identified on the terminal block marking strip.

Outlet, Device and Junction Box Installation:

Where exposed rigid steel conduits are connected to an exposed outlet, device, or junction box at or below switch height, the box must be a cast box.

No unused openings must be left in any box. Threaded conduit plugs must be installed to close openings.

Adjustments to locations of outlet, device and junction boxes may be made as required by structural conditions and to suit coordination requirements of other trades.

Pull Box Installation:

Pull box installation must comply with section 86-2.06C and the following:

1. Top of pull boxes must be flush with surrounding grade or top of curb. In unpaved areas where pull box is not immediately adjacent to and protected by a concrete foundation, pole or other protective construction, the top of pull box must be set at plus one inch above surrounding grade. Pull boxes shown in the vicinity of curbs must be placed adjacent to the back of curb.

Grounding Electrode Installation: The grounding electrode must be driven vertically until the top is 6 inches above the surrounding surface. When vertical penetration of the ground rod cannot be obtained, an equivalent horizontal grounding system, authorized by the Engineer, must be installed.

Anchorages:

Hangers, brackets, conduit straps, supports, and electrical equipment must be rigidly and securely fastened to surfaces by means of toggle bolts on hollow masonry; expansion shields and machine screws, or expansion anchors and studs or standard preset inserts on concrete or solid masonry; machine screws or bolts on metal surfaces; and wood or lag screws on wood construction.

Anchorage devices must be installed to comply with the anchorage manufacturer's instructions.

Mounting heights: Electrical system components must be mounted at the following mounting heights, unless otherwise shown. The mounting height dimensions must be measured above the finished floor to the bottom of the device or component.

Wall switches	3’-8”
Convenience outlets	1’-6”

99-16050D Payment

Not Used

99-16420 MAIN SERVICE SWITCHBOARD

99-16420A General

99-16420A(1) Summary

Scope: This work consists of furnishingmain service switchboard, service entrance busbars system and busbars vertical supports and horizontal wall seismic bracing equipment to comply with the serving utilities.

Related Work:

Permits, licenses, charges, fees and costs for arranging utility connections and extensions must comply with "Utility Connection," in section 010000.

Concrete and reinforcement for service and distribution equipment must comply with section 99-03300.

99-16420A(2) Definitions

Not Used

99-16420A(3) Submittals

Installation Details: After the short circuit coordination and arc flash hazard study has been authorized, you must submit complete service installation shop drawings to the Engineer. After drawings have been authorized, you must submit an authorized copy of the service installation shop drawings to the servce utility for their review and authorization. Resubmit service installation shop drawings as needed until authorized.

Product Data:

Submit a list of all materials and equipment to be installed and the manufacturer's descriptive data.

Manufacturer's descriptive data must include complete description, performance data and installation instructions for the materials and equipment.

Shop Drawings: Shop drawings must show the internal electrical components layout, shape, size, and method of attachment for each component used in the work. Control and wiring diagrams must include rough-in dimensions, component layout and conductor number identification.

Test Reports: Test results for main service switchboard equipment must be delivered to the Engineer within 3 business days of completion.

99-16420A(4) Quality Control and Assurance

Prior to shipping, the Manufacturer must test the assembled main service switchboard equipment for proper operation.

99-16420B Materials

Main Service Switchboard Equipment:

Enclosure must be NEMA 12 enclosure consisting of a service entrance busbars landing section, a fully bussed meter and current/potential transformers section, and fully bussed service disconnect section. There must be an horizontal busbar on top of the service disconnect section as shown to make connection to the tunnel control equipment sections as shown. All busbars to be tin coated copper busbars. Exterior must be 12-gage sheet steel. The interior must be 14-gage sheet steel. All screws, latches, hasps, hinge pins and similar hardware must be stainless steel. Exterior door must be deadfront and lockable with a padlock. The withstand ratings of the assembled service switchboard must be 65,000 amperes (symmetrical) at 480 volts. Service disconnect must have externally operable handle. The main service switchboard and all interior components must be factory assembled. Enclosure finish must be baked enamel or baked thermosetting polyester finish.

Service disconnect switch must be 3-pole, 600-volt, 1200-ampere frame, 1200-ampere trip, UL1066 listed, molded case circuit breaker with shunt trip and ground fault protection. The interrupting capacity of the circuit breaker must be 65,000 amperes (symmetrical) at 480 volts. The ground fault protection system must be of the residual sensing method type. The service disconnect must have a microprocessor based trip unit. The trip unit must have integrally mounted phase current sensors and one identical rating current sensor on the neutral bus. Under normal operating conditions, the vector sum of all phase currents in the current sensors must be zero and underground fault conditions, the residual sum will not be zero and as a result must initiate an internally powered shunt trip of the circuit breaker.

Service Entrance Busbars:

Service entrance busbars system must be of 1200-ampere, copper only, 3-phase, 4-wire, and 600-volt rated and weatherproof type construction bus bars system suitable for 65,000 amperes (symmterical amperes) withstand rating and complete with vertical and seismic braces support system.

99-16420C Construction

Installation of the main service switchboard and service entrance busbars must be in accordance with the requirements of the servce utility company and as shown on the authorized drawings. You must provide seismic anchoring and equipment anchorage details coordinated with the equipment mounting provisions, prepared and stamped by a licensed civil engineer.

99-16420D Payment

Not Used

99-16432 TUNNELS CONTROL EQUIPMENT

99-16432A General

99-16432A(1) Summary

Scope: This work consists of designing, furnishing and installing fully operational tunnels control equipment and related accessories.

System Description:

The tunnels control equipment must control and operate the following:

1. Tunnel No. 1, 2, and 3 lighting fixtures ON/OFF and it must be based upon the ambient light level. The tunnel light fixtures intensity between 0 to maximum must be based upon the ambient light level outside the tunnel and the light level within the tunnels.
2. Peripheral devices, including ambient light sensor, and light sensors inside each tunnel.

On night time, only night time tunnel light fixtures must be ON. The light intensity of the night time tunnel light fixtures must be adjusted in the field in a manner to provide night time luminance of 5 foot candles on the roadway pavement.

On day time, lighting level of all lighting fixtures inside the tunnels must be controlled between 10 percent to maximum based upon the ambient light level outside the tunnel. The maximum day time light level inside the tunnels must be adjusted to provide day time luminance of 230 foot candles on the roadway pavement.

The interior tunnels light sensor must monitor the health of the tunnel's interior cleanliness and must provide feedback to the tunnels control equipment. Based upon the feedback from the tunnels interior light sensors, the maximum day and night time luminance inside the tunnels must be compensated continuously.

Related Work:

Anchorage devices must comply with section 99-16050.

Attention is directed to the requirements specified under section 99-16020.

Attention is directed to requirements for testing and training for main service switchboard as specified under section 99-16010.

99-16432A(2) Definitions

CPU: Central Processing Unit

I/O: Input/Output

PLC: Programmable Logic Controller

ISA: International Society of Automation

CSIA: Control System Integrators Association

99-16432A(3) Submittals

Installation Details: After the short circuit coordination and arc flash hazard study has been authorized, you must submit complete installation shop drawings to the Engineer for review and authorization.

Product Data:

Submit a list of all materials and equipment to be installed and the manufacturer's descriptive data.

Manufacturer's descriptive data must include complete description, performance data and installation instructions for the materials and equipment. Control and wiring diagrams, rough-in dimensions, and component layout must be included where applicable. All control and power conductors on the shop drawings must be identified with wire numbers.

Manufacturer's descriptive data must be submitted for the following:

1. 15 kVA Transformer, transformer primary and secondary disconnects and Panelboard P
2. Panelboard T1 and Lighting Contactors Panel LC T1
3. Panelboard T2 and Lighting Contactors Panel LC T2
4. Panelboard T3 and Lighting Contactors Panel LC T3
5. Corner Section
6. Hot Standby Programmable Logic Controller cabinet and all associated control components
7. Tunnel No. 1, 2 & 3 Light Circuit Divider Control Panels
8. Tunnel No. 1, 2 & 3 Dimming Control Panels

Shop Drawings:

Shop drawings must be submitted for authorization. Shop drawings must be of 22 by 34 inch size and include one line of the complete tunnel control equipment. An open source networking communication protocol must be used. Submit the ladder logic diagram for the entire system for authorization.

Closeout Submittals:

Prior to the completion of the contract, 3 sets of identified paper copies in bound form, and 5 sets of CDs or DVDs containing PDF copies of the Operation and Maintenance Manual must be delivered to the Engineer. Bound manuals must include tabbed dividers. The instructions and part list must be complete for the equipment installed. The Operation and Maintenance Manual must include the following:

1. Index
2. Specifications
3. General operation theory
4. Functions of all controls
5. Troubleshooting technique
6. Diagnostic routines
7. Block circuit diagrams
8. Component layout
9. Schematic diagram
10. Parts list, including stock numbers and source

Incomplete manuals will be returned for correction and resubmitted.

99-16432A(4) Quality Control and Assurance

System Integrator:

The work must be performed by an independent System Integrator hired by you.Coordinate with the System Integrator as required to accomplish the work.

The System Integrator company must have experience in incorporating controls centers, instrumentation, and programmable logic controllers. The System Integrator must have been in business for at least 10 years under the same name performing and implementing the same type of integrated systems in these markets as well as being a certified UL shop with certifications under UL-891 and UL-508. The System Integrator must have ISA certified technicians and be a CSIA certified integrator with current certification on file for review at any time.

The System Integrator must provide, install, assemble, develop program and program all hardware for tunnels control system as shown. The System Integrator must design and configure automatic dimming of tunnel lighting fixture in all tunnels. The system must be calibrated, tested, programmed and setup such that the Department's operators can manage the system after the specified training without the need for assistance. All PLCs, and tunnels dimming control panels software must be installed and configured by the System Integrator.

The System Integrator must be responsible for final connection and calibration of all computer and computer peripheral equipment that would not normally fall within the capabilities of electricians. The System Integrator must review and assist the Engineer in authorizing the submittals for all the software related equipment.

The technical support must be provided at the Pasadena Tunnels site as required, by an authorized representative of the PLC system integrator and by telephone.

Source Quality Control:

The System Integrator must conduct a factory test of the completed system witnessed by the Engineer prior to shipment to the job site. The factory test must include the complete functioning of the Tunnels Control Equipment, the Tunnel Dimming Control Panels, and auto dimmable tunnel light fixtures to verify communication compatibility and to make certain that tunnel light fixtures are being dimmed between 10 to 100 percent of their maximum intensity. The testing must include dummy ambient and tunnel interior light inputs and output signals to mimic the signals from the various field devices and equipments which are to be installed in the field. The system integrator must make an arrangement to borrow a maximum of four light fixtures for testing. Failure modes must be included in the testing. The equipment must not be shipped to the job site until written release to ship is received from the Engineer.

The PLC control cabinet and the dimming control panels must be burned in as a complete system for 7 consecutive days by the System Integrator prior to testing. All control components must be subjected to an elevated temperature test of 120 degrees F for 24 hours while operating continuously. The complete system must cycle through all commands every 10 minutes.

Any panels or components that exhibit erroneous behavior must be repaired or replaced, and testing repeated until successful.

Technical Support:

The PLC software supplier and Control System Integrator must provide comprehensive software maintenance and support program to ensure that the user receives full benefit of the software for the duration of its life cycle. The PLC software supplier and System Integrator must have a staff of experienced personnel available to provide service on 24-hour notice. Such personnel must be capable of fully testing and diagnosing the hardware and software delivered and of implementing corrective measures.

The PLC software supplier and System Integrator must provide technical assistance and guidance in the operation, maintenance and trouble shooting of operational problems for the PLC system for one year following the acceptance of the entire contract. The technical support must also be provided at no additional cost to the State.

The technical support must be provided at the Pasadena Tunnels site as required, by an authorized representative of the PLC system integrator and by telephone.

Warranty: A five year factory warranty must be provided for hot standby PLC and tunnels control equipment.

99-16432B Materials

99-16432B(1) Tunnels Control Equipment

Tunnels control equipment enclosure must be completely factory assembled, switchboard type equipment complying with UL-508 and wired in conformance with NEMA Class IIC wiring. Tunnels control equipment enclosure must be of NEMA 12 type enclosure consisting of an 15 kVa transformer with primary and secondary protection circuit breakers and Panelboard P section, Panelboard T3 section, Lighting Contactors LC-T3 section, corner section, Panelboard T2 section, Lighting Contactors LC-T2 section, PLC Control Cabinet and associated controls equipment section, Panelboard T1 section, and Lighting Contactor LC-T1 section. There must be a horizontal busbar on top of all sections as shown to supply power to the tunnel control equipment sections as shown. Each tunnel power supply panelboards section must be equipped with a digital multimeter as shown. NEMA 12 enclosure exterior must be 12-gage sheet steel and the interior must be 14-gage sheet steel. All screws, latches, hasps, hinge pins and similar hardware must be stainless steel. Exterior door must be lockable with thumb type screws. All circuit breaker handles must be externally operable type handle. The tunnels control equipment and all interior components must be factory assembled and be rated to withstand 65,000 amperes (symmetrical amperes) of fault current. Enclosure finish must be baked enamel or baked thermosetting polyester finish.

99-16432B(2) Panelboards

Panelboard P must be indoor type, factory assembled, 3-phase, 4-wire, 120/208-volt, AC panelboard with 50-ampere main circuit breaker, insulated groundable neutral, hinged door and molded case branch circuit breakers as shown. The interrupting capacity of the circuit breaker must be 22,000 amperes (symmetrical) at 240 volts, AC.

99-16432B(3) Starters

Not Used

99-16432B(4) Switches

Not Used

99-16432B(5) Transformer

Transformer must be indoor, dry type, 3-phase, 480-volt primary, 120/208-volt secondary, 15-KVA transformer. Transformer must have two 2 1/2 percent full capacity taps above and four 2 1/2 percent full capacity taps below normal primary voltage.

Transformer primary and secondary circuit breakers must be of the voltage, number of poles and ampere rating as shown. The interrupting capacity of the primary circuit breaker must be 65,000 amperes (symmetrical) at 480 volts.

Panelboard T1, T2, and T3 must be indoor type, factory assembled, 3-phase, 4-wire, 480-volt, AC panelboard with 400-ampere main circuit breaker with adjustable magnetic trip, insulated groundable neutral, hinged door and molded case branch circuit breakers as shown. The interrupting capacity of the main circuit breaker must be 65,000 amperes (symmetrical) at 480 volts, AC.

The digital multimeter for all tunnels supply panelboards must be combination voltage and current metering device rated for 480-volts input voltage complete with busbar type current transformers, scroll up/down button for both voltage and current, and equipped with red LED type display.

Lighting contactors for all tunnel LC-T1, LC-T2 & LC-T3 sections must be 3-pole, 480-volt, 50-ampere continuous rated lighting contactors with 120-volt coil. Lighting contactor must be of electrically operated and mechanical held type contactors.

PLC Control Cabinet:

The hot standby PLC system must be standalone type, the main PLC and hot standby PLC to control the operation of the tunnels lighting system complete with communication modules, programming software, I/O modules, expansion chassis, selector switch to manually switch between main and hot standby PLC, pilot lights to indicate PLC controlling, interconnecting cables, and fusible type terminal blocks. Upon failure of the main PLC, the HOT standby PLC must take over the controls without interruption.

The Department utilizes Tesco Controls of Sacramento Site Glass enterprise SCADA control system for centrally monitoring of various sites within the State. Therefore, hot standby PLC type selected must be fully compatible with existing SCADA hub equipment (located in Sacramento, CA) and be setup in an manner to interoperate smoothly with the SCADA hub equipment. Information relating to Tesco's SCADA system can be obtained from Tesco Controls of Sacramento, CA. The PLC must be capable of communicating with all the field devices specified elsewhere in these special provisions.

Each PLC must consist of a power supply and central processing unit operating and must operate under the following conditions:

1. Operating temperature: 32to 140 degrees F
2. Humidity: 20 to 95 percent non-condensing
3. Vibration: 57 to 150 HZ, constant acceleration 2 g
4. Shock: 15 g, semi-sinusoidal 11 ms.
5. Noise immunity: 27 thru 500 MHz, 10V/m

Both PLCs must be programmed with a ladder logic diagram consisting of relays symbols and function blocks. The program must be stored in a non-volatile memory, or may include battery back-up. If used, the battery must be easily accessible and serviceable without taking the CPU out of operation. An LED must be provided to indicate if the battery is low and requires replacement.

Both PLCs must be equipped with a computer link module using TIA-232 ports that allow them to communicate with a personal computer for programming and monitoring.

The I/O modules must be capable of being plugged into the expansion chassis in any slot. Each module must have a keyed plug in the terminal strip for each type of module for ease of replacement of the module without having to disturb the wiring connection. Each terminal strip must have a marking strip under a clear plastic cover inscribed with the conductor identification. Each module must have LED indicators to show when each input or output is turned on. Each module must operate at the voltage level of the connected devices and must be optically isolated from the PLC.

The discrete module must be not more than 16 points.

Analog output must have the current or voltage level output required by the connected field devices. Analog I/O modules must have built-in signal isolator rated 1000-V(dc) or higher.

Wiring diagram for input and output modules is diagrammatic only. Install an external network necessary to counteract induced signals, capacitance on the conductors for proper operation of the input or output modules, to minimize the interference on the PLC, and as recommended by the manufacturer. Compatibility between external devices and the output modules, in regards to current sinking or sourcing, must be matched to avoid operational problems.

Provide CPU, power supply, I/O modules and analog module with the following characteristics:

General	Value
Clock speed	20Mhz
Onboard memory	
RAM	2M bytes
Flash PROM	1M bytes
Programming memory	
Logic Space	512K words
Discrete I/O References	64k
Register References	64k
Scan Performance	
Logic Solve time (minimum)	0.3 ms/k
Logic Solve time (type)	0.3-1.4 ms/k
Local I/O Configurations	
Local I/O Words	64 in / 64 out
Local I/O Backplanes	1
Remote I/O Configurations	
Remote I/O Words/Drop	64 in / 64 out
Remote I/O Drop	31
Remote I/O Networks	1
Communication Ports	
Serial	6
Modbus, Modbus Plus	1 each
Power Consumption	790 mA
Backplane Slots	1

Power Supply	
Input voltage	115/230 VAC
Operating Voltage	110-276 VAC
Input Frequency	47-63 Hz
Input Power Interruption	10 ms
Output Voltage	5.1 VDC
Output Rating	3 A

Analog 4-20 mA Current Mode or 0-5V or 0-10 V Voltage Mode input modules	
Number of points	16
Resolution	12 bits
Type	Differential or Single Ended
Accuracy	± 0.01%
Linearity	± 0.01%
Input Impedance	4M ohm in voltage mode
Common mode Voltage	96V
Internal Sample Rate	10 us
Overcurrent Protection	Replaceable Fuse

12/24 VDC Input Modules	
Number of points	16
Operating Voltage	12-30 VDC
Maximum Continuous input voltage	30 VDC
Response Time	
OFF to ON	1 ms
Power Consumption	330 mA

120 VAC Input Modules	
Number of points	16
Operating Voltage	79-132 VAC
Max. Continuous input voltage	132 VAC
Response Time	--
OFF to ON	4.9 ms
ON to OFF	7.3 ms
Power Consumption	180 mA

120 VDC Output Modules	
Number of points	16
Operating Voltage	20-250 VAC
Max. Load current each point	2 A
Relay Type	Form A
Response Time	--
OFF to ON	10 ms
ON to OFF	20 ms
Power Consumption	1100 mA

12/24 VDC Output Modules	
Number of points	16
Operating Voltage	5.0 to 30 VDC
Max. Load current each point	0.5 A
Relay Type	Form A
Response Time	
OFF to ON	1 ms
ON to OFF	1 ms
Power Consumption	330 mA

Analog 0-10 V modules	
Number of points	4
Operating Voltage	0-10 VDC
Max. Load current each output	2 mA
Resolution	12 bits
Response Time	3 mS
Power Consumption	700 mA

Analog 4-20 mA Current Mode or 0-5V or 0-10 V Voltage Mode output modules	
Number of points	4
Operating Voltage	0-5 VDC or 0-10 VDC
Max. Load current each output	2 mA
Resolution	12 bits
Response Time	3 mS
Power Consumption	700 mA

The expansion chassis must contain slots to support all the I/O modules as shown on the plans. Include at least 25 percent empty slots.The PLC must be capable of two-way communication using the approved manufacturer interconnection method for the third party PLC's.

The software installation discs with all available manuals must be provided along with a disc copy of the running application program. The application program must include detailed comments for each controlled segment/network.

An Ethernet firewall must be provided for secured access to the hot standby PLC system. Ethernet firewall must be constructed from ruggedized metal housing, suitable for industrial environments.

General Purpose Relay: All general purpose control relays and interfacing relays that is not shown but may be required must be 3-pole, double-throw, plug in type relays with socket. Coil voltage and contact rating must suit external devices they are connected to.

DC Battery Charger System: DC battery charger system must be dual redundant type charger system of the voltage input and output rating as shown. Each battery charger system must be connected in parallel and must be capable of charging the DC batteries individually in the event one charger has failed. As part of the control, automatic disconnect devices must be provided to automatically disconnect the non functional battery charger system in the event of battery charger failure. Power supply modules must be adequately sized to supply all the needs for the PLC Control Cabinet and for the low voltage field installed devices with an additional 25 percent safety factor, over temperature protection, LED pilot lights, and thermostatically controlled cooling fan.

DC Batteries: DC batteries must be of dual redundant type battery system connected in parallel. Each battery module must be adequately sized to supply for minimum of four hours all the needs for the PLC Control Cabinet and for the low voltage field installed devices with an additional 25 percent safety factor.

Selector Switches: All selector switches must be heavy duty, general purpose switches with normally open or normally closed contacts. Contacts must have an inductive pilot duty rating of 60-ampere make, 6-ampere break, and 10-ampere continuous at 120-volt and 35 percent power factor.

Test Switches: All test switches must be heavy duty, general purpose pushbutton with normally open or normally closed contact. Contacts must have an inductive pilot duty rating of 60-ampere make, 6-ampere break, and 10-ampere continuous at 120-volt and 35 percent power factor.

Light Level Simulator Potentiometers: All light level simulator potentiometers must be heavy duty, rotary, panel mounted type potentiometer. Level simulator potentiometer must be suitable for providing simulated input to the PLCs.

Pilot Lights: All pilot lights must be panel-mounted, push to test type, heavy duty, plastic screw on lens, DC LED lamp pilot light of the voltage and color as shown.

Terminal Block: Terminal block must be tubular type, comply with NEMA ICS 4, be 30-ampere, 600-volt, molded plastic have high-pressure clamp connectors, DIN rail mounted or attached to the enclosure. Nameplates must be screwed to each block or a computer printed plastic label attached with adhesive. All conductor numbers must be identified.

PLC Operational Software:

The tunnels interior lighting system must be designed to operate in two modes. These two modes must be the night time and day time tunnel lighting modes. The night time mode must be the mode when the ambient light level is below 30 foot candles. The day time mode must be the mode when the ambient light level is 30 foot candles and above.

The PLC software must control the night time lighting mode as follows:

1. Night lights must be designed to operate 24/7/365. On night time, the tunnel light fixtures marked as night light must be dimmed automatically so that their maximum light output is adjusted to provide a night time luminance of 5 foot candles on the roadway pavement. In addition, this night time light level must be continuously adjusted up and down based upon the health of the tunnels interior cleanliness which is determined by the output of the tunnels interior light sensors.

2. While in "night mode" and upon depressing the DPB twice in a sequence within two seconds, a password prompt should appear on the laptop screen and ask the operator to put in the password. Once the password is accepted, the operator must be able to select the desired dimming level of all daytime tunnel lighting levels fixtures. Once the desired level is selected, pushing the DPB once should keep this selected level in memory until changed. This selected dimming level should remain in effect as long as the night time mode switch is in the ON position. For testing purposes, as long as the DPB is kept depressed, the operator should be able to change the dimming level of all daytime lighting fixtures. This level of dimming must remain ON only as long as the button is depressed and upon letting go the DPB, the dimming level should revert back to the original set dimming level.

The day time tunnel lighting mode must consists of four modes. These four modes must be the auto dimming, auto, manual, and OFF modes.

The PLC software must control the day time "auto dimming lighting mode" as follow:

1 For auto dimming mode, main tunnel selector switch SST1 thru SST3 and each tunnel selector switch T1SS1 thru T3SS3 must be in auto dimming and auto position respectively.

2. During dawn as the sun rises and when the ambient light level is above 30 foot candles, all lighting contactors inside the LC-T control panels must energize. Once all lighting contactors are energized, all tunnel lighting fixtures must come ON. The luminance inside the tunnel must vary between minimum of 10 percent to a maximum light output of 230 foot candles based upon the intensity of the ambient light level.
3. The tunnels interior health must be determined by the output of the tunnels interior light sensors. Therefore, the upper maximum limit of the luminance level inside the tunnel must be adjusted continuously up or down based upon the health of the tunnel's interior cleanliness.
4. As dusk approaches or due to foggy or cloudy conditions and as the ambient light level decreases, the luminance inside the tunnel must also decrease continuously.
5. When the ambient light level decreases below 30 foot candles, all lighting contactors inside all LC-T control panels must de-energize and tunnel interior light level controls must revert to night time lighting mode as described above.

The PLC software must control the day time "auto lighting mode" in 5 steps. These 5 steps are called Light Level 1 thru Light Level 5. Description below will be for Tunnel No. 1 and software steps for Tunnel No.2 and Tunnel 3 are similar:

1 For auto mode, main tunnel selector switch SST1 thru SST3, and each tunnel selector switch T1SS1 thru T3SS3, must be in the auto position.
2 When the Selector Switches, SST1 (master selector switch) thru SST3, and T1SS1thru T3SS1, are in "Auto" position, then the daytime Light Level 1 tunnel light fixtures must remain ON as long as the outside lighting level is more than the lighting level programmed on the PLC. When the Selector Switch, T1SS1 thru T3SS1, are in the "Hand" position, then all daytime Light Level 1 light fixtures must remain ON. When the Selector Switch, T1SS1 thru T3SS1, are in the "Photocell" position, then all daytime Light Level 1 light fixtures must energize or de-energize in a manner as determined by the output of the photocell.
3 When the Selector Switches, SST1 (master selector switch) thru SST3 and T1SS2 thru T3SS2, are in "Auto" position, then the daytime Light Level 2 tunnel light fixtures must remain ON as long as the outside lighting level is more than the lighting level programmed on the PLC. When the Selector Switch, T1SS2 thru T3SS2, are in the "Hand" position, then all daytime Light Level 2 light fixtures must remain ON. When the Selector Switch, T1SS2 thru T3SS2, are in the "Photocell" position, then all daytime Light Level 2 light fixtures must energize or de-energize in a manner as determined by the output of the photocell.
4 When the Selector Switches, SST1 thru SST3 (master selector switch), and T1SS3 thru T3SS3, are in "Auto" position, then the daytime Light Level 3 tunnel light fixtures must remain ON as long as the outside lighting level is more than the lighting level programmed on the PLC. When the Selector Switch, T1SS3 thru T3SS3, are in the "Hand" position, then all daytime Light Level 3 light fixtures must remain ON. When the Selector Switch, T1SS3 thru T3SS3, are in the "Photocell" position, then all daytime Light Level 3 light fixtures must energize or de-energize in a manner as determined by the output of the photocell.
5 When the Selector Switches, SST1 (master selector switch) thru SST3 and T1SS4 thru T3SS4, are in the "Auto" position, then the daytime Light Level 4 tunnel light fixtures must remain ON as long as the outside lighting level is more than the lighting level programmed on the PLC. When the Selector Switch, T1SS4 thruT3SS4, are in the "Hand" position, then all daytime Light Level 4 light fixtures must remain ON. When the Selector Switch, T1SS4 thru T3SS4, are in the "Photocell" position, then all daytime Light Level 4 light fixtures must energize or de-energize in a manner as determined by the output of the photocell.

6 When the Selector Switches, SST1 (master selector switch) thru SST3 and T1SS5 thru T3SS5, are in the "Auto" position, then the daytime Light Level 5 tunnel light fixtures must remain ON as long as the outside lighting level is more than the lighting level programmed on the PLC. When the Selector Switch, T1SS5 thru T3SS5, are in the "Hand" position, then all daytime Light Level 5 light fixtures must remain ON. When the Selector Switch, T1SS5 thru T3SS5, are in the "Photocell" position, then all daytime Light Level 5 light fixtures must energize or de-energize in a manner as determined by the output of the photocell.

The PLC must be programmed to turn ON daytime tunnel lighting fixtures at the following ambient light level values:

1.	Light Level 1	30 foot candles
2.	Light Level 2	55 foot candles
3.	Light Level 3	80 foot candles
4.	Light Level 4	130 foot candles
5.	Light Level 5	180 foot candles

Once the daytime ambient light level has been reached there must be an added time delay prior to turning ON the daytime tunnel lighting fixtures. This time delay must be programmable delay initially set at 20 seconds.

While in "auto mode" and upon depressing the DPB twice in a sequence within two seconds, a password prompt should appear on the lap top screen and ask the operator to put in the pass word. Once the password is accepted, the operator must be able to select the desired dimming level of all daytime tunnel lighting levels fixtures. Once the desired level is selected, pushing the DPB once should keep this selected level in memory until changed. This selected dimming level should remain in effect as long as the auto mode switch is in the ON position.
For testing purpose, as long as the DPB is kept depressed, the operator should be able to change the dimming level of all daytime lighting fixturess. This level of dimming must remain ON only as long as the button is depressed and upon letting go the DPB, the dimming level should revert back to the original set dimming level.

The PLC software must control the day time "MANUAL lighting mode" as follow:

1. When the Selector Switches, SST1 (master selector switch) thru SST3 are in "MANUAL" position and T1SS1-5 and T3SS1-5, are in "Auto" position, then all daytime light levels tunnel light fixtures must remain ON as long as the selector switch is in MANUAL position

While in "manual mode" and upon depressing the DPB twice in a sequence within two seconds, a password prompt should appear on the lap top screen and ask the operator to put in the pass word. Once the password is accepted, the operator must be able to select the desired dimming level of all daytime tunnel lighting levels fixtures. Once the desired level is selected, pushing the DPB once should keep this selected level in memory until changed. This selected dimming level should remain in effect as long as the manual mode switch is in the ON position.
For testing purpose, as long as the DPB is kept depressed, the operator should be able to change the dimming level of all daytime lighting fixtures. This level of dimming must remain ON only as long as the button is depressed and upon letting go the DPB, the dimming level should revert back to the original set dimming level.

The PLC software must control the day time "OFF lighting mode" as follow:

1. When the Selector Switches, SST1 (master selector switch) thru SST3 are in the "OFF" position and T1SS1 thru T1SS3, are in "Auto" position, then all daytime light level tunnel light fixtures must remain OFF as long as the selector switch is in OFF position

99-16432B(6) Miscellaneous Materials

Ambient Light sensor: The ambient light sensor must be of photo diode type, Class 2, low voltage type ambient light sensor suitable for interfacing with the analog input of the programmable logic controller. The sensor must be capable of a fully adjustable ambient light response from 0 to 10,000 foot candles.

Tunnel Light Sensor: All tunnel light sensors must be of photo diode type, Class 2, low voltage type ambient light sensors suitable for interfacing with the analog input of the programmable logic controller. The sensor must be capable of a fully adjustable light response from 0 to 500 foot candles. The sensor must be enclosed inside ceiling mounted type clear glass dome.

Photoelectric Unit: Photoelectric unit, PEU, must be cadmium sulfide photoelectric control with capacity of 1800-watt incandescent or 1000-watt inductive or fluorescent load, and must have normally open contact as shown with internal controls, mounting adapter, and NEMA twist lock receptacle.

Tunnels Light Circuit Divider Control Panel: All tunnels light circuit divider control panels must be completely factory assembled type control panel wired in conformance with NEMA Class IIC wiring, Circuit divider control panel must be of NEMA 4X enclosure consisting of dual door with hasp as shown, back mounting panel, twelve branch circuit breakers and terminals block with factory made jumpers.

All circuit breaker must be 2-pole, 480-volt, AC, 30-ampere, and back panel mounted type molded case circuit breakers.

Terminal block must be tubular type, comply with NEMA ICS 4, be 30-ampere, 600-volt, molded plastic have high-pressure clamp connectors, DIN rail mounted or attached to the enclosure. Nameplates must be screwed to each block or a computer printed plastic label attached with adhesive. All conductor numbers must be identified.

Tunnels Dimming Control panel: All tunnels dimming control panel must be completely factory assembled type control panel wired in conformance with NEMA Class IIC wiring, Tunnels dimming control panel must be of NEMA 4X enclosure consisting of dual door with hasp as shown, back mounting panel, metal barrier as shown, power ON/OFF switch, power ON pilot light, transformer fuse, 1 kVA, 120/24-volt, AC rated transformer, tunnel lights fixtures dimming control module. and terminals blocks.

Power ON/OFF switch must be 1-pole, 15-ampere, 120/277-volt, AC, rated back panel mounted type molded case circuit breaker.

Pilot light must be panel-mounted, push to test type, heavy duty, plastic screw on lens, LED lamp pilot light of the voltage and color as shown.

Fuse must be 1-pole, barrier type, and 120-volt, AC, rated fuse sized for protecting primary of the transformer.

Transformer must be indoor, dry type, 1-phase, 120-volt primary, 24-volt secondary, and 1-kVA rated transformer.

Dimming control module must be rated for input of 24-volt, AC, completely solid state and microprocessor based type converter capable of converting 4-20 milliampere light intensity (converted to 2-10 volts at the input of the dimming control module as shown) signal from the PLC controller to 0-10 volts proportional output for controlling the light output intensity of the tunnel light fixture as shown. Dimming control modules' input and output circuits must be optically isolated for fast and quick response. In addition, dimming control module must have module configuration dipswitches and must be suitable for accepting pulse width modulation type input and capable of providing output for light output feedback and set point feedback in milliamperes. Each dimming control module must have minimum of four output channels for providing 0-10 volts output proportional to 2-10 volts input. Each output channel must be sized to handle the electrical burden of tunnel light fixture dimming drivers of at least forty tunnel light fixtures.

Nameplates: Nameplates must be laminated phenolic plastic with white core and black front and back. Nameplate inscription must be in capitals letters etched through the outer layer of the nameplate material.

Warning Plates: Warning plates must be laminated phenolic plastic with white core and red front and back. Warning plates inscription must be in capital letters etched through the outer layer of the nameplate material.

99-16432C Construction

99-16432C(1) Fabrication

Panelboard Cabinet:

All panelboard cabinet T1, T2, and T3 must be wired with bused type current transformer and digital multimeter on top and main circuit breaker and branch circuit breaker at the bottom.

All wires within the panelboard must be neatly trained and bundled. A typewritten directory under transparent protective cover must be installed and set in metal frame inside each cabinet door. Directory panel designation for each circuit breaker must include complete information concerning equipment supplied.

Where "Future" or “Space" is shown, branch connectors, mounting brackets, and other hardware must be furnished and installed for future breaker.

Lighting Contactors Cabinet:

All lighting contactor cabinet LC-T1, LC-T2, and LC-T3 must be factory prewired using red colored insulation conductors for general wiring and white colored insulation for neutrals of 120 V system. Use of gray colored insulation conductors for wiring un-grounded conductor is prohibited.

All ungrounded circuit conductors proceeding towards the Tunnel Light Circuit Divider Panels must be black color insulation conductors identified at both ends with wire Identification. All wires entering the enclosure must terminate on terminal blocks. Wires must be neatly trained and bundled, and wiring troughs must be provided in the enclosure as necessary. Wiring must be arranged so that any piece of apparatus may be removed without disconnecting any wires except the leads to that piece of apparatus.

No equipment or device must be mounted on the side or at the bottom of the panel. A minimum of 8 inches of empty space must be provided at the bottom of the panel for bundling and terminating field conductors.

A wiring diagram encased between 2 heat fused laminated plastic sheets must be provided with brass mounting eyelets attached to the inside of the control stations.

PLC Control Cabinet:

The PLC control cabinet must be factory prewired in conformance with NEMA Class IIC wiring. All wires entering the enclosure must terminate on terminal blocks. Power distribution type terminal blocks must be installed as required for distributing power to various power devices. Control wiring must be 7 strand No. 14 MTW except for hinge wiring, which must be 19 strand No. 14 MTW.

The PLC control cabinet must be wired using red colored insulation conductors for general wiring and white colored insulation for neutrals of 120 V system. Use of gray colored insulation conductors for wiring un-grounded conductor is prohibited.

Wires must be neatly trained and bundled, and wiring troughs must be provided in the enclosure as necessary. Wiring must be arranged so that any piece of apparatus may be removed without disconnecting any wires except the leads to that piece of apparatus.

No equipment or device must be mounted on the side or at the bottom of the panel. A minimum of 8 inches of empty space must be provided at the bottom of the panel for bundling field conductors and terminating field

A wiring diagram encased between 2 heat fused laminated plastic sheets must be provided with brass mounting eyelets attached to the inside of the control stations.

99-16432C(2) Installation

Tunnels control equipment must be installed and anchored to the floor and walls utilizing specified anchors as shown. All field conductors must be terminated on terminals blocks per authorized shop submittals.

Equipment Identification:

Equipment must be identified with nameplates fastened with self-tapping, cadmium-plated screws or nickel-plated bolts.

99-16432C(3) Testing

Operational Testing: The operation test for the entire tunnels control equipment inclusive of all dimming control panels and all tunnel lighting system must be performed by the system integrator. Duration of the operational test must be for at least 1 week period under real time conditions at the site. The operational tests must demonstrate that all functions of the system operate in the manner described in the manufacturer's literature and that the system is stable under normal vibration and shocks to components. Notify the Engineer in writing not less than 10 days in advance of performing the operational tests. The system integrator must certify in writing that the equipment has been installed, adjusted and tested in accordance with the manufacturer recommendation.

99-16432C(4) Training

Provide four hours of on-site training on the use, operation, and maintenance of each system for not more than 8 designated Department employees for 4 hours. Notify the Engineer in writing not less than 10 days in advance of proposed training class. The training for the complete tunnels control system must be performed by the system integrator.

99-16432D Payment

Not Used

99-16500 LIGHTING

99-16500A General

99-16500A(1) Summary

Scope: This work consists of furnishing, installing and connecting all lighting equipment.

99-16500A(2) Definitions

Not Used

99-16500A(3) Submittals

Not Used

99-16500A(4) Quality Control and Assurance

Not Used

99-16500B Materials

99-16500B(1) General

Lighting Fixture Lamps: Lighting fixture lamps must be LED type and size as shown.

Lighting Fixtures: Lighting fixtures must be as shown. Outdoor luminaires must be listed and labeled "Fixture Suitable For Wet Locations."

L1: Wall mounted outdoor type LED fixture with maximum 70 watt LED array, type 3 medium distribution, full cutoff fixture, 6500 minimum delivered lumens at 4000 degrees Kelvin and with CRI of at least 80. The fixture must be constructed from die-cast, rugged aluminum with integral, watertight LED engine driver compartment, and high performance aluminum heat sinks. All exposed die-cast surfaces must have an epoxy primer to provide resistance to corrosion and white powder topcoat. LED engine driver must be UL Listed for wet locations, 120 volt, 50,000 hour design life, 0.9 minimum power factor, 5-year warranty, complete with internal thermal protection and overload protection and must have a surge protection device meeting a minimum 10kA surge (per ANSI C62.41.2). Fixture must be UL listed for wet location and IP66 rated. The fixture must be provided with a NEMA twist lock receptacle for mounting the photocell. Provide compatible photocell for each fixture. Integral motion sensors must be factory installed on the fixture housing and use passive infrared technology to detect motion. Motion sensor coverage must be equal or greater than 2 mounting height diameter coverage with a 180 degree pattern. Motion sensor controls must dim the fixture light output by 50 percent when there is no motion detected. Fixture must be Gardco, 121 series; Lithonia, D Series Type 2; or equal with required options for LED engine wattage, LED drive current, LED color temperature, light distribution, voltage, finish, motion sensor and photocontroller.

L2: Nominal 1 foot by 4 foot low profile LED light with extruded acrylic lens, maximum 50 watt LED engine, 4700 minimum delivered lumens at 4000 degrees Kelvin and with CRI of at least 80. The fixture must be surface or pendant mount type, constructed from one piece of cold rolled steel coated polyester, painted white, complete with end plates. Fixture must be designed for lifetime of minimum 50,000 hours, it must have linear faceted reflector cavity, sloped end plates and advanced optical film. AC LED engine driver must be UL Listed damp location, 120 volt, 50,000 hour design life, 0-10 volt dimming, 0.9 minimum power factor, 5-year warranty, complete with internal thermal protection and overload protection. Fixture must come with mounting adapters and hardware required for surface mounting to gypboard and pendant mounting. Fixture must be Lithonia Lighting, STL series; CREE, CR-LE series; or equal.

99-16500B(2) Fabrication

Not Used

99-16500C Construction

Lighting Fixtures:

Lighting fixtures must be mounted securely to comply with the manufacturer's instructions. Mounting methods must be suitable for the particular type of ceiling or support at each location.

The Contractor must provide all supports, hangers, spacers, channels, fasteners and other hardware necessary to support the fixtures.

Fixtures must be set at the mounting heights shown, except heights shown must be adjusted to meet conditions.

99-16500D Payment

Not Used

99-16560 Temporary Lighting

99-16560A General

99-16560A(1) Summary

Scope: This work includes temporary lighting design, providing and installing temporary lighting, temporary lighting power distribution equipment, hangers, supports, fasteners, cables, and standby power production equipment for the temporary lighting inside all three tunnels.

99-16560A(1)(a) Design Requirements

Provide and install temporary lighting for all three tunnels during the duration of construction and until new lighting is completed and are operating as designed. The temporary lighting must be designed to provide 20 foot candle of maintained illumination on the roadway surface during day time and 5 foot candle on night time. Provide, install, maintain, and remove temporary power distribution equipment and temporary power source for powering the temporary lighting.

99-16560A(2) Definitions

Not Used

99-16560A(3) Submittals

Provide complete design and installation details for authorization to the Engineer prior to installation of the temporary lighting.

99-16560A(4) Quality Control and Assurance

Not Used.

99-16560B Materials

Not Used

99-16560C Construction

Temporary lighting system assemblies must be hung from the tunnel soffit with J hooks. The temporary lighting fixtures and related accessories must not hung below the height of 14 1/2 feet above the travel surface. The temporary lighting must be installed at a reasonable safe clearance from the active travel way. The temporary lighting system must be kept in fully operational and working condition during the duration of construction as described above.

The standby power and distribution equipment for the temporary lighting should be located at a safe place as directed by the Engineer. At your option , the existing power source (480-volt, 3-phase, 3-wire) that presently supplies existing tunnels lighting could be utilized as power source for supplying temporary lighting. The availability of this existing power source is available at the exit portal of each tunnel.

99-16560D Payment

Not Used.

99-16570 Tunnels Lighting System

99-16570A General

99-16570A(1) Summary

Scope: This work includes installing tunnel lighting fixtures, miscellaneous electrical equipment, construction channels, supports, and fasteners for the tunnel lighting system inside all three tunnels as shown.

99-16570A(2) Definitions

Not Used

99-16570A(3) Submittals

Submit a list of all materials and equipment to be installed and the manufacturer's descriptive data for authorization. Manufacturer's descriptive information, photometric curves, catalog cuts, and installation instructions must be submitted for authorization.

System layouts shown are generally diagrammatic and location of equipment is approximate. Exact locations of lights, routing of conduits and other facilities and location of equipment is to be governed by the design, structural conditions and other physical obstructions inside the tunnel, and must be coordinated in advance of the beginning of work. Equipment requiring maintenance and inspection must be located where it is readily accessible for the performance of such maintenance and inspection.

Submit shop drawings for authorization. Shop drawings must show the shape, size, and method of attachment for each component used in the work. Submit control and wiring diagrams that include rough-in dimensions, component layout and conductor identifications.

99-16570A(4) Quality Control and Assurance

Comply with Section 86-1.02 for Codes and regulations.

Tunnel lighting system has been designed in compliance with ANSI/IESNA RP22 "American National Standard Practice for Tunnel Lighting.

99-16570B Materials

Tunnel Light Fixture:

The tunnel light fixture must be 24 by 17-1/2 inches in size, specifically manufactured for tunnel type application, be made of low copper content die cast A360 aluminum alloy housing containing electrical components and an optical system with photometrics. Die cast aluminum housing must have integral heat sink fins to optimize thermal management through conductive and convective cooling. The light fixture door must be fully gasketed and latched type (with minimum of six latches) door capable of withstanding 100 psi of water pressure for washing of the light fixture and with at least two internal hinges at the bottom of the door for easy access to the internal components such as light emitting diodes (LED), three LED drivers, surge protective devices, light fixture quick disconnect plug and low and high voltage terminal blocks. The light fixture door lens must be fully gasketed and borosilicate, heat treated, clear prismatic glass lens secured to the fixture housing with screws. The light fixtures must be designed to withstand vibrations rated for 3G applications per ANSIC136.31.

The light fixture must be IP 66 rated light fixture per IEC 60529 standards and have super durable thermoset powder coat finish over anodized aluminum pre-finish. Powder coating must meet AAMA-2605. The light fixtures gray finish must have factory tested for and had passed 5000 hours salt fog test per ASTMB117 and D1654. The light fixture must be wall mounted type fixture equipped with two 316 grade stainless steel type heavyduty brackets and be suitable for continuous row type mounting as shown. Light fixture must have three (one on each side and one at the top of the light fixture). 3/4-inch 14 NPT, threaded entries for connecting with threaded conduit raceway. System.

Light fixture must be rated for 480-volt input, 365-watt, have total of 9 LED arranged in three rows, must deliver minimum of 30,500 lumens output, three Class 1 LED drivers rated for 100,000 hours life, double fusing, quick disconnect type connectors for ease of installation and maintenance, and a surge protection device rated for 1 kV/5 kA and complying with ANSI/IEEEC62.41. LEDs must be constructed of multiple chip on board technology with color temperature of 4000 degree K and a color rendering index of 70 or greater. All LED drivers power factor must be 90 percent or greater. LED driver must meet or exceeds maximum total harmonic distortion (THD) of 20 percent. LED driver must be thermally managed to have a minimum rated driver life of 100,000 hours at 25 degrees C ambient. All LED drivers must be a dimming type drivers suitable for accepting 0-10 volts control signal from the Dimming Control Panel for controlling the intensity of the light fixtures' light output ranging between 10 to 100 percent. There must be separate terminals block at the top for terminating incoming as well as outgoing dimming control conductors. LED drivers must be factory programmed in a manner so that in the event of signal failure, light fixtures must lit at their maximum light intensity.

Light fixtures must have 4 segmented and mirrored type internal reflectors designed for superior optical control in Crossbeam type light distribution pattern.

The tunnel light fixture must be safety listed as UL 1598 at 40 degrees C in wet location and 30 degrees C when use as raceway. The tunnel light fixture must comply with the following standards:

1. IEC 61000 - Electromagnetic Compatibility Test (EMC)
2. ANSI/IEEE C62.41.2 Surge Protection
3. IEEE 519 -Harmonic control in Electrical Power systems
4. ANSIC82.77 - Harmonic distortion
5. ANSIC136.31- Luminaire vibration
6. ASTM B 117 – Salt spray test
7. IEC 60529 - Degrees of protection provided by enclosure (IP)

99-16570C Construction

99-16570C(1) Installation

Tunnel lighting fixtures must be wall mounted securely in accordance as shown. Provide all supports, hangers, spacers, channels, fasteners and other hardware necessary to support the lighting fixtures.

Tunnel lighting fixtures must be set at the mounting heights shown, except heights shown may be adjusted to meet physical conditions and as directed by the Engineer.

99-16570C(2) Technical Support

The manufacturer of the lighting fixture must provide technical assistance and guidance in the operation, maintenance and trouble shooting of operational problems for the system for one year following the acceptance of the Contract. The technical support must be provided at no additional cost to the Department.

Technical support must be provided at the facility site by an authorized representative of the lighting controller manufacturer and by a toll free telephone service to the manufacturer.

Warranty: A five year complete parts and labor re[placement factory warranty after the date of acceptance of the construction contract must be provided for the tunnel light fixtures.

99-16570D Payment

Not Used.

99-16580 AIR CONDITIONER (WALL MOUNTED)

99-16580A General

99-16580A(1) Summary

Scope: This work consists of installing and testing air conditioning equipment.

The performance rating and electrical service of the equipment must be as shown.

Equipment and systems must comply with California Energy Commission regulations including the California Building Energy Efficiency Standards and the Appliance Efficiency Regulations and, where applicable, must comply with standards of the Air-Conditioning, Heating, and Refrigeration Institute (AHRI), Sheet Metal and Air Conditioning Contractors National Association, Inc. (SMACNA), and Air Movement and Control Association International (AMCA).

Cooling and refrigeration equipment and components must be certified by AHRI for the performance rating shown, under the AHRI or ARI rating systems.

Safety: Equipment must be certified compliant with UL 1995 or with ASHRAE 15, NFPA 90A, and NFPA 90B.

Work must include furnishing all labor, materials, equipment and services required for providing air conditioning equipment.

Electrical, and such other work incidental and necessary to the installation and operation of the mechanical work must comply with the requirements described for similar type work elsewhere.

99-16580A(2) Definitions

Not Used

99-16580A(3) Submittals

Product Data:

A list of materials and equipment to be installed, manufacturer's descriptive data, and such other data as may be requested by the Engineer must be submitted.

Manufacturer's descriptive data must include complete description, performance data, and installation instructions for the materials and equipment described.

Manufacturer's descriptive data must be submitted for the Wall mounted air conditioner

Submit product literature and installation instructions for all products including ductwork and accessories. Include energy efficiency ratio (EER) and seasonal energy efficiency ratio (SEER) for cooling equipmentand type and quantity of refrigerant for each cooling unit.

For heaters, air conditioners, economizers, fans, dampers, and duct layout on full size sheets, drawn at same scale as the plans or larger scale as needed for clarity, but not less than 1/4 inch scale.

99-16580A(4) Closeout Submittals

Operation and Maintenance Manuals:

Prior to the completion of the contract, submit 3 identified copies of the operation and maintenance instructions with parts lists for the equipment used. The instructions and parts lists must be indexed and bound in a manual form and must be complete and adequate for the equipment installed. Inadequate or incomplete material must be returned. Resubmit adequate and complete manuals.

Operation and maintenance manuals must be submitted for the Wall mounted air conditioner

99-16580A(5) Quality Control and Assurance

Codes and Standards: Mechanical work, including equipment, materials and installation, must comply with the CBC: CMC; CPC; CEC; the California Building Energy Efficiency Standards; and 8 CA Code of Regs Ch 4, Division of Industrial Safety (DIS).

99-16580A(6) Warranty

Warranties and Guarantees: Manufacturer's warranties and guarantees for materials or equipment used in the work must be delivered to the Engineer at the job site prior to acceptance of the contract.

99-16580B Materials

Air Conditioner (Wall Mounted): Air conditioner (wall mounted) must be through-the-wall type, cooling only, rotary type compressor, and must include slide-out chassis design, thermostat, adjustable discharge grilles, multi-speed fan, integral thermal overload protection.

99-16580C Field Quality Control

Pre-test Requirements:

Before starting or operating systems, equipment must be cleaned and checked for proper installation, lubrication and servicing.

Replace or revise any equipment, systems or work found deficient during tests.

Project Completion Tests:

The Engineer must be notified at least 3 business days in advance of starting project completion tests.

Upon completion of mechanical work and pre-test requirements, or at such time prior to completion as determined by the Engineer, the Contractor must operate and test installed mechanical systems for at least 3 consecutive 8-hour days to demonstrate satisfactory overall operation.

99-16580D Payment

Not Used

REVISED STANDARD SPECIFICATIONS APPLICABLE TO THE 2010 EDITION OF THE STANDARD SPECIFICATIONS

REVISED STANDARD SPECIFICATIONS DATED 06-06-14

ORGANIZATION

Revised standard specifications are under headings that correspond with the main-section headings of the *Standard Specifications*. A main-section heading is a heading shown in the table of contents of the *Standard Specifications*. A date under a main-section heading is the date of the latest revision to the section.

Each revision to the *Standard Specifications* begins with a revision clause that describes or introduces a revision to the *Standard Specifications*. For a revision clause that describes a revision, the date on the right above the clause is the publication date of the revision. For a revision clause that introduces a revision, the date on the right above a revised term, phrase, clause, paragraph, or section is the publication date of the revised term, phrase, clause, paragraph, or section. For a multiple-paragraph or multiple-section revision, the date on the right above a paragraph or section is the publication date of the paragraphs or sections that follow.

Any paragraph added or deleted by a revision clause does not change the paragraph numbering of the *Standard Specifications* for any other reference to a paragraph of the *Standard Specifications*.

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Add to the 1st table in section 1-1.06:

04-19-13

LCS	Department's lane closure system
POC	pedestrian overcrossing
QSD	qualified SWPPP developer
QSP	qualified SWPPP practitioner
TRO	time-related overhead
WPC	water pollution control

06-20-12

Delete the abbreviation and its meaning for *UDBE* in the 1st table of section 1-1.06.

10-19-12

Delete "Contract completion date" and its definition in section 1-1.07B.

10-19-12

Delete "critical delay" and its definition in section 1-1.07B.

Replace "day" and its definition in section 1-1.07B with:

10-19-12

day: 24 consecutive hours running from midnight to midnight; calendar day.

1. **business day:** Day on the calendar except a Saturday and a holiday.
2. **working day:** Time measure unit for work progress. A working day is any 24-consecutive-hour period except:
   - 2.1. Saturday and holiday.
   - 2.2. Day during which you cannot perform work on the controlling activity for at least 50 percent of the scheduled work shift with at least 50 percent of the scheduled labor and equipment due to any of the following:
     - 2.2.1. Adverse weather-related conditions.
     - 2.2.2. Maintaining traffic under the Contract.
     - 2.2.3. Suspension of a controlling activity that you and the Engineer agree benefits both parties.
     - 2.2.4. Unanticipated event not caused by either party such as:
       - 2.2.4.1. Act of God.
       - 2.2.4.2. Act of a public enemy.
       - 2.2.4.3. Epidemic.
       - 2.2.4.4. Fire.
       - 2.2.4.5. Flood.
       - 2.2.4.6. Governor-declared state of emergency.
       - 2.2.4.7. Landslide.
       - 2.2.4.8. Quarantine restriction.
     - 2.2.5. Issue involving a third party, including:
       - 2.2.5.1. Industry or area-wide labor strike.
       - 2.2.5.2. Material shortage.
       - 2.2.5.3. Freight embargo.
       - 2.2.5.4. Jurisdictional requirement of a law enforcement agency.
       - 2.2.5.5. Workforce labor dispute of a utility or nonhighway facility owner resulting in a nonhighway facility rearrangement not described and not solely for the Contractor's convenience. Rearrangement of a nonhighway facility includes installation, relocation, alteration, or removal of the facility.
   - 2.3. Day during a concurrent delay.
3. **original working days:**

3.1. Working days to complete the work shown on the *Notice to Bidders* for a non–cost plus time based bid.
3.2. Working days bid to complete the work for a cost plus time based bid.

Where working days is specified without the modifier "original" in the context of the number of working days to complete the work, interpret the number as the number of original working days as adjusted by any time adjustment.

Replace "Contract" in the definition of "early completion time" in section 1-1.07B with:

10-19-12

work

Replace "excusable delay" and its definition in section 1-1.07B with:

10-19-12

delay: Event that extends the completion of an activity.

1. **excusable delay:** Delay caused by the Department and not reasonably foreseeable when the work began such as:
   1.1. Change in the work
   1.2. Department action that is not part of the Contract
   1.3. Presence of an underground utility main not described in the Contract or in a location substantially different from that specified
   1.4. Described facility rearrangement not rearranged as described, by the utility owner by the date specified, unless the rearrangement is solely for the Contractor's convenience
   1.5. Department's failure to obtain timely access to the right-of-way
   1.6. Department's failure to review a submittal or provide notification in the time specified
2. **critical delay:** Excusable delay that extends the scheduled completion date
3. **concurrent delay:** Occurrence of at least 2 of the following events in the same period of time, either partially or entirely:
   3.1. Critical delay
   3.2. Delay to a controlling activity caused by you
   3.3. Non–working day

Replace "project" in the definition of "scheduled completion date" in section 1-1.07B with:

10-19-12

work

Add to section 1-1.07B:

10-19-12

Contract time: Number of original working days as adjusted by any time adjustment.

06-20-12

Disadvantaged Business Enterprise: Disadvantaged Business Enterprise as defined in 49 CFR 26.5.

Replace "PO BOX 911" in the District 3 mailing address in the table in section 1-1.08 with:

04-20-12

703 B ST

Replace the Web site for the Department of General Services, Office of Small Business and DVBE Services in the table in section 1-1.11 with:

11-15-13

http://www.dgs.ca.gov/dgs/ProgramsServices/BusServices.aspx

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2 BIDDING

06-06-14

Replace the headings and paragraphs in section 2 with:

02-21-14

2-1.01 GENERAL

Section 2 includes specifications related to bid eligibility and the bidding process.

The electronic bid specifications in section 2 apply if *Electronic Bidding Contract* is shown on the cover of the *Notice to Bidders and Special Provisions*.

2-1.02 BID INELIGIBILITY

A firm that has provided architectural or engineering services to the Department for this contract before bid submittal for this contract is prohibited from any of the following:

1. Submitting a bid
2. Subcontracting for a part of the work
3. Supplying materials

2-1.03–2-1.05 RESERVED

2-1.06 BID DOCUMENTS

2-1.06A General

Standard Specifications and *Standard Plans* may be viewed at the Bidders' Exchange website and may be purchased at the Publication Distribution Unit.

The *Notice to Bidders and Special Provisions* and project plans may be viewed at the Bidders' Exchange website and at the street address.

Bid books may be ordered at the Bidders' Exchange website.

For an informal-bid contract, in addition to viewing and ordering them as specified above, the *Notice to Bidders and Special Provisions*, project plans, and *Bid* book may be obtained at the Bidders' Exchange street address.

The *Notice to Bidders and Special Provisions* includes the *Notice to Bidders*, revised standard specifications, and special provisions.

2-1.06B Supplemental Project Information

The Department makes supplemental information available as specified in the special provisions.

Logs of test borings are supplemental project information.

If an *Information Handout* or cross sections are available:

1. You may view them at the Contract Plans and Special Provisions link at the Bidders' Exchange website
2. For an informal-bid contract, you may obtain them at the Bidders' Exchange street address

If rock cores are available, you may view them by sending a request to Coreroom@dot.ca.gov.

If other supplemental project information is available for inspection, you may view it by phoning in a request.

Make your request at least 7 days before viewing. Include in your request:

1. District-County-Route
2. Contract number
3. Viewing date
4. Contact information, including telephone number

For rock cores, also include the bridge number in your request.

If bridge as-built drawings are available:

1. For a project in District 1 through 6 or 10, you may request them from the Office of Structure Maintenance and Investigations, fax (916) 227-8357
2. For a project in District 7, 8, 9, 11, or 12, you may request them from the Office of Structure Maintenance and Investigations, fax (916) 227-8357, and they are available at the Office of Structure Maintenance and Investigations, Los Angeles, CA, telephone (213) 897-0877

As-built drawings may not show existing dimensions and conditions. Where new construction dimensions are dependent on existing bridge dimensions, verify the field dimensions and adjust dimensions of the work to fit existing conditions.

2-1.06C–2-1.06D Reserved

2-1.07 JOB SITE AND DOCUMENT EXAMINATION

Examine the job site and bid documents.

Bid submission is your acknowledgment that you have examined the job site and bid documents and are satisfied with:

1. General and local conditions to be encountered
2. Character, quality, and scope of work to be performed
3. Quantities of materials to be furnished
4. Character, quality, and quantity of surface and subsurface materials or obstacles
5. Requirements of the contract

2-1.08 RESERVED

2-1.09 BID ITEM LIST

Submit a bid based on the bid item quantities the Department shows on the Bid Item List.

06-06-14

2-1.10 SUBCONTRACTOR LIST

On the Subcontractor List form, list each subcontractor to perform work in an amount in excess of 1/2 of 1 percent of the total bid or $10,000, whichever is greater (Pub Cont Code § 4100 et seq.).

For each subcontractor listed, the Subcontractor List form must show:

1. Business name and the location of its place of business.
2. For a non-federal-aid contract, its California contractor license number.
3. Portion of work it will perform. Show the portion of the work by:
   - 3.1. Description of portion of subcontracted work
   - 3.2. Bid item numbers for the work involved in the portion of work listed
   - 3.3. Percentage of the portion of work in each bid item listed

02-21-14

2-1.11 RESERVED

2-1.12 DISADVANTAGED BUSINESS ENTERPRISES

2-1.12A General

Section 2-1.12 applies to a federal-aid contract.

Under 49 CFR 26.13(b):

> The contractor, sub recipient or subcontractor shall not discriminate on the basis of race, color, national origin, or sex in the performance of this contract. The contractor shall carry out applicable requirements of 49 CFR part 26 in the award and administration of DOT-assisted contracts. Failure by the contractor to carry out these requirements is a material breach of this contract, which may result in the termination of this contract or such other remedy as the recipient deems appropriate.

Take necessary and reasonable steps to ensure that DBEs have opportunity to participate in the Contract (49 CFR 26).

2-1.12B Disadvantaged Business Enterprise Goal

2-1.12B(1) General

Section 2-1.12B applies if a DBE goal is shown on the *Notice to Bidders*.

To ensure equal participation of DBEs provided in 49 CFR 26.5, the Department shows a goal for DBEs.

Make work available to DBEs and select work parts consistent with available DBE subcontractors and suppliers.

Meet the DBE goal shown on the *Notice to Bidders* or demonstrate that you made adequate good faith efforts to meet this goal.

You are responsible to verify that the at the bid opening date the DBE firm is certified as DBE by the CA Unified Certification Program.

All DBE participation will count toward the Department's federally-mandated statewide overall DBE goal.

Credit for materials or supplies you purchase from DBEs counts toward the goal in the following manner:

1. 100 percent if the materials or supplies are obtained from a DBE manufacturer.
2. 60 percent if the materials or supplies are obtained from a DBE regular dealer.
3. Only fees, commissions, and charges for assistance in the procurement and delivery of materials or supplies, if they are obtained from a DBE that is neither a manufacturer nor regular dealer. 49 CFR 26.55 defines "manufacturer" and "regular dealer."

You receive credit toward the goal if you employ a DBE trucking company that performs a commercially useful function as defined in 49 CFR 26.55(d)(1)–(4), (6).

2-1.12B(2) DBE Commitment Submittal

Submit DBE information under section 2-1.33.

Bidders other than the apparent low bidder, the 2nd low bidder, and the 3rd low bidder are not required to submit the DBE commitment form unless the Department requests it. If the Department requests a DBE commitment form from you, submit the completed form within 4 business days of the request.

Submit written confirmation from each DBE shown on the form stating that it will be participating in the Contract. Include confirmation with the DBE commitment form. A copy of a DBE's quote will serve as written confirmation that the DBE will be participating in the Contract.

2-1.12B(3) Good Faith Efforts Submittal

If you have not met the DBE goal, complete and submit the Good Faith Efforts Documentation under section 2-1.33 showing that you made adequate good faith efforts to meet the goal. Only good faith efforts directed toward obtaining participation by DBEs are considered.

If your DBE commitment form shows that you have met the DBE goal or if you are required to submit the DBE commitment form, you must submit good faith efforts documentation within the specified time to protect your eligibility for award of the contract in the event the Department finds that the DBE goal has not been met.

The Department may consider DBE commitments of the 2nd and 3rd bidders in determining whether the low bidder made good faith efforts to meet the DBE goal.

2-1.13–2-1.14 RESERVED

2-1.15 DISABLED VETERAN BUSINESS ENTERPRISES

2-1.15A General

Section 2-1.15 applies to a non-federal-aid contract.

Take necessary and reasonable steps to ensure that DVBEs have opportunity to participate in the Contract.

Comply with Mil & Vet Code § 999 et seq.

2-1.15B Projects $5 Million or Less

Section 2-1.15B applies to a project with an estimated cost of $5 million or less.

Make work available to DVBEs and select work parts consistent with available DVBE subcontractors and suppliers.

Meet the goal shown on the *Notice to Bidders*.

Complete and submit the Certified DVBE Summary form under section 2-1.33. List all DVBE participation on this form.

If a DVBE joint venture is used, submit the joint venture agreement with the Certified DVBE Summary form.

List each 1st-tier DVBE subcontractor on the Subcontractor List form regardless of percentage of the total bid.

2-1.15C Projects More Than $5 Million

2-1.15C(1) General

Section 2-1.15C applies to a project with an estimated cost of more than $5 million.

The Department encourages bidders to obtain DVBE participation to ensure the Department achieves its State-mandated overall DVBE goal.

If you obtain DVBE participation:

1. Complete and submit the Certified DVBE Summary form under section 2-1.33. List all DVBE participation on this form.
2. List each 1st tier DVBE subcontractor in the Subcontractor List form regardless of percentage of the total bid.

If a DVBE joint venture is used, submit the joint venture agreement with the Certified DVBE Summary form.

2-1.15C(2) DVBE Incentive

The Department grants a DVBE incentive to each bidder who achieves a DVBE participation of 1 percent or greater (Mil & Vet Code 999.5 and Code of Regs § 1896.98 et seq.).

To receive this incentive, submit the Certified DVBE Summary form under section 2-1.33.

Bidders other than the apparent low bidder, the 2nd low bidder, and the 3rd low bidder may be required to submit the Certified DVBE Summary form if the bid ranking changes. If the Department requests a Certified DVBE Summary form from you, submit the completed form within 4 business days of the request.

2-1.15C(3) Incentive Evaluation

The Department applies the small business and non–small business preference during bid verification and proceeds with the evaluation specified below for DVBE incentive.

The DVBE incentive is a reduction, for bid comparison only, in the total bid submitted by the lesser of the following amounts:

1. Percentage of DVBE achievement rounded to 2 decimal places of the verified total bid of the low bidder
2. 5 percent of the verified total bid of the low bidder
3. $250,000

The Department applies DVBE incentive and determines whether bid ranking changes.

A non–small business bidder cannot displace a small business bidder. However, a small business bidder with higher DVBE achievement can displace another small business bidder.

The Department proceeds with awarding the contract to the new low bidder and posts the new verified bid results at the Department's Web site.

2-1.16–2-1.17 RESERVED

2-1.18 SMALL BUSINESS AND NON–SMALL BUSINESS SUBCONTRACTOR PREFERENCES

2-1.18A General

Section 2-1.18 applies to a non-federal-aid contract.

The Department applies small business preferences and non–small business preferences under Govt Code § 14835 et seq. and 2 CA Code of Regs § 1896 et seq.

Any contractor, subcontractor, supplier, or service provider who qualifies as a small business is encouraged to apply for certification as a small business by submitting its application to the Department of General Services, Office of Small Business and DVBE Services.

Contract award is based on the total bid, not the reduced bid.

2-1.18B Small Business Preference

The Department allows a bidder certified as a small business by the Department of General Services, Office of Small Business and DVBE Services, a preference if:

1. Bidder submitted a completed Request for Small Business Preference or Non–Small Business Preference form with its bid
2. Low bidder did not request the preference or is not certified as a small business

The bidder's signature on the Request for Small Business Preference or Non–Small Business Preference form certifies that the bidder is certified as a small business at the date and time of bid or has submitted a complete application to the Department of General Services. The complete application and any required substantiating documentation must be received by the Department of General Services by 5:00 p.m. on the bid opening date.

The Department of General Services determines whether a bidder was certified on the bid opening date. The Department of Transportation confirms the bidder's status as a small business before applying the small business preference.

The small business preference is a reduction for bid comparison in the total bid submitted by the small business contractor by the lesser of the following amounts:

1. 5 percent of the verified total bid of the low bidder
2. $50,000

If the Department determines that a certified small business bidder is the low bidder after the application of the small business preference, the Department does not consider a request for non–small business preference.

2-1.18C Non–Small Business Subcontractor Preference

The Department allows a bidder not certified as a small business by the Department of General Services, Office of Small Business and DVBE Services, a preference if:

1. Bidder submitted a completed Request for Small Business Preference or Non–Small Business Preference form with its bid

2. Certified Small Business Listing for the Non–Small Business Preference form shows that you are subcontracting at least 25 percent to certified small businesses

Each listed subcontractor and supplier must be certified as a small business at the date and time of bid or must have submitted a complete application to the Department of General Services. The complete application and any required substantiating documentation must be received by the Department of General Services by 5:00 p.m. on the bid opening date.

The non–small business subcontractor preference is a reduction for bid comparison in the total bid submitted by the non–small business contractor requesting the preference by the lesser of the following amounts:

1. 5 percent of the verified total bid of the low bidder
2. $50,000

2-1.19–2-1.26 RESERVED

2-1.27 CALIFORNIA COMPANIES

Section 2-1.27 applies to a non-federal-aid contract.

Under Pub Cont Code § 6107, the Department gives preference to a "California company," as defined, for bid comparison purposes over a nonresident contractor from any state that gives or requires a preference to be given to contractors from that state on its public entity construction contracts.

Complete a California Company Preference form.

The California company reciprocal preference amount is equal to the preference amount applied by the state of the nonresident contractor with the lowest responsive bid unless the California company is eligible for a small business preference or a non–small business subcontractor preference, in which case the preference amount is the greater of the two, but not both.

If the low bidder is not a California company and a California company's bid with reciprocal preference is equal to or less than the lowest bid, the Department awards the contract to the California company on the basis of its total bid.

2-1.28 RESERVED

2-1.29 OPT OUT OF PAYMENT ADJUSTMENTS FOR PRICE INDEX FLUCTUATIONS

You may opt out of the payment adjustments for price index fluctuations specified in section 9-1.07. To opt out, submit a completed Opt Out of Payment Adjustments for Price Index Fluctuations form under section 2-1.33.

2-1.30–2-1.32 RESERVED

06-06-14

2-1.33 BID DOCUMENT COMPLETION AND SUBMITTAL

Complete forms in the *Bid* book.

For a paper bid, submit your bid:

1. Under sealed cover
2. Marked as a bid
3. Identifying the contract number and the bid opening date

For an electronic bid, complete and submit the *Bid* book under the *Electronic Bidding Guide* at the Bidders' Exchange website.

Submit the forms and form information at the times shown in the following table:

Bid Form Submittal Schedule				
Contract type	Forms to be submitted at the time of bid	Forms to be submitted no later than 24 hours after bid opening[a]	Forms to be submitted no later than 4 p.m. on the 2nd business day after bid opening[a]	Forms to be submitted no later than 4 p.m. on the 4th business day after bid opening[a]
All contracts	• Bid to the Department of Transportation • Business name and location and description of portion of subcontracted work on the Subcontractor List • Opt Out of Payment Adjustments for Price Index Fluctuations[c]	• Bid item nos. and percentage of bid item subcontracted on the Subcontractor List[b]	--	--
Non-federal-aid contracts only	• California contractor license number on the Subcontractor List • California Company Preference • Request for Small Business Preference or Non–Small Business Preference[c]	--	• Certified Small Business Listing for the Non–Small Business Preference[c]	• Certified DVBE Summary[d]
Federal-aid contracts only	• Small Business Status	--	--	• Caltrans Bidder - DBE - Commitment[e] • Good Faith Efforts Documentation - DBE[f]

[a]The forms and information may be submitted at the time of bid.
[b]If the information is not submitted at the time of bid, fax it to (916) 227-6282. This after-bid submittal does not apply to an informal-bid contract. For an informal bid contract, submit the completed form at the time of bid.
[c]Applicable only if the preference or option is chosen.
[d]Not applicable to an informal-bid contract or a project with an estimated cost of more than $5 million. For an informal bid contract, submit the completed form at the time of bid. For a project with an estimated cost of more than $5 million, applicable only if you obtain DVBE participation or you are the apparent low bidder, 2nd low bidder, or 3rd low bidder and you choose to receive the specified incentive.
[e]If not submitted at the time of bid, applicable only to the apparent low bidder, 2nd low bidder, and 3rd low bidder.
[f]Applicable only if you have not met the DBE goal.

For an electronic bid:

1. Forms to be submitted at the time of bid must be submitted as described in the *Electronic Bidding Guide* or faxed to (916) 227-6282 before the bid opening date and time.
2. Your authorized digital signature is your confirmation of and agreement to all certifications and statements contained in the *Bid* book.

3. On forms and certifications that you submit through the electronic bidding service, you agree that each form and certification where a signature is required is deemed as having your signature. On forms that you submit after bid opening, sign the forms where a signature is required in ink.

Failure to submit the forms and information as specified results in a nonresponsive bid.

If an agent other than the authorized corporation officer or a partnership member signs the bid, file a Power of Attorney with the Department either before opening bids or with the bid. Otherwise, the bid may be nonresponsive.

02-21-14

2-1.34 BIDDER'S SECURITY

Submit one of the following forms of bidder's security equal to at least 10 percent of the bid:

1. Cash
2. Cashier's check
3. Certified check
4. Signed bidder's bond by an admitted surety insurer
5. For an electronic bid, electronic bidder's bond by an admitted surety insurer submitted using an electronic registry service approved by the Department.

Submit cash, cashier’s check, certified check, or bidder's bond to the Department at the Bidders Exchange before the bid opening time.

Submit electronic bidder’s bond with the electronic bid.

If using a bidder's bond, you may use the form in the *Bid* book. If you do not use the form in the *Bid* book, use a form containing the same information.

2-1.35–2-1.39 RESERVED

2-1.40 BID WITHDRAWAL

For a paper bid:

1. An authorized agent may withdraw a bid before the bid opening date and time by submitting a written bid withdrawal request at the location where the bid was submitted. Withdrawing a bid does not prevent you from submitting a new bid.
2. After the bid opening time, you cannot withdraw a bid.

For an electronic bid:

1. Bids are not filed with the Department until the date and time of bid opening.
2. A bidder may withdraw or revise a bid after it has been submitted to the electronic bidding service if this is done before the bid opening date and time.

2-1.41–2-1.42 RESERVED

2-1.43 BID OPENING

The Department publicly opens and reads bids at the time and place shown on the *Notice to Bidders*.

2-1.44–2-1.45 RESERVED

2-1.46 DEPARTMENT'S DECISION ON BID

The Department's decision on the bid amount is final.

The Department may reject:

1. All bids
2. A nonresponsive bid

2-1.47 BID RELIEF

The Department may grant bid relief under Pub Cont Code § 5100 et seq. Submit any request for bid relief to the Office Engineer. The Relief of Bid Request form is available at the Department's website.

2-1.48 RESERVED

2-1.49 SUBMITTAL FAILURE HISTORY

The Department considers a bidder's past failure to submit documents required after bid opening in determining a bidder's responsibility.

2-1.50 BID RIGGING

Section 2-1.50 applies to a federal-aid contract.

The U.S. Department of Transportation (DOT) provides a toll-free hotline to report bid rigging activities. Use the hotline to report bid rigging, bidder collusion, and other fraudulent activities. The hotline number is (800) 424-9071. The service is available 24 hours 7 days a week and is confidential and anonymous.. The hotline is part of the DOT's effort to identify and investigate highway construction contract fraud and abuse and is operated under the direction of the DOT Inspector General.

^^

3 CONTRACT AWARD AND EXECUTION

02-21-14

Replace section 3-1.02 with:

02-21-14

3-1.02 CONSIDERATION OF BIDS

3-1.02A General

For a lump sum based bid, the Department compares bids based on the total price.

For a unit price based bid, the Department compares bids based on the sum of the item totals.

For a cost plus time based bid, the Department compares bids based on the sum of the item totals and the total bid for time.

3-1.02B Tied Bids

The Department breaks a tied bid with a coin toss except:

1. If a small business bidder and a non–small business bidder request preferences and the reductions result in a tied bid, the Department awards the contract to the small business bidder.
2. If a DVBE small business bidder and a non-DVBE small business bidder request preferences and the reduction results in a tied bid, the Department awards the contract to the DVBE small business bidder.

Add to the end of section 3-1.04:

10-19-12

You may request to extend the award period by faxing a request to (916) 227-6282 before 4:00 p.m. on the last day of the award period. If you do not make this request, after the specified award period:

1. Your bid becomes invalid
2. You are not eligible for the award of the contract

Replace the paragraph in section 3-1.11 with:

10-19-12

Complete and deliver to the Office Engineer a *Payee Data Record* when requested by the Department.

Replace section 3-1.13 with:

07-27-12

3-1.13 FORM FHWA-1273

For a federal-aid contract, form FHWA-1273 is included with the Contract form in the documents sent to the successful bidder for execution. Comply with its provisions. Interpret the training and promotion section as specified in section 7-1.11A.

Add to item 1 in the list in the 2nd paragraph of section 3-1.18:

07-27-12

, including the attached form FHWA-1273

10-19-12

Delete item 4 of the 2nd paragraph of section 3-1.18.

^^

5 CONTROL OF WORK

05-30-14

Add between "million" and ", professionally" in the 3rd paragraph of section 5-1.09A:

10-19-12

and 100 or more working days

Add to the list in the 4th paragraph of section 5-1.09A:

10-19-12

9. Considering discussing with and involving all stakeholders in evaluating potential VECPs

Add to the end of item 1.1 in the list in the 7th paragraph of section 5-1.09A:

10-19-12

, including VECPs

Replace the 1st paragraph of section 5-1.09C with:

10-19-12

For a contract with a total bid over $10 million and 100 or more working days, training in partnering skills development is required.

10-19-12

Delete the 2nd paragraph of section 5-1.09C.

Replace "at least 2 representatives" in the 5th paragraph of section 5-1.09C with:

10-19-12

field supervisory personnel

Replace the 1st and 2nd sentences in the 7th paragraph of section 5-1.13B(1) with:

06-20-12

If a DBE is decertified before completing its work, the DBE must notify you in writing of the decertification date. If a business becomes a certified DBE before completing its work, the business must notify you in writing of the certification date.

Replace "90" in the last sentence of the 7th paragraph of section 5-1.13B(1) with:

06-20-12

30

Replace "Underutilized" in "Underutilized Disadvantaged Business Enterprises" in the heading of section 5-1.13B(2) with:

06-20-12

Performance of

06-20-12

Delete *U* in *UDBE* at each occurrence in section 5-1.13B(2).

Replace the 3rd paragraph of section 5-1.13B(2) with:

06-20-12

Do not terminate or substitute a listed DBE for convenience and perform the work with your own forces or obtain materials from other sources without authorization from the Department.

Replace item 6 in the list in the 4th paragraph of section 5-1.13B(2) with:

06-20-12

6. Listed DBE is ineligible to work on the project because of suspension or debarment.

Add to the list in the 4th paragraph of section 5-1.13B(2):

06-20-12

8. Listed DBE voluntarily withdraws with written notice from the Contract.
9. Listed DBE is ineligible to receive credit for the type of work required.
10. Listed DBE owner dies or becomes disabled resulting in the inability to perform the work on the Contract.
11. Department determines other documented good cause.

Add between the 4th and 5th paragraphs of section 5-1.13B(2):

07-20-12

Notify the original DBE of your intent to use other forces or material sources and provide the reasons. Provide the DBE with 5 days to respond to your notice and advise you and the Department of the reasons why the use of other forces or sources of materials should not occur. Your request to use other forces or material sources must include:

1. 1 or more of the reasons listed in the preceding paragraph
2. Notices from you to the DBE regarding the request
3. Notices from the DBE to you regarding the request

Add between "terminated" and ", you" in the 5th paragraph of section 5-1.13B(2):

07-20-12

or substituted

Replace the paragraphs of section 5-1.13C with:

11-15-13

Section 5-1.13C applies to a non-federal-aid contract.

Use each DVBE as shown on the *Certified DVBE Summary* form unless you receive authorization from the Department for a substitution. The substitute must be another DVBE unless DVBEs are not available, in which case, you must substitute with a small business. Any authorization for a substitute is contingent upon the Department of General Services' approval of the substitute.

The requirement that DVBEs be certified by the bid opening date does not apply to DVBE substitutions after Contract award.

The Department authorizes substitutions for any of the reasons provided in 2 CA Code of Regs § 1896.73.

Include in your substitution request:

1. Copy of the written notice issued to the DVBE with proof of delivery
2. Copy of the DVBE's response to the notice
3. Name and certification number of the listed DVBE and the proposed substitute

Requests for substitutions of a listed DVBE with a small business must include documentation of the unavailability of DVBEs, including:

1. Contact with the small business/DVBE advocate from the Department and the Department of Veterans Affairs
2. Search results from the Department of General Services' website of available DVBEs
3. Communication with a DVBE community organization nearest the job site, if applicable
4. Documented communication with the DVBE and small businesses describing the work to be performed, the percentage of the total bid, the corresponding dollar amount, and the responses to the communication

The Department forwards your substitution request to the Department of General Services. The Department of General Services issues a notice of approval or denial. The Department provides you this notice.

If you fail to use a listed DVBE without an authorized substitution request, the Department issues a penalty of up to 10 percent of the dollar amount of the work of the listed DVBE.

Maintain records of subcontracts made with DVBEs. Include in the records:

1. Name and business address of each business
2. Total amount paid to each business

For the purpose of determining compliance with Pub Cont Code § 10115 et seq.:

1. Upon work completion, complete and submit *Final Report - Utilization of Disabled Veteran Business Enterprises (DVBE) State Funded Projects Only* form.
2. Upon reasonable notice and during normal business hours, permit access to its premises for the purposes of:
   2.1. Interviewing employees.
   2.2. Inspecting and copying books, records, accounts and other material that may be relevant to a matter under investigation.

Replace "Reserved" in section 5-1.20C with:

10-19-12

If the Contract includes an agreement with a railroad company, the Department makes the provisions of the agreement available in the *Information Handout* in the document titled "Railroad Relations and Insurance Requirements." Comply with the requirements in the document.

Replace section 5-1.20E with:

05-30-14

5-1.20E Water Meter Charges

Section 5-1.20E applies if a bid item for water meter charges is shown on the Bid Item List. The charges are specified in a special provision for section 5-1.20E.

The local water authority will install the water meters.

The charges by the local water authority include:

1. Furnishing and installing each water meter
2. Connecting to the local water authority's main water line, including any required hot tap or tee
3. Furnishing and installing an extension pipe from the main water line to the water meter
4. Sterilizing the extension pipe

Make arrangements and pay the charges for the installation of the water meters.

If a charge is changed at the time of installation, the Department adjusts the lump sum price based on the difference between the specified charges and the changed charges.

Replace section 5-1.20F with:

05-30-14

5-1.20F Irrigation Water Service Charges

Reserved

Add between the 2nd and 3rd paragraphs of section 5-1.23A:

10-19-12

Submit action and informational submittals to the Engineer.

Add between the 5th and 6th paragraphs of section 5-1.23B(1):

07-19-13

For a revised submittal, allow the same number of days for review as for the original submittal.

07-19-13

Delete the 1st sentence in the 10th paragraph of section 5-1.23B(2).

Add to the list in the 1st paragraph of section 5-1.36A:

07-19-13

10. Survey monuments

Add to section 5-1.36C:

07-20-12

If the Contract does not include an agreement with a railroad company, do not allow personnel or equipment on railroad property.

Prevent material, equipment, and debris from falling onto railroad property.

Add to section 5-1.36:

07-19-13

5-1.36E Survey Monuments

Protect survey monuments on and off the highway. Upon discovery of a survey monument not identified and located immediately:

1. Stop work near the monument
2. Notify the Engineer

Do not resume work near the monument until authorized.

Add between the 1st and 2nd paragraphs of section 5-1.37A:

10-19-12

Do not remove any padlock used to secure a portion of the work until the Engineer is present to replace it. Notify the Engineer at least 3 days before removing the lock.

Replace the 1st sentence of the 1st paragraph of section 5-1.39C(2) with:

10-19-12

Section 5-1.39C(2) applies if a plant establishment period of 3 years or more is shown on the *Notice to Bidders*.

Replace "working days" in the 1st paragraph of section 5-1.43E(1)(a) with:

10-19-12

original working days

^^

6 CONTROL OF MATERIALS

07-19-13

Replace section 6-2.05C with:

04-19-13

6-2.05C Steel and Iron Materials

Steel and iron materials must be melted and manufactured in the United States except:

1. Foreign pig iron and processed, pelletized, and reduced iron ore may be used in the domestic production of the steel and iron materials
2. If the total combined cost of the materials does not exceed the greater of 0.1 percent of the total bid or $2,500, materials produced outside the United States may be used if authorized

Furnish steel and iron materials to be incorporated into the work with certificates of compliance and certified mill test reports. Mill test reports must indicate where the steel and iron were melted and manufactured.

All melting and manufacturing processes for these materials, including an application of a coating, must occur in the United States. Coating includes all processes that protect or enhance the value of the material to which the coating is applied.

Replace "Precast concrete members specified section 11-2" in the table in section 6-3.05B with:

07-19-13

Precast concrete members specified as tier 1 or tier 2 in section 90-4.01D(1)

^^

7 LEGAL RELATIONS AND RESPONSIBILITY TO THE PUBLIC

05-30-14

Replace "$50" in the 1st sentence in the 6th paragraph of section 7-1.02K(2) with:

07-19-13

$200

Replace "$25" in the 2nd sentence in the 13th paragraph of section 7-1.02K(3) with:

07-19-13

$100

05-30-14

Delete "water or" in the 9th paragraph of section 7-1.03.

Replace "20 days" in the 14th paragraph of section 7-1.04 with:

09-16-11

25 days

Replace "90 days" in the 14th paragraph of section 7-1.04 with:

09-16-11

125 days

Add between the 18th and 19th paragraphs of section 7-1.04:

09-16-11

Temporary facilities that could be a hazard to public safety if improperly designed must comply with design requirements described in the Contract for those facilities or, if none are described, with standard design criteria or codes appropriate for the facility involved. Submit shop drawings and design calculations for the temporary facilities and show the standard design criteria or codes used. Shop drawings and supplemental calculations must be sealed and signed by an engineer who is registered as a civil engineer in the State.

Replace the 2nd paragraph of section 7-1.11A with:

07-27-12

A copy of form FHWA-1273 is included in section 7-1.11B. The training and promotion section of section II refers to training provisions as if they were included in the special provisions. The Department specifies the provisions in section 7-1.11D of the *Standard Specifications*. If a number of trainees or apprentices is

required, the Department shows the number on the *Notice to Bidders*. Interpret each FHWA-1273 clause shown in the following table as having the same meaning as the corresponding Department clause:

FHWA-1273 Nondiscrimination Clauses

FHWA-1273 section	FHWA-1273 clause	Department clause
Training and Promotion	In the event a special provision for training is provided under this contract, this subparagraph will be superseded as indicated in the special provision.	If section 7-1.11D applies, section 7-1.11D supersedes this subparagraph.
Records and Reports	If on-the-job training is being required by special provision, the contractor will be required to collect and report training data.	If the Contract requires on-the-job training, collect and report training data.

Replace the form in section 7-1.11B with:

07-20-12

REQUIRED CONTRACT PROVISIONS
FEDERAL-AID CONSTRUCTION CONTRACTS

I. General
II. Nondiscrimination
III. Nonsegregated Facilities
IV. Davis-Bacon and Related Act Provisions
V. Contract Work Hours and Safety Standards Act Provisions
VI. Subletting or Assigning the Contract
VII. Safety: Accident Prevention
VIII. False Statements Concerning Highway Projects
IX. Implementation of Clean Air Act and Federal Water Pollution Control Act
X. Compliance with Governmentwide Suspension and Debarment Requirements
XI. Certification Regarding Use of Contract Funds for Lobbying

ATTACHMENTS

A. Employment and Materials Preference for Appalachian Development Highway System or Appalachian Local Access Road Contracts (included in Appalachian contracts only)

I. GENERAL

1. Form FHWA-1273 must be physically incorporated in each construction contract funded under Title 23 (excluding emergency contracts solely intended for debris removal). The contractor (or subcontractor) must insert this form in each subcontract and further require its inclusion in all lower tier subcontracts (excluding purchase orders, rental agreements and other agreements for supplies or services).

The applicable requirements of Form FHWA-1273 are incorporated by reference for work done under any purchase order, rental agreement or agreement for other services. The prime contractor shall be responsible for compliance by any subcontractor, lower-tier subcontractor or service provider.

Form FHWA-1273 must be included in all Federal-aid design-build contracts, in all subcontracts and in lower tier subcontracts (excluding subcontracts for design services, purchase orders, rental agreements and other agreements for supplies or services). The design-builder shall be responsible for compliance by any subcontractor, lower-tier subcontractor or service provider.

Contracting agencies may reference Form FHWA-1273 in bid proposal or request for proposal documents, however, the Form FHWA-1273 must be physically incorporated (not referenced) in all contracts, subcontracts and lower-tier subcontracts (excluding purchase orders, rental agreements and other agreements for supplies or services related to a construction contract).

2. Subject to the applicability criteria noted in the following sections, these contract provisions shall apply to all work performed on the contract by the contractor's own organization and with the assistance of workers under the contractor's immediate superintendence and to all work performed on the contract by piecework, station work, or by subcontract.

3. A breach of any of the stipulations contained in these Required Contract Provisions may be sufficient grounds for withholding of progress payments, withholding of final payment, termination of the contract, suspension / debarment or any other action determined to be appropriate by the contracting agency and FHWA.

4. Selection of Labor: During the performance of this contract, the contractor shall not use convict labor for any purpose within the limits of a construction project on a Federal-aid highway unless it is labor performed by convicts who are on parole, supervised release, or probation. The term Federal-aid highway does not include roadways functionally classified as local roads or rural minor collectors.

II. NONDISCRIMINATION

The provisions of this section related to 23 CFR Part 230 are applicable to all Federal-aid construction contracts and to all related construction subcontracts of $10,000 or more. The provisions of 23 CFR Part 230 are not applicable to material supply, engineering, or architectural service contracts.

In addition, the contractor and all subcontractors must comply with the following policies: Executive Order 11246, 41 CFR 60, 29 CFR 1625-1627, Title 23 USC Section 140, the Rehabilitation Act of 1973, as amended (29 USC 794), Title VI of the Civil Rights Act of 1964, as amended, and related regulations including 49 CFR Parts 21, 26 and 27; and 23 CFR Parts 200, 230, and 633.

The contractor and all subcontractors must comply with: the requirements of the Equal Opportunity Clause in 41 CFR 60-1.4(b) and, for all construction contracts exceeding $10,000, the Standard Federal Equal Employment Opportunity Construction Contract Specifications in 41 CFR 60-4.3.

Note: The U.S. Department of Labor has exclusive authority to determine compliance with Executive Order 11246 and the policies of the Secretary of Labor including 41 CFR 60, and 29 CFR 1625-1627. The contracting agency and the FHWA have the authority and the responsibility to ensure compliance with Title 23 USC Section 140, the Rehabilitation Act of 1973, as amended (29 USC 794), and Title VI of the Civil Rights Act of 1964, as amended, and related regulations including 49 CFR Parts 21, 26 and 27; and 23 CFR Parts 200, 230, and 633.

The following provision is adopted from 23 CFR 230, Appendix A, with appropriate revisions to conform to the U.S. Department of Labor (US DOL) and FHWA requirements.

1. Equal Employment Opportunity: Equal employment opportunity (EEO) requirements not to discriminate and to take affirmative action to assure equal opportunity as set forth under laws, executive orders, rules, regulations (28 CFR 35, 29 CFR 1630, 29 CFR 1625-1627, 41 CFR 60 and 49 CFR 27) and orders of the Secretary of Labor as modified by the provisions prescribed herein, and imposed pursuant to 23 U.S.C. 140 shall constitute the EEO and specific affirmative action standards for the contractor's project activities under

this contract. The provisions of the Americans with Disabilities Act of 1990 (42 U.S.C. 12101 et seq.) set forth under 28 CFR 35 and 29 CFR 1630 are incorporated by reference in this contract. In the execution of this contract, the contractor agrees to comply with the following minimum specific requirement activities of EEO:

a. The contractor will work with the contracting agency and the Federal Government to ensure that it has made every good faith effort to provide equal opportunity with respect to all of its terms and conditions of employment and in their review of activities under the contract.

b. The contractor will accept as its operating policy the following statement:

"It is the policy of this Company to assure that applicants are employed, and that employees are treated during employment, without regard to their race, religion, sex, color, national origin, age or disability. Such action shall include: employment, upgrading, demotion, or transfer; recruitment or recruitment advertising; layoff or termination; rates of pay or other forms of compensation; and selection for training, including apprenticeship, pre-apprenticeship, and/or on-the-job training."

2. EEO Officer: The contractor will designate and make known to the contracting officers an EEO Officer who will have the responsibility for and must be capable of effectively administering and promoting an active EEO program and who must be assigned adequate authority and responsibility to do so.

3. Dissemination of Policy: All members of the contractor's staff who are authorized to hire, supervise, promote, and discharge employees, or who recommend such action, or who are substantially involved in such action, will be made fully cognizant of, and will implement, the contractor's EEO policy and contractual responsibilities to provide EEO in each grade and classification of employment. To ensure that the above agreement will be met, the following actions will be taken as a minimum:

a. Periodic meetings of supervisory and personnel office employees will be conducted before the start of work and then not less often than once every six months, at which time the contractor's EEO policy and its implementation will be reviewed and explained. The meetings will be conducted by the EEO Officer.

b. All new supervisory or personnel office employees will be given a thorough indoctrination by the EEO Officer, covering all major aspects of the contractor's EEO obligations within thirty days following their reporting for duty with the contractor.

c. All personnel who are engaged in direct recruitment for the project will be instructed by the EEO Officer in the contractor's procedures for locating and hiring minorities and women.

d. Notices and posters setting forth the contractor's EEO policy will be placed in areas readily accessible to employees, applicants for employment and potential employees.

e. The contractor's EEO policy and the procedures to implement such policy will be brought to the attention of employees by means of meetings, employee handbooks, or other appropriate means.

4. Recruitment: When advertising for employees, the contractor will include in all advertisements for employees the notation: "An Equal Opportunity Employer." All such advertisements will be placed in publications having a large circulation among minorities and women in the area from which the project work force would normally be derived.

a. The contractor will, unless precluded by a valid bargaining agreement, conduct systematic and direct recruitment through public and private employee referral sources likely to yield qualified minorities and women. To meet this requirement, the contractor will identify sources of potential minority group employees, and establish with such identified sources procedures whereby minority and women applicants may be referred to the contractor for employment consideration.

b. In the event the contractor has a valid bargaining agreement providing for exclusive hiring hall referrals, the contractor is expected to observe the provisions of that agreement to the extent that the system meets the contractor's compliance with EEO contract provisions. Where implementation of such an agreement has the effect of discriminating against minorities or women, or obligates the contractor to do the same, such implementation violates Federal nondiscrimination provisions.

c. The contractor will encourage its present employees to refer minorities and women as applicants for employment. Information and procedures with regard to referring such applicants will be discussed with employees.

5. Personnel Actions: Wages, working conditions, and employee benefits shall be established and administered, and personnel actions of every type, including hiring, upgrading, promotion, transfer, demotion, layoff, and termination, shall be taken without regard to race, color, religion, sex, national origin, age or disability. The following procedures shall be followed:

a. The contractor will conduct periodic inspections of project sites to insure that working conditions and employee facilities do not indicate discriminatory treatment of project site personnel.

b. The contractor will periodically evaluate the spread of wages paid within each classification to determine any evidence of discriminatory wage practices.

c. The contractor will periodically review selected personnel actions in depth to determine whether there is evidence of discrimination. Where evidence is found, the contractor will promptly take corrective action. If the review indicates that the discrimination may extend beyond the actions reviewed, such corrective action shall include all affected persons.

d. The contractor will promptly investigate all complaints of alleged discrimination made to the contractor in connection with its obligations under this contract, will attempt to resolve such complaints, and will take appropriate corrective action within a reasonable time. If the investigation indicates that the discrimination may affect persons other than the complainant, such corrective action shall include such other persons. Upon completion of each investigation, the contractor will inform every complainant of all of their avenues of appeal.

6. Training and Promotion:

a. The contractor will assist in locating, qualifying, and increasing the skills of minorities and women who are

applicants for employment or current employees. Such efforts should be aimed at developing full journey level status employees in the type of trade or job classification involved.

b. Consistent with the contractor's work force requirements and as permissible under Federal and State regulations, the contractor shall make full use of training programs, i.e., apprenticeship, and on-the-job training programs for the geographical area of contract performance. In the event a special provision for training is provided under this contract, this subparagraph will be superseded as indicated in the special provision. The contracting agency may reserve training positions for persons who receive welfare assistance in accordance with 23 U.S.C. 140(a).

c. The contractor will advise employees and applicants for employment of available training programs and entrance requirements for each.

d. The contractor will periodically review the training and promotion potential of employees who are minorities and women and will encourage eligible employees to apply for such training and promotion.

7. Unions: If the contractor relies in whole or in part upon unions as a source of employees, the contractor will use good faith efforts to obtain the cooperation of such unions to increase opportunities for minorities and women. Actions by the contractor, either directly or through a contractor's association acting as agent, will include the procedures set forth below:

a. The contractor will use good faith efforts to develop, in cooperation with the unions, joint training programs aimed toward qualifying more minorities and women for membership in the unions and increasing the skills of minorities and women so that they may qualify for higher paying employment.

b. The contractor will use good faith efforts to incorporate an EEO clause into each union agreement to the end that such union will be contractually bound to refer applicants without regard to their race, color, religion, sex, national origin, age or disability.

c. The contractor is to obtain information as to the referral practices and policies of the labor union except that to the extent such information is within the exclusive possession of the labor union and such labor union refuses to furnish such information to the contractor, the contractor shall so certify to the contracting agency and shall set forth what efforts have been made to obtain such information.

d. In the event the union is unable to provide the contractor with a reasonable flow of referrals within the time limit set forth in the collective bargaining agreement, the contractor will, through independent recruitment efforts, fill the employment vacancies without regard to race, color, religion, sex, national origin, age or disability; making full efforts to obtain qualified and/or qualifiable minorities and women. The failure of a union to provide sufficient referrals (even though it is obligated to provide exclusive referrals under the terms of a collective bargaining agreement) does not relieve the contractor from the requirements of this paragraph. In the event the union referral practice prevents the contractor from meeting the obligations pursuant to Executive Order 11246, as amended, and these special provisions, such contractor shall immediately notify the contracting agency.

8. Reasonable Accommodation for Applicants / Employees with Disabilities: The contractor must be familiar with the requirements for and comply with the Americans with Disabilities Act and all rules and regulations established there under. Employers must provide reasonable accommodation in all employment activities unless to do so would cause an undue hardship.

9. Selection of Subcontractors, Procurement of Materials and Leasing of Equipment: The contractor shall not discriminate on the grounds of race, color, religion, sex, national origin, age or disability in the selection and retention of subcontractors, including procurement of materials and leases of equipment. The contractor shall take all necessary and reasonable steps to ensure nondiscrimination in the administration of this contract.

a. The contractor shall notify all potential subcontractors and suppliers and lessors of their EEO obligations under this contract.

b. The contractor will use good faith efforts to ensure subcontractor compliance with their EEO obligations.

10. Assurance Required by 49 CFR 26.13(b):

a. The requirements of 49 CFR Part 26 and the State DOT's U.S. DOT-approved DBE program are incorporated by reference.

b. The contractor or subcontractor shall not discriminate on the basis of race, color, national origin, or sex in the performance of this contract. The contractor shall carry out applicable requirements of 49 CFR Part 26 in the award and administration of DOT-assisted contracts. Failure by the contractor to carry out these requirements is a material breach of this contract, which may result in the termination of this contract or such other remedy as the contracting agency deems appropriate.

11. Records and Reports: The contractor shall keep such records as necessary to document compliance with the EEO requirements. Such records shall be retained for a period of three years following the date of the final payment to the contractor for all contract work and shall be available at reasonable times and places for inspection by authorized representatives of the contracting agency and the FHWA.

a. The records kept by the contractor shall document the following:

(1) The number and work hours of minority and non-minority group members and women employed in each work classification on the project;

(2) The progress and efforts being made in cooperation with unions, when applicable, to increase employment opportunities for minorities and women; and

(3) The progress and efforts being made in locating, hiring, training, qualifying, and upgrading minorities and women;

b. The contractors and subcontractors will submit an annual report to the contracting agency each July for the duration of the project, indicating the number of minority, women, and non-minority group employees currently engaged in each work classification required by the contract work. This information is to be reported on Form FHWA-1391. The staffing data should represent the project work force on board in all or any part of the last payroll period preceding the end of July. If on-the-job training is being required by special provision, the contractor

will be required to collect and report training data. The employment data should reflect the work force on board during all or any part of the last payroll period preceding the end of July.

III. NONSEGREGATED FACILITIES

This provision is applicable to all Federal-aid construction contracts and to all related construction subcontracts of $10,000 or more.

The contractor must ensure that facilities provided for employees are provided in such a manner that segregation on the basis of race, color, religion, sex, or national origin cannot result. The contractor may neither require such segregated use by written or oral policies nor tolerate such use by employee custom. The contractor's obligation extends further to ensure that its employees are not assigned to perform their services at any location, under the contractor's control, where the facilities are segregated. The term "facilities" includes waiting rooms, work areas, restaurants and other eating areas, time clocks, restrooms, washrooms, locker rooms, and other storage or dressing areas, parking lots, drinking fountains, recreation or entertainment areas, transportation, and housing provided for employees. The contractor shall provide separate or single-user restrooms and necessary dressing or sleeping areas to assure privacy between sexes.

IV. DAVIS-BACON AND RELATED ACT PROVISIONS

This section is applicable to all Federal-aid construction projects exceeding $2,000 and to all related subcontracts and lower-tier subcontracts (regardless of subcontract size). The requirements apply to all projects located within the right-of-way of a roadway that is functionally classified as Federal-aid highway. This excludes roadways functionally classified as local roads or rural minor collectors, which are exempt. Contracting agencies may elect to apply these requirements to other projects.

The following provisions are from the U.S. Department of Labor regulations in 29 CFR 5.5 "Contract provisions and related matters" with minor revisions to conform to the FHWA-1273 format and FHWA program requirements.

1. Minimum wages

a. All laborers and mechanics employed or working upon the site of the work, will be paid unconditionally and not less often than once a week, and without subsequent deduction or rebate on any account (except such payroll deductions as are permitted by regulations issued by the Secretary of Labor under the Copeland Act (29 CFR part 3)), the full amount of wages and bona fide fringe benefits (or cash equivalents thereof) due at time of payment computed at rates not less than those contained in the wage determination of the Secretary of Labor which is attached hereto and made a part hereof, regardless of any contractual relationship which may be alleged to exist between the contractor and such laborers and mechanics.

Contributions made or costs reasonably anticipated for bona fide fringe benefits under section 1(b)(2) of the Davis-Bacon Act on behalf of laborers or mechanics are considered wages paid to such laborers or mechanics, subject to the provisions of paragraph 1.d. of this section; also, regular contributions made or costs incurred for more than a weekly period (but not less often than quarterly) under plans, funds, or programs which cover the particular weekly period, are deemed to be constructively made or incurred during such weekly period. Such laborers and mechanics shall be paid the appropriate wage rate and fringe benefits on the wage determination for the classification of work actually performed, without regard to skill, except as provided in 29 CFR 5.5(a)(4). Laborers or mechanics performing work in more than one classification may be compensated at the rate specified for each classification for the time actually worked therein: Provided, That the employer's payroll records accurately set forth the time spent in each classification in which work is performed. The wage determination (including any additional classification and wage rates conformed under paragraph 1.b. of this section) and the Davis-Bacon poster (WH–1321) shall be posted at all times by the contractor and its subcontractors at the site of the work in a prominent and accessible place where it can be easily seen by the workers.

b.(1) The contracting officer shall require that any class of laborers or mechanics, including helpers, which is not listed in the wage determination and which is to be employed under the contract shall be classified in conformance with the wage determination. The contracting officer shall approve an additional classification and wage rate and fringe benefits therefore only when the following criteria have been met:

(i) The work to be performed by the classification requested is not performed by a classification in the wage determination; and

(ii) The classification is utilized in the area by the construction industry; and

(iii) The proposed wage rate, including any bona fide fringe benefits, bears a reasonable relationship to the wage rates contained in the wage determination.

(2) If the contractor and the laborers and mechanics to be employed in the classification (if known), or their representatives, and the contracting officer agree on the classification and wage rate (including the amount designated for fringe benefits where appropriate), a report of the action taken shall be sent by the contracting officer to the Administrator of the Wage and Hour Division, Employment Standards Administration, U.S. Department of Labor, Washington, DC 20210. The Administrator, or an authorized representative, will approve, modify, or disapprove every additional classification action within 30 days of receipt and so advise the contracting officer or will notify the contracting officer within the 30-day period that additional time is necessary.

(3) In the event the contractor, the laborers or mechanics to be employed in the classification or their representatives, and the contracting officer do not agree on the proposed classification and wage rate (including the amount designated for fringe benefits, where appropriate), the contracting officer shall refer the questions, including the views of all interested parties and the recommendation of the contracting officer, to the Wage and Hour Administrator for determination. The Wage and Hour Administrator, or an authorized representative, will issue a determination within 30 days of receipt and so advise the contracting officer or

will notify the contracting officer within the 30-day period that additional time is necessary.

(4) The wage rate (including fringe benefits where appropriate) determined pursuant to paragraphs 1.b.(2) or 1.b.(3) of this section, shall be paid to all workers performing work in the classification under this contract from the first day on which work is performed in the classification.

c. Whenever the minimum wage rate prescribed in the contract for a class of laborers or mechanics includes a fringe benefit which is not expressed as an hourly rate, the contractor shall either pay the benefit as stated in the wage determination or shall pay another bona fide fringe benefit or an hourly cash equivalent thereof.

d. If the contractor does not make payments to a trustee or other third person, the contractor may consider as part of the wages of any laborer or mechanic the amount of any costs reasonably anticipated in providing bona fide fringe benefits under a plan or program, Provided, That the Secretary of Labor has found, upon the written request of the contractor, that the applicable standards of the Davis-Bacon Act have been met. The Secretary of Labor may require the contractor to set aside in a separate account assets for the meeting of obligations under the plan or program.

2. Withholding

The contracting agency shall upon its own action or upon written request of an authorized representative of the Department of Labor, withhold or cause to be withheld from the contractor under this contract, or any other Federal contract with the same prime contractor, or any other federally-assisted contract subject to Davis-Bacon prevailing wage requirements, which is held by the same prime contractor, so much of the accrued payments or advances as may be considered necessary to pay laborers and mechanics, including apprentices, trainees, and helpers, employed by the contractor or any subcontractor the full amount of wages required by the contract. In the event of failure to pay any laborer or mechanic, including any apprentice, trainee, or helper, employed or working on the site of the work, all or part of the wages required by the contract, the contracting agency may, after written notice to the contractor, take such action as may be necessary to cause the suspension of any further payment, advance, or guarantee of funds until such violations have ceased.

3. Payrolls and basic records

a. Payrolls and basic records relating thereto shall be maintained by the contractor during the course of the work and preserved for a period of three years thereafter for all laborers and mechanics working at the site of the work. Such records shall contain the name, address, and social security number of each such worker, his or her correct classification, hourly rates of wages paid (including rates of contributions or costs anticipated for bona fide fringe benefits or cash equivalents thereof of the types described in section 1(b)(2)(B) of the Davis-Bacon Act), daily and weekly number of hours worked, deductions made and actual wages paid. Whenever the Secretary of Labor has found under 29 CFR 5.5(a)(1)(iv) that the wages of any laborer or mechanic include the amount of any costs reasonably anticipated in providing benefits under a plan or program described in section 1(b)(2)(B) of the Davis-Bacon Act, the contractor shall maintain records which show that the commitment to provide such benefits is enforceable, that the plan or program is financially responsible, and that the plan or program has been communicated in writing to the laborers or mechanics affected, and records which show the costs anticipated or the actual cost incurred in providing such benefits. Contractors employing apprentices or trainees under approved programs shall maintain written evidence of the registration of apprenticeship programs and certification of trainee programs, the registration of the apprentices and trainees, and the ratios and wage rates prescribed in the applicable programs.

b.(1) The contractor shall submit weekly for each week in which any contract work is performed a copy of all payrolls to the contracting agency. The payrolls submitted shall set out accurately and completely all of the information required to be maintained under 29 CFR 5.5(a)(3)(i), except that full social security numbers and home addresses shall not be included on weekly transmittals. Instead the payrolls shall only need to include an individually identifying number for each employee (e.g. , the last four digits of the employee's social security number). The required weekly payroll information may be submitted in any form desired. Optional Form WH–347 is available for this purpose from the Wage and Hour Division Web site at http://www.dol.gov/esa/whd/forms/wh347instr.htm or its successor site. The prime contractor is responsible for the submission of copies of payrolls by all subcontractors. Contractors and subcontractors shall maintain the full social security number and current address of each covered worker, and shall provide them upon request to the contracting agency for transmission to the State DOT, the FHWA or the Wage and Hour Division of the Department of Labor for purposes of an investigation or audit of compliance with prevailing wage requirements. It is not a violation of this section for a prime contractor to require a subcontractor to provide addresses and social security numbers to the prime contractor for its own records, without weekly submission to the contracting agency..

(2) Each payroll submitted shall be accompanied by a "Statement of Compliance," signed by the contractor or subcontractor or his or her agent who pays or supervises the payment of the persons employed under the contract and shall certify the following:

(i) That the payroll for the payroll period contains the information required to be provided under §5.5 (a)(3)(ii) of Regulations, 29 CFR part 5, the appropriate information is being maintained under §5.5 (a)(3)(i) of Regulations, 29 CFR part 5, and that such information is correct and complete;

(ii) That each laborer or mechanic (including each helper, apprentice, and trainee) employed on the contract during the payroll period has been paid the full weekly wages earned, without rebate, either directly or indirectly, and that no deductions have been made either directly or indirectly from the full wages earned, other than permissible deductions as set forth in Regulations, 29 CFR part 3;

(iii) That each laborer or mechanic has been paid not less than the applicable wage rates and fringe benefits or cash equivalents for the classification of work performed, as specified in the applicable wage determination incorporated into the contract.

(3) The weekly submission of a properly executed certification set forth on the reverse side of Optional Form WH–347 shall satisfy the requirement for submission of the "Statement of Compliance" required by paragraph 3.b.(2) of this section.

(4) The falsification of any of the above certifications may subject the contractor or subcontractor to civil or criminal prosecution under section 1001 of title 18 and section 231 of title 31 of the United States Code.

c. The contractor or subcontractor shall make the records required under paragraph 3.a. of this section available for inspection, copying, or transcription by authorized representatives of the contracting agency, the State DOT, the FHWA, or the Department of Labor, and shall permit such representatives to interview employees during working hours on the job. If the contractor or subcontractor fails to submit the required records or to make them available, the FHWA may, after written notice to the contractor, the contracting agency or the State DOT, take such action as may be necessary to cause the suspension of any further payment, advance, or guarantee of funds. Furthermore, failure to submit the required records upon request or to make such records available may be grounds for debarment action pursuant to 29 CFR 5.12.

4. Apprentices and trainees

a. Apprentices (programs of the USDOL).

Apprentices will be permitted to work at less than the predetermined rate for the work they performed when they are employed pursuant to and individually registered in a bona fide apprenticeship program registered with the U.S. Department of Labor, Employment and Training Administration, Office of Apprenticeship Training, Employer and Labor Services, or with a State Apprenticeship Agency recognized by the Office, or if a person is employed in his or her first 90 days of probationary employment as an apprentice in such an apprenticeship program, who is not individually registered in the program, but who has been certified by the Office of Apprenticeship Training, Employer and Labor Services or a State Apprenticeship Agency (where appropriate) to be eligible for probationary employment as an apprentice.

The allowable ratio of apprentices to journeymen on the job site in any craft classification shall not be greater than the ratio permitted to the contractor as to the entire work force under the registered program. Any worker listed on a payroll at an apprentice wage rate, who is not registered or otherwise employed as stated above, shall be paid not less than the applicable wage rate on the wage determination for the classification of work actually performed. In addition, any apprentice performing work on the job site in excess of the ratio permitted under the registered program shall be paid not less than the applicable wage rate on the wage determination for the work actually performed. Where a contractor is performing construction on a project in a locality other than that in which its program is registered, the ratios and wage rates (expressed in percentages of the journeyman's hourly rate) specified in the contractor's or subcontractor's registered program shall be observed.

Every apprentice must be paid at not less than the rate specified in the registered program for the apprentice's level of progress, expressed as a percentage of the journeymen hourly rate specified in the applicable wage determination. Apprentices shall be paid fringe benefits in accordance with the provisions of the apprenticeship program. If the apprenticeship program does not specify fringe benefits, apprentices must be paid the full amount of fringe benefits listed on the wage determination for the applicable classification. If the Administrator determines that a different practice prevails for the applicable apprentice classification, fringes shall be paid in accordance with that determination.

In the event the Office of Apprenticeship Training, Employer and Labor Services, or a State Apprenticeship Agency recognized by the Office, withdraws approval of an apprenticeship program, the contractor will no longer be permitted to utilize apprentices at less than the applicable predetermined rate for the work performed until an acceptable program is approved.

b. Trainees (programs of the USDOL).

Except as provided in 29 CFR 5.16, trainees will not be permitted to work at less than the predetermined rate for the work performed unless they are employed pursuant to and individually registered in a program which has received prior approval, evidenced by formal certification by the U.S. Department of Labor, Employment and Training Administration.

The ratio of trainees to journeymen on the job site shall not be greater than permitted under the plan approved by the Employment and Training Administration.

Every trainee must be paid at not less than the rate specified in the approved program for the trainee's level of progress, expressed as a percentage of the journeyman hourly rate specified in the applicable wage determination. Trainees shall be paid fringe benefits in accordance with the provisions of the trainee program. If the trainee program does not mention fringe benefits, trainees shall be paid the full amount of fringe benefits listed on the wage determination unless the Administrator of the Wage and Hour Division determines that there is an apprenticeship program associated with the corresponding journeyman wage rate on the wage determination which provides for less than full fringe benefits for apprentices. Any employee listed on the payroll at a trainee rate who is not registered and participating in a training plan approved by the Employment and Training Administration shall be paid not less than the applicable wage rate on the wage determination for the classification of work actually performed. In addition, any trainee performing work on the job site in excess of the ratio permitted under the registered program shall be paid not less than the applicable wage rate on the wage determination for the work actually performed.

In the event the Employment and Training Administration withdraws approval of a training program, the contractor will no longer be permitted to utilize trainees at less than the applicable predetermined rate for the work performed until an acceptable program is approved.

c. Equal employment opportunity. The utilization of apprentices, trainees and journeymen under this part shall be in conformity with the equal employment opportunity requirements of Executive Order 11246, as amended, and 29 CFR part 30.

d. Apprentices and Trainees (programs of the U.S. DOT).

Apprentices and trainees working under apprenticeship and skill training programs which have been certified by the Secretary of Transportation as promoting EEO in connection with Federal-aid highway construction programs are not subject to the requirements of paragraph 4 of this Section IV. The straight time hourly wage rates for apprentices and trainees under such programs will be established by the particular programs. The ratio of apprentices and trainees to journeymen shall not be greater than permitted by the terms of the particular program.

5. Compliance with Copeland Act requirements. The contractor shall comply with the requirements of 29 CFR part 3, which are incorporated by reference in this contract.

6. Subcontracts. The contractor or subcontractor shall insert Form FHWA-1273 in any subcontracts and also require the subcontractors to include Form FHWA-1273 in any lower tier subcontracts. The prime contractor shall be responsible for the compliance by any subcontractor or lower tier subcontractor with all the contract clauses in 29 CFR 5.5.

7. Contract termination: debarment. A breach of the contract clauses in 29 CFR 5.5 may be grounds for termination of the contract, and for debarment as a contractor and a subcontractor as provided in 29 CFR 5.12.

8. Compliance with Davis-Bacon and Related Act requirements. All rulings and interpretations of the Davis-Bacon and Related Acts contained in 29 CFR parts 1, 3, and 5 are herein incorporated by reference in this contract.

9. Disputes concerning labor standards. Disputes arising out of the labor standards provisions of this contract shall not be subject to the general disputes clause of this contract. Such disputes shall be resolved in accordance with the procedures of the Department of Labor set forth in 29 CFR parts 5, 6, and 7. Disputes within the meaning of this clause include disputes between the contractor (or any of its subcontractors) and the contracting agency, the U.S. Department of Labor, or the employees or their representatives.

10. Certification of eligibility.

a. By entering into this contract, the contractor certifies that neither it (nor he or she) nor any person or firm who has an interest in the contractor's firm is a person or firm ineligible to be awarded Government contracts by virtue of section 3(a) of the Davis-Bacon Act or 29 CFR 5.12(a)(1).

b. No part of this contract shall be subcontracted to any person or firm ineligible for award of a Government contract by virtue of section 3(a) of the Davis-Bacon Act or 29 CFR 5.12(a)(1).

c. The penalty for making false statements is prescribed in the U.S. Criminal Code, 18 U.S.C. 1001.

V. CONTRACT WORK HOURS AND SAFETY STANDARDS ACT

The following clauses apply to any Federal-aid construction contract in an amount in excess of $100,000 and subject to the overtime provisions of the Contract Work Hours and Safety Standards Act. These clauses shall be inserted in addition to the clauses required by 29 CFR 5.5(a) or 29 CFR 4.6. As used in this paragraph, the terms laborers and mechanics include watchmen and guards.

1. Overtime requirements. No contractor or subcontractor contracting for any part of the contract work which may require or involve the employment of laborers or mechanics shall require or permit any such laborer or mechanic in any workweek in which he or she is employed on such work to work in excess of forty hours in such workweek unless such laborer or mechanic receives compensation at a rate not less than one and one-half times the basic rate of pay for all hours worked in excess of forty hours in such workweek.

2. Violation; liability for unpaid wages; liquidated damages. In the event of any violation of the clause set forth in paragraph (1.) of this section, the contractor and any subcontractor responsible therefor shall be liable for the unpaid wages. In addition, such contractor and subcontractor shall be liable to the United States (in the case of work done under contract for the District of Columbia or a territory, to such District or to such territory), for liquidated damages. Such liquidated damages shall be computed with respect to each individual laborer or mechanic, including watchmen and guards, employed in violation of the clause set forth in paragraph (1.) of this section, in the sum of $10 for each calendar day on which such individual was required or permitted to work in excess of the standard workweek of forty hours without payment of the overtime wages required by the clause set forth in paragraph (1.) of this section.

3. Withholding for unpaid wages and liquidated damages. The FHWA or the contacting agency shall upon its own action or upon written request of an authorized representative of the Department of Labor withhold or cause to be withheld, from any moneys payable on account of work performed by the contractor or subcontractor under any such contract or any other Federal contract with the same prime contractor, or any other federally-assisted contract subject to the Contract Work Hours and Safety Standards Act, which is held by the same prime contractor, such sums as may be determined to be necessary to satisfy any liabilities of such contractor or subcontractor for unpaid wages and liquidated damages as provided in the clause set forth in paragraph (2.) of this section.

4. Subcontracts. The contractor or subcontractor shall insert in any subcontracts the clauses set forth in paragraph (1.) through (4.) of this section and also a clause requiring the subcontractors to include these clauses in any lower tier subcontracts. The prime contractor shall be responsible for compliance by any subcontractor or lower tier subcontractor with the clauses set forth in paragraphs (1.) through (4.) of this section.

VI. SUBLETTING OR ASSIGNING THE CONTRACT

This provision is applicable to all Federal-aid construction contracts on the National Highway System.

1. The contractor shall perform with its own organization contract work amounting to not less than 30 percent (or a greater percentage if specified elsewhere in the contract) of the total original contract price, excluding any specialty items designated by the contracting agency. Specialty items may be performed by subcontract and the amount of any such specialty items performed may be deducted from the total original contract price before computing the amount of work required to be performed by the contractor's own organization (23 CFR 635.116).

a. The term "perform work with its own organization" refers to workers employed or leased by the prime contractor, and equipment owned or rented by the prime contractor, with or without operators. Such term does not include employees or equipment of a subcontractor or lower tier subcontractor, agents of the prime contractor, or any other assignees. The term may include payments for the costs of hiring leased employees from an employee leasing firm meeting all relevant Federal and State regulatory requirements. Leased employees may only be included in this term if the prime contractor meets all of the following conditions:

(1) the prime contractor maintains control over the supervision of the day-to-day activities of the leased employees;
(2) the prime contractor remains responsible for the quality of the work of the leased employees;
(3) the prime contractor retains all power to accept or exclude individual employees from work on the project; and
(4) the prime contractor remains ultimately responsible for the payment of predetermined minimum wages, the submission of payrolls, statements of compliance and all other Federal regulatory requirements.

b. "Specialty Items" shall be construed to be limited to work that requires highly specialized knowledge, abilities, or equipment not ordinarily available in the type of contracting organizations qualified and expected to bid or propose on the contract as a whole and in general are to be limited to minor components of the overall contract.

2. The contract amount upon which the requirements set forth in paragraph (1) of Section VI is computed includes the cost of material and manufactured products which are to be purchased or produced by the contractor under the contract provisions.

3. The contractor shall furnish (a) a competent superintendent or supervisor who is employed by the firm, has full authority to direct performance of the work in accordance with the contract requirements, and is in charge of all construction operations (regardless of who performs the work) and (b) such other of its own organizational resources (supervision, management, and engineering services) as the contracting officer determines is necessary to assure the performance of the contract.

4. No portion of the contract shall be sublet, assigned or otherwise disposed of except with the written consent of the contracting officer, or authorized representative, and such consent when given shall not be construed to relieve the contractor of any responsibility for the fulfillment of the contract. Written consent will be given only after the contracting agency has assured that each subcontract is evidenced in writing and that it contains all pertinent provisions and requirements of the prime contract.

5. The 30% self-performance requirement of paragraph (1) is not applicable to design-build contracts; however, contracting agencies may establish their own self-performance requirements.

VII. SAFETY: ACCIDENT PREVENTION

This provision is applicable to all Federal-aid construction contracts and to all related subcontracts.

1. In the performance of this contract the contractor shall comply with all applicable Federal, State, and local laws governing safety, health, and sanitation (23 CFR 635). The contractor shall provide all safeguards, safety devices and protective equipment and take any other needed actions as it determines, or as the contracting officer may determine, to be reasonably necessary to protect the life and health of employees on the job and the safety of the public and to protect property in connection with the performance of the work covered by the contract.

2. It is a condition of this contract, and shall be made a condition of each subcontract, which the contractor enters into pursuant to this contract, that the contractor and any subcontractor shall not permit any employee, in performance of the contract, to work in surroundings or under conditions which are unsanitary, hazardous or dangerous to his/her health or safety, as determined under construction safety and health standards (29 CFR 1926) promulgated by the Secretary of Labor, in accordance with Section 107 of the Contract Work Hours and Safety Standards Act (40 U.S.C. 3704).

3. Pursuant to 29 CFR 1926.3, it is a condition of this contract that the Secretary of Labor or authorized representative thereof, shall have right of entry to any site of contract performance to inspect or investigate the matter of compliance with the construction safety and health standards and to carry out the duties of the Secretary under Section 107 of the Contract Work Hours and Safety Standards Act (40 U.S.C.3704).

VIII. FALSE STATEMENTS CONCERNING HIGHWAY PROJECTS

This provision is applicable to all Federal-aid construction contracts and to all related subcontracts.

In order to assure high quality and durable construction in conformity with approved plans and specifications and a high degree of reliability on statements and representations made by engineers, contractors, suppliers, and workers on Federal-aid highway projects, it is essential that all persons concerned with the project perform their functions as carefully, thoroughly, and honestly as possible. Willful falsification, distortion, or misrepresentation with respect to any facts related to the project is a violation of Federal law. To prevent any misunderstanding regarding the seriousness of these and similar acts, Form FHWA-1022 shall be posted on each Federal-aid highway project (23 CFR 635) in one or more places where it is readily available to all persons concerned with the project:

18 U.S.C. 1020 reads as follows:

"Whoever, being an officer, agent, or employee of the United States, or of any State or Territory, or whoever, whether a person, association, firm, or corporation, knowingly makes any false statement, false representation, or false report as to the character, quality, quantity, or cost of the material used or to be used, or the quantity or quality of the work performed or to be performed, or the cost thereof in connection with the submission of plans, maps, specifications, contracts, or costs of construction on any highway or related project submitted for approval to the Secretary of Transportation; or

Whoever knowingly makes any false statement, false representation, false report or false claim with respect to the character, quality, quantity, or cost of any work performed or to be performed, or materials furnished or to be furnished, in connection with the construction of any highway or related project approved by the Secretary of Transportation; or

Whoever knowingly makes any false statement or false representation as to material fact in any statement, certificate, or report submitted pursuant to provisions of the Federal-aid Roads Act approved July 1, 1916, (39 Stat. 355), as amended and supplemented;

Shall be fined under this title or imprisoned not more than 5 years or both."

IX. IMPLEMENTATION OF CLEAN AIR ACT AND FEDERAL WATER POLLUTION CONTROL ACT

This provision is applicable to all Federal-aid construction contracts and to all related subcontracts.

By submission of this bid/proposal or the execution of this contract, or subcontract, as appropriate, the bidder, proposer, Federal-aid construction contractor, or subcontractor, as appropriate, will be deemed to have stipulated as follows:

1. That any person who is or will be utilized in the performance of this contract is not prohibited from receiving an award due to a violation of Section 508 of the Clean Water Act or Section 306 of the Clean Air Act.
2. That the contractor agrees to include or cause to be included the requirements of paragraph (1) of this Section X in every subcontract, and further agrees to take such action as the contracting agency may direct as a means of enforcing such requirements.

X. CERTIFICATION REGARDING DEBARMENT, SUSPENSION, INELIGIBILITY AND VOLUNTARY EXCLUSION

This provision is applicable to all Federal-aid construction contracts, design-build contracts, subcontracts, lower-tier subcontracts, purchase orders, lease agreements, consultant contracts or any other covered transaction requiring FHWA approval or that is estimated to cost $25,000 or more – as defined in 2 CFR Parts 180 and 1200.

1. Instructions for Certification – First Tier Participants:

a. By signing and submitting this proposal, the prospective first tier participant is providing the certification set out below.

b. The inability of a person to provide the certification set out below will not necessarily result in denial of participation in this covered transaction. The prospective first tier participant shall submit an explanation of why it cannot provide the certification set out below. The certification or explanation will be considered in connection with the department or agency's determination whether to enter into this transaction. However, failure of the prospective first tier participant to furnish a certification or an explanation shall disqualify such a person from participation in this transaction.

c. The certification in this clause is a material representation of fact upon which reliance was placed when the contracting agency determined to enter into this transaction. If it is later determined that the prospective participant knowingly rendered an erroneous certification, in addition to other remedies available to the Federal Government, the contracting agency may terminate this transaction for cause of default.

d. The prospective first tier participant shall provide immediate written notice to the contracting agency to whom this proposal is submitted if any time the prospective first tier participant learns that its certification was erroneous when submitted or has become erroneous by reason of changed circumstances.

e. The terms "covered transaction," "debarred," "suspended," "ineligible," "participant," "person," "principal," and "voluntarily excluded," as used in this clause, are defined in 2 CFR Parts 180 and 1200. "First Tier Covered Transactions" refers to any covered transaction between a grantee or subgrantee of Federal funds and a participant (such as the prime or general contract). "Lower Tier Covered Transactions" refers to any covered transaction under a First Tier Covered Transaction (such as subcontracts). "First Tier Participant" refers to the participant who has entered into a covered transaction with a grantee or subgrantee of Federal funds (such as the prime or general contractor). "Lower Tier Participant" refers any participant who has entered into a covered transaction with a First Tier Participant or other Lower Tier Participants (such as subcontractors and suppliers).

f. The prospective first tier participant agrees by submitting this proposal that, should the proposed covered transaction be entered into, it shall not knowingly enter into any lower tier covered transaction with a person who is debarred, suspended, declared ineligible, or voluntarily excluded from participation in this covered transaction, unless authorized by the department or agency entering into this transaction.

g. The prospective first tier participant further agrees by submitting this proposal that it will include the clause titled "Certification Regarding Debarment, Suspension, Ineligibility and Voluntary Exclusion-Lower Tier Covered Transactions," provided by the department or contracting agency, entering into this covered transaction, without modification, in all lower tier covered transactions and in all solicitations for lower tier covered transactions exceeding the $25,000 threshold.

h. A participant in a covered transaction may rely upon a certification of a prospective participant in a lower tier covered transaction that is not debarred, suspended, ineligible, or voluntarily excluded from the covered transaction, unless it knows that the certification is erroneous. A participant is responsible for ensuring that its principals are not suspended, debarred, or otherwise ineligible to participate in covered transactions. To verify the eligibility of its principals, as well as the eligibility of any lower tier prospective participants, each participant may, but is not required to, check the Excluded Parties List System website (https://www.epls.gov/), which is compiled by the General Services Administration.

i. Nothing contained in the foregoing shall be construed to require the establishment of a system of records in order to render in good faith the certification required by this clause. The knowledge and information of the prospective participant is not required to exceed that which is normally possessed by a prudent person in the ordinary course of business dealings.

j. Except for transactions authorized under paragraph (f) of these instructions, if a participant in a covered transaction knowingly enters into a lower tier covered transaction with a person who is suspended, debarred, ineligible, or voluntarily excluded from participation in this transaction, in addition to other remedies available to the Federal Government, the department or agency may terminate this transaction for cause or default.

* * * * *

2. Certification Regarding Debarment, Suspension, Ineligibility and Voluntary Exclusion – First Tier Participants:

a. The prospective first tier participant certifies to the best of its knowledge and belief, that it and its principals:

(1) Are not presently debarred, suspended, proposed for debarment, declared ineligible, or voluntarily excluded from participating in covered transactions by any Federal department or agency;

(2) Have not within a three-year period preceding this proposal been convicted of or had a civil judgment rendered against them for commission of fraud or a criminal offense in connection with obtaining, attempting to obtain, or performing a public (Federal, State or local) transaction or contract under a public transaction; violation of Federal or State antitrust statutes or commission of embezzlement, theft, forgery, bribery, falsification or destruction of records, making false statements, or receiving stolen property;

(3) Are not presently indicted for or otherwise criminally or civilly charged by a governmental entity (Federal, State or local) with commission of any of the offenses enumerated in paragraph (a)(2) of this certification; and

(4) Have not within a three-year period preceding this application/proposal had one or more public transactions (Federal, State or local) terminated for cause or default.

b. Where the prospective participant is unable to certify to any of the statements in this certification, such prospective participant shall attach an explanation to this proposal.

2. **Instructions for Certification - Lower Tier Participants:**

(Applicable to all subcontracts, purchase orders and other lower tier transactions requiring prior FHWA approval or estimated to cost $25,000 or more - 2 CFR Parts 180 and 1200)

a. By signing and submitting this proposal, the prospective lower tier is providing the certification set out below.

b. The certification in this clause is a material representation of fact upon which reliance was placed when this transaction was entered into. If it is later determined that the prospective lower tier participant knowingly rendered an erroneous certification, in addition to other remedies available to the Federal Government, the department, or agency with which this transaction originated may pursue available remedies, including suspension and/or debarment.

c. The prospective lower tier participant shall provide immediate written notice to the person to which this proposal is submitted if at any time the prospective lower tier participant learns that its certification was erroneous by reason of changed circumstances.

d. The terms "covered transaction," "debarred," "suspended," "ineligible," "participant," "person," "principal," and "voluntarily excluded," as used in this clause, are defined in 2 CFR Parts 180 and 1200. You may contact the person to which this proposal is submitted for assistance in obtaining a copy of those regulations. "First Tier Covered Transactions" refers to any covered transaction between a grantee or subgrantee of Federal funds and a participant (such as the prime or general contract). "Lower Tier Covered Transactions" refers to any covered transaction under a First Tier Covered Transaction (such as subcontracts). "First Tier Participant" refers to the participant who has entered into a covered transaction with a grantee or subgrantee of Federal funds (such as the prime or general contractor). "Lower Tier Participant" refers any participant who has entered into a covered transaction with a First Tier Participant or other Lower Tier Participants (such as subcontractors and suppliers).

e. The prospective lower tier participant agrees by submitting this proposal that, should the proposed covered transaction be entered into, it shall not knowingly enter into any lower tier covered transaction with a person who is debarred, suspended, declared ineligible, or voluntarily excluded from participation in this covered transaction, unless authorized by the department or agency with which this transaction originated.

f. The prospective lower tier participant further agrees by submitting this proposal that it will include this clause titled "Certification Regarding Debarment, Suspension, Ineligibility and Voluntary Exclusion-Lower Tier Covered Transaction," without modification, in all lower tier covered transactions and in all solicitations for lower tier covered transactions exceeding the $25,000 threshold.

g. A participant in a covered transaction may rely upon a certification of a prospective participant in a lower tier covered transaction that is not debarred, suspended, ineligible, or voluntarily excluded from the covered transaction, unless it knows that the certification is erroneous. A participant is responsible for ensuring that its principals are not suspended, debarred, or otherwise ineligible to participate in covered transactions. To verify the eligibility of its principals, as well as the eligibility of any lower tier prospective participants, each participant may, but is not required to, check the Excluded Parties List System website (https://www.epls.gov/), which is compiled by the General Services Administration.

h. Nothing contained in the foregoing shall be construed to require establishment of a system of records in order to render in good faith the certification required by this clause. The knowledge and information of participant is not required to exceed that which is normally possessed by a prudent person in the ordinary course of business dealings.

i. Except for transactions authorized under paragraph e of these instructions, if a participant in a covered transaction knowingly enters into a lower tier covered transaction with a person who is suspended, debarred, ineligible, or voluntarily excluded from participation in this transaction, in addition to other remedies available to the Federal Government, the

department or agency with which this transaction originated may pursue available remedies, including suspension and/or debarment.

* * * * *

Certification Regarding Debarment, Suspension, Ineligibility and Voluntary Exclusion--Lower Tier Participants:

1. The prospective lower tier participant certifies, by submission of this proposal, that neither it nor its principals is presently debarred, suspended, proposed for debarment, declared ineligible, or voluntarily excluded from participating in covered transactions by any Federal department or agency.

2. Where the prospective lower tier participant is unable to certify to any of the statements in this certification, such prospective participant shall attach an explanation to this proposal.

* * * * *

XI. CERTIFICATION REGARDING USE OF CONTRACT FUNDS FOR LOBBYING

This provision is applicable to all Federal-aid construction contracts and to all related subcontracts which exceed $100,000 (49 CFR 20).

1. The prospective participant certifies, by signing and submitting this bid or proposal, to the best of his or her knowledge and belief, that:

a. No Federal appropriated funds have been paid or will be paid, by or on behalf of the undersigned, to any person for influencing or attempting to influence an officer or employee of any Federal agency, a Member of Congress, an officer or employee of Congress, or an employee of a Member of Congress in connection with the awarding of any Federal contract, the making of any Federal grant, the making of any Federal loan, the entering into of any cooperative agreement, and the extension, continuation, renewal, amendment, or modification of any Federal contract, grant, loan, or cooperative agreement.

b. If any funds other than Federal appropriated funds have been paid or will be paid to any person for influencing or attempting to influence an officer or employee of any Federal agency, a Member of Congress, an officer or employee of Congress, or an employee of a Member of Congress in connection with this Federal contract, grant, loan, or cooperative agreement, the undersigned shall complete and submit Standard Form-LLL, "Disclosure Form to Report Lobbying," in accordance with its instructions.

2. This certification is a material representation of fact upon which reliance was placed when this transaction was made or entered into. Submission of this certification is a prerequisite for making or entering into this transaction imposed by 31 U.S.C. 1352. Any person who fails to file the required certification shall be subject to a civil penalty of not less than $10,000 and not more than $100,000 for each such failure.

3. The prospective participant also agrees by submitting its bid or proposal that the participant shall require that the language of this certification be included in all lower tier subcontracts, which exceed $100,000 and that all such recipients shall certify and disclose accordingly.

^^

8 PROSECUTION AND PROGRESS

10-19-12

Replace "working days" in the 1st paragraph of section 8-1.02B(1) with:

10-19-12

original working days

Replace "working days" at each occurrence in the 1st paragraph of section 8-1.02C(1) with:

10-19-12

original working days

04-20-12

Delete the 4th paragraph of section 8-1.02C(1).

Replace "Contract" in the 9th paragraph of section 8-1.02C(1) with:

10-19-12

work

Replace the 1st paragraph of section 8-1.02C(3)(a) with:

04-20-12

Submit a description of your proposed schedule software for authorization.

04-20-12

Delete the last paragraph of section 8-1.02C(3)(a).

Replace section 8-1.02C(3)(b) with:

10-19-12

8-1.02C(3)(b) Reserved

04-20-12

Delete the 3rd paragraph of section 8-1.02C(5).

Replace "Contract" in the last paragraph of section 8-1.02C(5) with:

10-19-12

original

Replace "working days" in the 1st paragraph of section 8-1.02D(1) with:

10-19-12

original working days

Replace "8-1.02D(1)" in the 2nd paragraph of section 8-1.02D(1) with:

01-20-12

8-1.02C(1)

Replace "Contract" in the 3rd paragraph of section 8-1.02D(2) with:

10-19-12

work

Replace "Contract" in item 9 in the list in the 4th paragraph of section 8-1.02D(4) with:

10-19-12

work

Replace "Contract completion" in the 4th paragraph of section 8-1.02D(6) with:

10-19-12

work completion

Replace "Contract working days" in the 4th paragraph of section 8-1.02D(6) with:

10-19-12

original working days

04-20-12

Delete items 1.3 and 1.4 in the list in the 1st paragraph of section 8-1.02D(10).

Replace the last paragraph of section 8-1.04B with:

10-19-12

The Department does not adjust time for starting before receiving notice of Contract approval.

Replace the 1st paragraph of section 8-1.05 with:

10-19-12

Contract time starts on the last day specified to start job site activities in section 8-1.04 or on the day you start job site activities, whichever occurs first.

Replace the 2nd paragraph of section 8-1.05 with:

10-19-12

Complete the work within the Contract time.

10-19-12

Delete "unless the Contract is suspended for reasons unrelated to your performance" in the 4th paragraph of section 8-1.05.

Replace the headings and paragraphs in section 8-1.06 with:

10-19-12

The Engineer may suspend work wholly or in part due to conditions unsuitable for work progress. Provide for public safety and a smooth and unobstructed passageway through the work zone during the suspension as specified under sections 7-1.03 and 7-1.04. Providing the passageway is force account work. The Department makes a time adjustment for the suspension due to a critical delay.

The Engineer may suspend work wholly or in part due to your failure to (1) fulfill the Engineer's orders, (2) fulfill a Contract part, or (3) perform weather-dependent work when conditions are favorable so that weather-related unsuitable conditions are avoided or do not occur. The Department may provide for a

smooth and unobstructed passageway through the work during the suspension and deduct the cost from payments. The Department does not make a time adjustment for the suspension.

Upon the Engineer's order of suspension, suspend work immediately. Resume work when ordered.

Replace the 1st sentence in the 1st paragraph of section 8-1.07B with:

10-19-12

For a critical delay, the Department may make a time adjustment.

Add to the end of section 8-1.07C:

10-19-12

The Department does not make a payment adjustment for overhead incurred during non–working days that extend the Contract into an additional construction season.

Replace the 1st paragraph of section 8-1.07C with:

10-19-12

For an excusable delay that affects your costs, the Department may make a payment adjustment.

Replace "8-1.08B and 8-1.08C" in the 1st paragraph of section 8-1.10A with:

08-05-11

8-1.10B and 8-1.10C

Replace section 8-1.10D with:

10-19-12

8-1.10D Reserved

^^

9 PAYMENT

11-15-13

Add to the list in the 1st paragraph of section 9-1.03:

07-19-13

3. Any royalties and costs arising from patents, trademarks, and copyrights involved in the work

Replace item 1 in the 3rd paragraph of section 9-1.03 with:

01-18-13

1. Full compensation for all work involved in each bid item shown on the Bid Item List by the unit of measure shown for that bid item

Replace "in" in the 3rd paragraph of section 9-1.04A with:

10-19-12

for

Add to the end of section 9-1.04A:

10-19-12

For nonsubcontracted work paid by force account for a contract with a TRO bid item, the markups are those shown in the following table instead of those specified in sections 9-1.04B–D:

Cost	Percent markup
Labor	30
Materials	10
Equipment rental	10

04-20-12

Delete ", Huntington Beach," in the 3rd paragraph of section 9-1.07A.

Replace the formula in section 9-1.07B(2) with:

04-20-12

$Qh = HMATT \times Xa$

Replace "weight of dry aggregate" in the definition of the variable *Xa* in section 9-1.07B(2) with:

04-20-12

total weight of HMA

Replace the formula in section 9-1.07B(3) with:

04-20-12

$Qrh = RHMATT \times 0.80 \times Xarb$

Replace "weight of dry aggregate" in the definition of the variable *Xarb* in section 9-1.07B(3) with:

04-20-12

total weight of rubberized HMA

Replace the heading of section 9-1.07B(4) with:

04-20-12

Hot Mix Asphalt with Modified Asphalt Binder

Add between "in" and "modified" in the introductory clause of section 9-1.07B(4):

04-20-12

HMA with

Replace the formula in section 9-1.07B(4) with:

04-20-12

$Qmh = MHMATT \times [(100 - Xam) / 100] \times Xmab$

Replace "weight of dry aggregate" in the definition of the variable *Xmab* in section 9-1.07B(4) with:

04-20-12

total weight of HMA

Replace the formula in section 9-1.07B(5) with:

04-20-12

Qrap = *HMATT* x *Xaa*

Replace "weight of dry aggregate" in the definitions of the variables *Xaa* and *Xta* in section 9-1.07B(5) with:

04-20-12

total weight of HMA

Add after the variable definitions in section 9-1.07B(9):

04-20-12

The quantity of extender oil is included in the quantity of asphalt.

Replace the headings and paragraphs in section 9-1.11 with:

10-19-12

9-1.11A General

Section 9-1.11 applies if a bid item for time-related overhead is included in the Contract. If a bid item for time-related overhead is included, you must exclude the time-related overhead from every other bid item price.

9-1.11B Payment Quantity

The TRO quantity does not include the number of working days to complete plant establishment work.

For a contract with a TRO lump sum quantity on the Bid Item List, the Department pays you based on the following conversions:

1. LS unit of measure is replaced with WDAY
2. Lump sum quantity is replaced with the number of working days bid
3. Lump sum unit price is replaced with the item total divided by the number of working days bid

9-1.11C Payment Inclusions

Payment for the TRO bid item includes payment for time-related field- and home-office overhead for the time required to complete the work.

The field office overhead includes time-related expenses associated with the normal and recurring construction activities not directly attributed to the work, including:

1. Salaries, benefits, and equipment costs of:
   1.1. Project managers
   1.2. General superintendents
   1.3. Field office managers
   1.4. Field office staff assigned to the project
2. Rent
3. Utilities
4. Maintenance
5. Security
6. Supplies
7. Office equipment costs for the project's field office

The home-office overhead includes the fixed general and administrative expenses for operating your business, including:

1. General administration

2. Insurance
3. Personnel and subcontract administration
4. Purchasing
5. Accounting
6. Project engineering and estimating

Payment for the TRO bid item does not include payment for:

1. The home-office overhead expenses specifically related to:
   1.1. Your other contracts or other businesses
   1.2. Equipment coordination
   1.3. Material deliveries
   1.4. Consultant and legal fees
2. Non-time-related costs and expenses such as mobilization, licenses, permits, and other charges incurred once during the Contract
3. Additional overhead involved in incentive/disincentive provisions to satisfy an internal milestone or multiple calendar requirements
4. Additional overhead involved in performing additional work that is not a controlling activity
5. Overhead costs incurred by your subcontractors of any tier or suppliers

9-1.11D Payment Schedule

For progress payments, the total work completed for the TRO bid item is the number of working days shown for the pay period on the *Weekly Statement of Working Days*.

For progress payments, the Department pays a unit price equal to the lesser of the following amounts:

1. Price per working day as bid or as converted under section 9-1.11B.
2. 20 percent of the total bid divided by the number of original working days

For a contract without plant establishment work, the Department pays you the balance due of the TRO item total as specified in section 9-1.17B.

For a contract with plant establishment work, the Department pays you the balance due of the TRO item total in the 1st progress payment after all non–plant establishment work is completed.

9-1.11E Payment Adjustments

The 3rd paragraph of section 9-1.17C does not apply.

The Department does not adjust the unit price for an increase or decrease in the TRO quantity except as specified in section 9-1.11E.

Section 9-1.17D(2)(b) does not apply except as specified for the audit report below.

If the TRO bid item quantity exceeds 149 percent of the quantity shown on the Bid Item List or as converted under section 9-1.11B, the Engineer may adjust or you may request an adjustment of the unit price for the excess quantity. For the adjustment, submit an audit report within 60 days of the Engineer's request. The report must be prepared as specified for an audit report for an overhead claim in section 9-1.17D(2)(b).

Within 20 days of the Engineer's request, make your financial records available for an audit by the State for the purpose of verifying the actual rate of TRO described in your audit. The actual rate of TRO described is subject to the Engineer's authorization.

The Department pays the authorized actual rate for TRO in excess of 149 percent of the quantity shown on the Bid Item List or as converted under section 9-1.11B.

The Department pays for 1/2 the cost of the report; the Contractor pays for the other 1/2. The cost is determined under section 9-1.05.

Replace the paragraphs of section 9-1.16D with:

07-19-13

9-1.16D(1) General

Section 9-1.16D applies if a bid item for mobilization is shown on the Bid Item List.

Payments for mobilization made under section 9-1.16D are in addition to the partial payments made under Pub Cont Code § 10261.

Section 9-1.16D(2) applies unless the Contract includes a special provision for section 9-1.16D(1) that specifies section 9-1.16D(3) applies.

11-15-13

9-1.16D(2) Mobilization for Projects Except for Those Over Water Requiring Marine Access

07-19-13

The Department makes partial payments for mobilization under Pub Cont Code § 10264(a) except the amount of work completed does not include the amount earned for mobilization. The partial payment amount is reduced by a prorated amount bid in excess of the maximum allowed under Pub Cont Code § 10264(a)(5).

The Department pays the item total for mobilization in excess of the maximum allowed under Pub Cont Code § 10264(a)(5) in the 1st payment after Contract acceptance.

9-1.16D(3) Mobilization for Projects Over Water Requiring Marine Access

The Department makes partial payments for mobilization under Pub Cont Code § 10264(b) except the amount of work completed does not include the amount earned for mobilization. The partial payment amount is reduced by a prorated amount bid in excess of the maximum allowed under Pub Cont Code § 10264(b)(6).

The Department pays the item total for mobilization in excess of the maximum allowed under Pub Cont Code § 10264(b)(6) in the 1st payment after Contract acceptance.

10-19-12

Delete "revised Contract" in item 1 of the 1st paragraph of section 9-1.16E(2).

Replace "2014" in the 1st paragraph of section 9-1.16F with:

10-19-12

2020

Replace the 2nd paragraph of section 9-1.17C with:

10-19-12

Submit either a written acceptance of the proposed final estimate or a claim statement postmarked or hand delivered before the 31st day after receiving the proposed final estimate.

Add between "the" and "final estimate" in the 1st sentence in the 3rd paragraph of section 9-1.17C:

10-19-12

proposed

Replace the 1st sentence in the 6th paragraph of section 9-1.17D(2)(b) with:

07-19-13

The CPA's audit must be performed as an examination-level engagement under the attestation engagements in the *Government Auditing Standards* published by the Comptroller General of the United States.

^^^

Replace section 10-5 with:

05-30-14

10-5 DUST CONTROL

Section 10-5 includes general specifications for controlling dust resulting from the work.

Prevent and alleviate dust by:

1. Applying a dust palliative under section 18
2. Applying temporary soil stabilization under section 13-5
3. Managing material stockpiles under section 13-4.03C(3)

04-19-13

10-6 JOB SITE WATER CONTROL

10-6.01 GENERAL

Section 10-6 includes specifications for controlling water to provide a dry working area at the job site.

10-6.02 WATER-FILLED COFFERDAM

Reserved

10-6.03–10-6.10 RESERVED

10-7–10-20 RESERVED

^^

11 QUALITY CONTROL AND ASSURANCE

07-19-13

Replace section 11-2 with:

07-19-13

11-2 RESERVED

Replace the table in the 3rd paragraph of section 11-3.01A with:

07-19-13

AWS code	Year of adoption
D1.1	2010
D1.3	2008
D1.4	2011
D1.5	2010
D1.6	2007
D1.8	2009

Replace "does" in the definition of "continuous inspection" in section 11-3.01B with:

07-19-13

do

Replace "gross nonconformance" and its definition in section 11-3.01B with:

07-19-13

gross nonconformance: Rejectable indications are present in more than 20 percent of the tested weld length.

Replace the introductory clause in the 1st paragraph of section 11-3.01C with:

07-19-13

Replace clause 6.1.3 of AWS D1.1, the 1st paragraph of clause 7.1.2 of AWS D1.4, and clause 6.1.2 of AWS D1.5 with:

Replace the 3rd paragraph of section 11-3.01C with:

07-19-13

For each inspection, including fit-up, WPS verification, and final weld inspection, the QC Inspector must confirm and document compliance with the specifications, AWS welding codes, and any referenced drawings.

Replace the paragraphs in section 11-3.01D with:

07-19-13

The Engineer has the authority to verify the qualifications or certifications of any welder, QC Inspector, or NDT personnel to specified levels by retests or other means determined by the Engineer. If welding will be performed without gas shielding, then qualification must also include welding without gas shielding.

Replace clause 6.14.6.1 of AWS D1.1, clause 7.8 of AWS D1.4, and clause 6.1.3.4 of AWS D1.5 with:

> Personnel performing NDT must be qualified and certified under American Society for Nondestructive Testing (ASNT) Recommended Practice No. SNT-TC-1A and the written practice of the NDT firm. The written practice of the NDT firm must comply with or exceed the guidelines of the ASNT Recommended Practice No. SNT-TC-1A. Individuals who perform NDT, review the results, and prepare the written reports must be one of the following:
>
> 1. Certified NDT Level II technicians
> 2. Level III technicians certified to perform the work of Level II technicians

Replace the heading and the 1st through 3rd paragraphs of section 11-3.01E with:

07-19-13

11-3.01E Weld Joint Details

If weld joint details proposed for use in the work are not prequalified under clause 3 of AWS D1.1 or figure 2.4 or 2.5 of AWS D1.5, submit the proposed WPS and the intended weld joint locations.

Upon authorization of the proposed joint detail locations and qualification of the proposed joint details, welders and welding operators using these details must weld an additional qualification test plate using the WPS variables and the weld joint detail to be used in production. The test plate must:

1. Have the maximum thickness to be used in production and a minimum length of 18 inches.
2. Be mechanically and radiographically tested. Mechanical and radiographic testing and acceptance criteria must comply with the applicable AWS codes.

If a nonprequalified weld joint configuration is proposed using a combination of WPSs for work welded under AWS D1.1, you may conduct a single test combining the WPSs to be used in production, if the essential variables, including weld bead placement, of each process are limited to those established in table 4.5 of AWS D1.1.

Replace the 1st paragraph of section 11-3.01F with:

07-19-13

Replace paragraph 3 of clause 6.26.3.2 of AWS D1.5 with:

3. If indications that exhibit these planar characteristics are present at scanning sensitivity, or other evidence exists to suggest the presence of transverse cracks, a more detailed evaluation of the discontinuity by other means must be performed (e.g., alternate UT techniques, RT, grinding, or gouging for visual inspection or MT of the excavated areas.). For welds that have transverse cracks, excavate the full length of the crack plus 2 inches of weld metal on each side adjacent to the crack and reweld.

Replace "section" in the 2nd paragraph of section 11-3.01F with:

07-19-13

clause

Replace the 1st paragraph of section 11-3.02A with:

07-19-13

Except for stud welding, section 11-3.02 applies to (1) work welded under sections 49, 52, 55, and 75-1.03E and (2) work in section 99 that must comply with an AWS welding code.

Replace the 4th through 6th paragraphs of section 11-3.02C(2) with:

07-19-13

Submit an amended welding QC plan or an addendum to the welding QC plan for any changes to:

1. WPSs
2. NDT firms
3. QC personnel or procedures
4. NDT personnel or procedures
5. Systems for tracking and identifying welds
6. Welding personnel

Allow 15 days for the Engineer's review of an amended welding QC plan or an addendum to the welding QC plan.

Submit 7 copies of each authorized QC plan and any authorized addendums. Make 1 copy available at each location where work is performed.

Replace the 1st paragraph of section 11-3.02C(3) with:

07-19-13

Submit a welding report within 7 days following the performance of any welding. The welding report must include:

1. Daily production log for welding for each day that welding is performed
2. Reports of all visual weld inspections and NDT performed, whether specified, additional, or informational
3. Radiographs and radiographic reports, and other required NDT reports
4. Summary of welding and NDT activities that occurred during the reporting period
5. Reports of each application of heat straightening
6. Summarized log listing the rejected lengths of weld by welder, position, process, joint configuration, and piece number
7. Documentation that you have:
   - 7.1. Evaluated all radiographs and radiograph reports and NDT and NDT reports
   - 7.2. Corrected all rejectable deficiencies and that all repaired welds have been reexamined using the required NDT and found acceptable
8. Reports or chart recordings of each application of any stress relieving used
9. Reports and chart recordings for any electroslag welding used

Add between "radiographic" and "envelopes" in the introductory clause in the 3rd paragraph of section 11-3.02C(3):

07-19-13

film

07-19-13

Delete the 3rd sentence in the 5th paragraph of section 11-3.02C(3).

Replace the introductory clause in the 1st paragraph of section 11-3.02D with:

07-19-13

Clauses 6.1.4.1 and 6.1.4.3 of AWS D1.1, the 2nd paragraph of clause 7.1.2 of AWS D1.4, clauses 6.1.3.1 through 6.1.3.3 of AWS D1.5, and clause 7.2.3 of AWS D1.8 are replaced with:

Replace items 1 and 2 in the list in the 2nd paragraph of section 11-3.02D with:

07-19-13

1. Work is welded at a permanent fabrication or manufacturing plant that is certified under the AISC Certification Program for Steel Bridge Fabricators, Intermediate Bridges, and Fracture-Critical Member endorsement if required.
2. Structural steel for building construction work is performed at a permanent fabrication or manufacturing plant that is certified under the AISC Quality Certification Program, Category STD, Standard for Steel Building Structures.

07-19-13

Delete the 3rd paragraph of section 11-3.02D.

Replace the 1st sentence in the 4th paragraph of section 11-3.02D with:

07-19-13

Except for the exempt facilities identified above, an authorized independent third party must witness the qualification tests for welders or welding operators.

Replace the paragraph in section 11-3.02F with:

07-19-13

Welding procedures qualification for work welded under AWS D1.5 must comply with clause 5.12 or 5.12.4 of AWS D1.5 and the following:

1. Unless considered prequalified, qualify fillet welds in each position. Conduct the fillet weld soundness test using the essential variables of the WPS as established by the PQR.
2. For qualifying joints that do not comply with figures 2.4 and 2.5 of AWS D1.5, conduct the test complying with figure 5.3 using the welding parameters that were established for the test conducted complying with figure 5.1.
3. Macroetch tests are required for WPS qualification tests, and acceptance must comply with clause 5.19.3 of AWS D1.5.
4. If a nonstandard weld joint is to be made using a combination of WPSs, you may conduct a test under figure 5.3, combining the qualified or prequalified WPSs to be used in production, if the essential variables, including weld bead placement, of each process are limited to those established in table 5.3 of AWS D1.5.
5. Before preparing mechanical test specimens, inspect the PQR welds by visual and radiographic tests. The backing bar must be 3 inches in width and must remain in place during NDT. Results of the

visual and radiographic tests must comply with clause 6.26.2 of AWS D1.5 excluding clause 6.26.2.2. All other requirements for clause 5.17 are applicable.

Add to the list in the 3rd paragraph of section 11-3.02G:

07-19-13

3. Repairs not included in the welding QC plan

Replace the 1st sentence of the 4th paragraph of section 11-3.02G with:

07-19-13

Requests to perform 3rd-time excavations, repairs of cracks, or repairs not included in the welding QC plan must include an engineering evaluation.

^^

12 TEMPORARY TRAFFIC CONTROL

05-30-14

Replace the 5th paragraph of section 12-3.01A(1) with:

05-30-14

Repair or replace traffic-handling equipment and devices damaged from any cause during the Contract, including repainting if necessary. The condition of temporary traffic control devices must comply with the current American Traffic Safety Services Association publication "Quality Guidelines for Temporary Traffic Control Devices and Features."

Replace the 1st paragraph of section 12-3.01A(4) with:

10-19-12

Category 2 temporary traffic control devices must be on FHWA's list of acceptable, crashworthy Category 2 hardware for work zones. This list is available on FHWA's Safety Program Web site.

Replace "project" in the 4th paragraph of section 12-3.02C with:

10-19-12

work

Add after "Display" in item 4 in the list in the 2nd paragraph of section 12-3.03B:

04-19-13

or Alternating Diamond

Replace "project" in the 3rd paragraph of section 12-3.07C with:

10-19-12

work

Add to section 12-3:

07-19-13

12-3.18 AUTOMATED WORK ZONE INFORMATION SYSTEM

Reserved

12-3.19–12-3.25 RESERVED

Replace the 7th through 9th paragraphs of section 12-4.02A with:

07-19-13

If pedestrian traffic is allowed to pass through construction areas, provide a temporary pedestrian facility through the construction areas within the highway. Include protective overhead covering as necessary to ensure protection from falling objects and drippings from overhead structures.

At locations where pedestrian openings through falsework are required, provide a temporary pedestrian facility with protective overhead covering during all bridge construction activities.

Temporary pedestrian facilities must comply with section 12-7.

If an activity requires a closure of a walkway, another walkway must be made available nearby, off of the traveled way.

07-19-13

Delete the 12th paragraph of section 12-4.02A.

Replace section 12-4.03 with:

07-19-13

12-4.03 CLOSURE SCHEDULES AND CONDITIONS

12-4.03A General

Submit closure schedule requests and closure schedule amendments using LCS to show the locations and times of the requested closures.

The Department provides LCS training. Request the LCS training at least 30 days before submitting the 1st lane closure request. The Department provides the training within 15 days after your request. The training may be web based.

Except for web-based training, the training is held at a time and location you and the Engineer agree to.

For web-based training, the Engineer provides you the website address to access the training.

Within 5 business days after completion of the training, the Department provides LCS accounts and user identifications to your assigned, trained representatives.

Each representative must maintain a unique password and current user information in the LCS.

12-4.03B Closure Schedules

Every Monday by noon, submit a closure schedule request of planned closures for the next week period. The next week period is defined as Sunday noon through the following Sunday noon.

Submit a closure schedule request not less than 25 days and not more than 125 days before the anticipated start of any activity that reduces:

1. Horizontal clearances of traveled ways, including shoulders, to 2 lanes or less due to activities such as temporary barrier placement and paving
2. Vertical clearances of traveled way, including shoulders, due to activities such as pavement overlays, overhead sign installation, falsework, or girder erection

Submit closure schedule amendments, including adding additional closures, by noon at least 3 business days before a planned closure.

Cancel closure requests using LCS at least 48 hours before the start time of the closure.

You will be notified through LCS of unauthorized closures or closures that require coordination with other parties as a condition for authorization.

The Engineer may reschedule a closure cancelled due to unsuitable weather.

If a closure is not opened to traffic by the specified time, suspend work. No further closures are allowed until the Engineer has reviewed and authorized a work plan submitted by you that ensures that future closures will be opened to traffic by the specified time. Allow 2 business days for review of your proposed work plan. The Department does not compensate you for your losses due to the suspension of work resulting from the late opening of closures.

Notify the Engineer of delays in your activities caused by:

1. Your closure schedule request being denied although your requested closures are within the specified time frame allowed for closures. The Department does not compensate you for your losses due to amendments to the closure schedule that are not authorized.
2. Your authorized closure being denied.

If you are directed to remove a closure before the time designated in the authorized closure schedule, you will be compensated for the delay.

12-4.03C Contingency Plan

Section 12-4.03C applies if a contingency plan is specified in the special provisions or if a contingency plan is requested.

If a contingency plan is requested, submit the contingency plan within 1 business day of the request.

The contingency plan must identify the activities, equipment, processes, and materials that may cause a delay in the opening of a closure to traffic. The plan must include:

1. List of additional or alternate equipment, materials, or workers necessary to ensure continuing activities and on-time opening of closures if a problem occurs. If the additional or alternate equipment, materials, or workers are not on site, specify their location, the method for mobilizing these items, and the required time to complete mobilization.
2. General time-scaled logic diagram displaying the major activities and sequence of planned operations. For each activity, identify the critical event when the contingency plan will be activated.

Based on the Engineer's review, additional materials, equipment, workers, or time to complete activities from that specified in the contingency plan may be required.

Submit revisions to a contingency plan at least 3 business days before starting the activity requiring a contingency plan. Allow 2 business days for review of the revised contingency plan.

Replace section 12-7 with:

07-19-13

12-7 TEMPORARY PEDESTRIAN FACILITIES

12-7.01 GENERAL

Section 12-7 includes specifications for constructing temporary pedestrian facilities.

Temporary pedestrian facilities must comply with the *California MUTCD*, Part 6, Chapter 6D, "Pedestrian and Worker Safety."

Design temporary pedestrian facilities with protective overhead covering to support all imposed loads.

The design load and maximum allowable stresses for temporary pedestrian facilities with protective overhead covering must comply with section 48-2.01D(3). The minimum design live load for the temporary pedestrian facilities with protective overhead covering must be 150 psf for the entire structure.

The minimum width of the temporary pedestrian facilities with protective overhead covering between the inside face of handrails must be 60 inches. The clear height of the temporary pedestrian facilities with protective overhead covering measured from the floor surface to the canopy overhead must be at least 8 feet. Provide adequate lighting at all times. Lighting must comply with section 86-6.13.

Submit shop drawings with supporting calculations for temporary pedestrian facilities with protective overhead covering. Shop drawings and calculations must be signed by an engineer who is registered as a civil engineer in the State.

12-7.02 MATERIALS

Walkways must be surfaced with HMA, portland cement concrete, or wood. The surface must be skid resistant and free of irregularities.

Hand railings must be S4S lumber and painted white.

Protective overhead covering of temporary pedestrian facilities must be plywood at least 3/4 inch thick or wood planking with a nominal thickness of 2 inches minimum.

12-7.03 CONSTRUCTION

Construct hand railings on each side of a temporary pedestrian facility as necessary to protect pedestrian traffic from hazards due to work activities or adjacent vehicular traffic.

Maintain temporary pedestrian facilities in good condition and keep them clear of obstructions.

12-7.04 PAYMENT

Not Used

^^

13 WATER POLLUTION CONTROL

05-30-14

04-19-13

Delete item 3 in the list in the 4th paragraph of section 13-1.01A.

Add to section 13-1.01A:

11-15-13

Comply with the Department's general permit issued by the State Water Resources Control Board for *Order No. 2012-0011-DWQ, NPDES No. CAS000003, National Pollutant Discharge Elimination System (NPDES) Permit, Statewide Storm Water Permit and Waste Discharge Requirements (WDRs) for the State of California, Department of Transportation (Caltrans)*. The Department's general permit governs stormwater and nonstormwater discharges from the Department's properties, facilities, and activities. The Department's general permit may be viewed at the Web site for the State Water Resources Control Board, Storm Water Program, Caltrans General Permit.

Add to the list in the 1st paragraph of section 13-1.01D(3)(b):

10-21-11

3. Have completed SWRCB approved QSD training and passed the QSD exam

Add to the list in the 2nd paragraph of section 13-1.01D(3)(b):

10-21-11

3. Have completed SWRCB approved QSP training and passed the QSP exam

Replace "NEL violation" in item 3.6.2 in the list in the 1st paragraph of section 13-1.01D(3)(c) with:

04-19-13

receiving water monitoring trigger

Replace the 1st paragraph in section 13-2.01B with:

04-19-13

Within 7 days after Contract approval, submit 2 copies of your WPCP for review. Allow 5 business days for review.

After the Engineer authorizes the WPCP, submit an electronic copy and 3 printed copies of the authorized WPCP.

If the RWQCB requires review of the authorized WPCP, the Engineer submits the authorized WPCP to the RWQCB for its review and comment. If the Engineer orders changes to the WPCP based on the RWQCB's comments, amend the WPCP within 3 business days.

Replace the 1st paragraph in section 13-3.01B(2)(a) with:

04-19-13

Within 15 days of Contract approval, submit 3 copies of your SWPPP for review. The Engineer provides comments and specifies the date when the review stopped if revisions are required. Change and resubmit a revised SWPPP within 15 days of receiving the Engineer's comments. The Department's review resumes when a complete SWPPP has been resubmitted.

When the Engineer authorizes the SWPPP, submit an electronic copy and 4 printed copies of the authorized SWPPP.

If the RWQCB requires review of the authorized SWPPP, the Engineer submits the authorized SWPPP to the RWQCB for its review and comment. If the Engineer requests changes to the SWPPP based on the RWQCB's comments, amend the SWPPP within 10 days.

Replace "NELs" in item 3.1 in the 3rd paragraph of section 13-3.01B(2)(a) with:

04-19-13

receiving water monitoring triggers

Replace section 13-3.01B(6)(c) with:

04-19-13

13-3.01B(6)(c) Receiving Water Monitoring Trigger Report

Whenever a receiving water monitoring trigger is exceeded, notify the Engineer and submit a receiving water monitoring trigger report within 48 hours after conclusion of a storm event. The report must include:

1. Field sampling results and inspections, including:
   - 1.1. Analytical methods, reporting units, and detection limits
   - 1.2. Date, location, time of sampling, visual observation and measurements
   - 1.3. Quantity of precipitation from the storm event
2. Description of BMPs and corrective actions

Replace "NEL" in the 6th paragraph of section 13-3.01C(1) with:

04-19-13

receiving water monitoring trigger

Replace section 13-3.01C(3) with:

04-19-13

13-3.01C(3) Receiving Water Monitoring Trigger

For a risk level 3 project, receiving water monitoring triggers must comply with the values shown in the following table:

Receiving Water Monitoring Trigger

Parameter	Test method	Detection limit (min)	Unit	Value
pH	Field test with calibrated portable instrument	0.2	pH	Lower limit = 6.0 Upper limit = 9.0
Turbidity	Field test with calibrated portable instrument	1	NTU	500 NTU max

The storm event daily average for storms up to the 5-year, 24-hour storm must not exceed the receiving water monitoring trigger for turbidity.

The daily average sampling results must not exceed the receiving water monitoring trigger for pH.

04-19-13

Delete "and NELs are violated" in the 3rd paragraph of section 13-3.03C.

Replace "working days" at each occurrence in section 13-3.04 with.

10-19-12

original working days

04-19-13

Delete the 1st sentence in the 2nd paragraph of section 13-4.03C(3).

Add between the 2nd and 3rd paragraphs of section 13-4.03C(3):

04-19-13

Manage stockpiles by implementing water pollution control practices on:

1. Active stockpiles before a forecasted storm event
2. Inactive stockpiles according to the WPCP or SWPPP schedule

05-30-14

Delete the 7th paragraph of section 13-4.03C(3).

Replace the heading of section 13-4.03E(1) with:

05-30-14

General

05-30-14

Delete the 1st through 5th sentences in the 2nd paragraph of section 13-4.03E(1).

Replace the 1st sentence of the 1st paragraph of section 13-4.03E(3) with:

05-30-14

Limit vehicle and equipment cleaning or washing at the job site to that needed for safety and protection of the equipment and compliance with PLACs.

Replace the paragraph in section 13-4.04 with:

04-20-12

Not Used

Replace "20-7.02D(6)" in section 13-5.02C with:

07-19-13

20-5.03E

10-19-12

Delete "or stockpile" in the 3rd paragraph of section 13-5.02F.

Replace "20-7.03I(10)" in section 13-5.03C with:

07-19-13

20-5.03E(3)

Replace section 13-5.03F with:

04-20-12

13-5.03F Reserved

10-19-12

Delete "or stockpile" in item 1 in the list in the 1st paragraph of section 13-5.03K.

10-19-12

Delete the 3rd paragraph of section 13-5.03K.

Replace the 2nd sentence in the 1st paragraph of section 13-9.01A with:

10-19-12

You may use any of the following systems for temporary concrete washout:

1. Temporary concrete washout facility
2. Portable temporary concrete washout
3. Temporary concrete washout bin

Replace the 2nd paragraph of section 13-9.01B with:

10-19-12

Retain and submit an informational submittal for records of disposed concrete waste.

10-19-12

Delete the 4th paragraph of section 13-9.01B.

10-19-12

Delete "if authorized" in the 1st sentence in the 1st paragraph of section 13-9.02A.

Replace "at least 3-inch" in the 3rd sentence in the 1st paragraph of section 13-9.02A with:

10-19-12

6-inch

^^

14 ENVIRONMENTAL STEWARDSHIP

05-30-14

Replace section 14-9.03 with:

05-30-14

14-9.03 RESERVED

^^

15 EXISTING FACILITIES

07-19-13

Replace section 15-1.03D with:

07-19-13

15-1.03D Reserved

Replace "metal beam guard railing" in the 1st paragraph of section 15-2.01C with:

07-19-13

guardrail

Replace the paragraphs of section 15-2.02B(1) with:

07-19-13

Section 15-2.02B includes specifications for removing pavement, base, subbase, and subgrade.

If only a portion of the pavement is removed, saw-cut the outline of the removal area on a neat line and with a power-driven saw before removing.

For asphalt concrete pavement, saw cuts must be at least 2 inches deep unless otherwise described.

Replace section 15-2.02B(4)(b) with:

07-19-13

15-2.02B(4)(b) Reserved

Add to section 15-2.02B:

07-19-13

15-2.02B(5) Remove Concrete Pavement

15-2.02B(5)(a) General

Remove only the portion of pavement to be replaced or repaired during the same lane closure. If there is overlying material on the concrete pavement, remove it with the pavement.

Do not impact the surface within 18 inches of the pavement to remain in place. Use removal methods that do not damage the remaining pavement and base. Slab-lifting equipment must attach to the pavement.

Instead of disposing of removed concrete pavement by removing it from the job site, you may dispose of it under section 15-3.01.

15-2.02B(5)(b) Saw Cuts

Saw cut using a diamond blade and make cuts perpendicular to the pavement surface. Saw cutting is not required where concrete pavement is adjacent to asphalt concrete pavement.

Saw cut (1) no more than 2 days before removing pavement and (2) such that traffic will not dislodge any pavement piece or segment. Saw cut perpendicular to the traveled way except you may cut parallel or diagonal to the traveled way when removing the pavement during the same lane closure as the saw cutting.

You may make additional saw cuts within the sawed outline.

Saw cuts must be the full depth of the pavement unless otherwise shown.

Saw cut at longitudinal and transverse joints to remove entire slabs. For partial-slab areas, the Engineer determines the exact saw-cut locations.

15-2.02B(5)(c) Reserved

15-2.02B(6) Reserved

15-2.02B(7) Payment

Reserved

Replace section 15-2.02G with:

07-19-13

15-2.02G Remove Guardrail

Where removing guardrail, remove any concrete anchors and steel foundation tubes.

Replace the 1st paragraph of section 15-2.02K with:

07-19-13

Box culverts, concrete pipes, inlets, headwalls, and endwalls must be completely removed if any portion of these structures is (1) within 3 feet of the grading plane in excavation areas, (2) within 1 foot of original ground in embankment areas, or (3) shown to be removed.

Replace "Metal beam guard railing" in the table in the 2nd paragraph of section 15-2.03A(2)(a) with:

07-19-13

Guardrail

Replace the heading of section 15-2.03B with:

07-19-13

Salvage Guardrail

Replace the heading of section 15-2.04D with:

07-19-13

Reconstruct Guardrail

Replace section 15-2.09D with:

07-19-13

15-2.09D Reserved

Replace the 4th paragraph of section 15-2.10B with:

01-18-13

Instead of using new materials similar in character to those in the existing structure, you may use raising devices to adjust a manhole to grade. Before starting paving work, measure and fabricate raising devices. Raising devices must:

1. Comply with the specifications for section 75 except that galvanizing is not required
2 Have a shape and size that matches the existing frame
3. Be match marked by painting identification numbers on the device and corresponding structure
4. Result in an installation that is equal to or better than the existing one in stability, support, and nonrocking characteristics
5. Be fastened securely to the existing frame without projections above the surface of the road or into the clear opening

Replace the heading of section 15-2.10D with:

07-19-13

Adjust Guardrail

Replace the paragraphs of section 15-3.01 with:

07-19-13

Section 15-3 includes specifications for removing all or a portion of a concrete facility.

Concrete facilities include curbs, gutters, gutter depressions, sidewalks, driveways, slope paving, island paving, barriers, retaining walls, sound walls, minor structures, aprons, spillways, and dams.

Where broken-concrete slope protection is shown, use removed concrete for the construction of the broken-concrete slope protection.

Instead of disposing of removed concrete by removing it from the job site, you may dispose of it on the job site by one of the following methods:

1. Burying it in embankments at authorized locations. Removed concrete must be broken into pieces that can be readily handled and incorporated into embankments and placed at a depth of at least 3 feet below finished grade and slope lines. Concrete must not be buried in areas where piling is to be placed or within 10 feet of trees, pipelines, poles, buildings or other permanent objects or structures.
2. Placing it at authorized locations. The removed concrete must not present an unsightly appearance from the highway.

Replace the paragraph of section 15-3.02 with:

07-19-13

Not Used

07-19-13

Delete the 5th paragraph of section 15-3.03.

Add to the end of section 15-4.01A(2):

04-19-13

Allow 20 days for review of the bridge removal work plan.

Replace the 1st paragraph of section 15-5.01C(1) with:

10-19-12

Before starting deck rehabilitation activities, complete the removal of any traffic stripes, pavement markings, and pavement markers.

Replace the 2nd and 3rd paragraphs of section 15-5.01C(2) with:

10-19-12

Perform the following activities in the order listed:

1. Abrasive blast the deck surface with steel shot. Perform abrasive blasting after the removal of any unsound concrete and placement of any rapid setting concrete patches.
2. Sweep the deck surface.
3. Blow the deck surface clean using high-pressure air.

Replace the 2nd paragraph of section 15-5.01C(4) with:

10-19-12

Before removing asphalt concrete surfacing, verify the depth of the surfacing at the supports and midspans of each structure (1) in each shoulder, (2) in the traveled way, and (3) at the roadway crown, if a crown is present.

04-19-13

Delete "and concrete expansion dams" in the 3rd paragraph of section 15-5.01C(4).

Replace the 2nd paragraph of section 15-5.03A(2) with:

10-19-12

For a contract with less than 60 original working days, submit certificates of compliance for the filler material and bonding agents.

Replace "51-1.02C" in the 1st paragraph of section 15-5.03B with:

04-19-13

51-1.02F

Replace the 4th paragraph of section 15-5.03B with:

10-19-12

For a contract with less than 60 original working days, alternative materials must be authorized before use.

Add between the 5th and 6th paragraphs of section 15-5.03C:

10-19-12

The final surface finish of the patched concrete surface must comply with section 51-1.03F.

10-19-12

Delete the 4th paragraph of section 15-5.05C.

Replace "51-1.03F(5)" in the 3rd paragraph of section 15-5.06C(1) with:

07-19-13

51-1.01D(4)(b)

Replace "51-1.03E(5)" in the 5th paragraph of section 15-5.06C(1) with:

10-19-12

51-1.03F(5)

10-19-12

Delete the 9th paragraph of section 15-5.06C(1).

04-19-13

Delete the 15th paragraph of section 15-5.06C(1).

Add between the 18th and 19th paragraphs of section 15-5.06C(1):

07-19-13

Texture the polyester concrete surface before gelling occurs by longitudinal tining under 51-1.03F(5)(b)(iii), except do not perform initial texturing.

Replace section 15-5.06C(2) with:

04-19-13

15-5.06C(2) Reserved

04-19-13

Delete the 3rd paragraph of section 15-5.06D.

Replace the 1st paragraph in section 15-5.07B(4) with:

10-19-12

Payment for furnishing dowels is not included in the payment for core and pressure grout dowel.

Replace section 15-5.09 with:

04-19-13

15-5.09 POLYESTER CONCRETE EXPANSION DAMS

15-5.09A General

Section 15-5.09 includes specifications for constructing polyester concrete expansion dams.

Polyester concrete expansion dams must comply with the specifications for polyester concrete overlays in section 15-5.06, except a trial slab is not required.

Reinforcement must comply with section 52.

15-5.09B Materials

Not Used

15-5.09C Construction

For new asphalt concrete overlays, place the asphalt concrete overlay before starting polyester concrete activities. Saw cut and remove asphalt concrete at expansion dam locations.

For existing asphalt concrete overlays, remove expansion dams and asphalt concrete to the limits shown. Removing expansion dams must comply with section 15-4 except a bridge removal work plan is not required.

Where a portion of the asphalt concrete overlay is to remain, saw cut a 2-inch-deep neat line along the edge to remain in place before removing the asphalt concrete. Do not damage the existing surfacing to remain in place.

Prepare the deck surface under section 15-5.01C(2).

You may use a mechanical mixer to mix the polyester concrete for expansion dams. The mixer capacity must not exceed 9 cu ft unless authorized. Initiate the resin and thoroughly blend it immediately before mixing it with the aggregate. Mix the polyester concrete for at least 2 minutes before placing.

The application rate of methacrylate resin must be approximately 100 sq ft/gal.

You may place and finish expansion dams using hand methods.

Protect expansion dams from moisture, traffic, and equipment for at least 4 hours after finishing.

For expansion dams over 6 feet long, install 1/4-inch-wide joint material at 6-foot intervals across the width of the expansion dam. Joint material must be either expanded polyurethane or expanded polyethylene.

15-5.09D Payment

Not Used

Add to section 15-6.01A(3)(a):

07-19-13

Within 5 days of completing annular space grouting at a culvert, submit the grouting records.

Replace "41-1.01" in item 10.3 in the list in the 2nd paragraph of section 15-6.01A(3)(d) with:

07-19-13

41-2

Replace "41-1.02" in 1st paragraph of section 15-6.01B(2) with:

07-19-13

41-2

Replace the heading of section 15-6.04 with:

01-18-13

INVERT PAVING

Replace the 1st paragraph of section 15-6.13A(1) with:

07-19-13

Section 15-6.13 includes specifications for installing machine spiral wound PVC pipeliners directly into the culvert.

Replace the heading of section 15-6.13B with:

07-19-13

Machine Spiral Wound PVC Pipeliners, Grouted

^^^

18-1.01B Definitions

Reserved

18-1.01C Submittals

If a dust suppressant or dust control binder is to be used, submit a dust treatment plan at least 15 days before starting job site activities. The dust treatment plan must include:

1. Product name and type
2. Manufacturer's name
3. Polymer emulsion type if synthetic polymer emulsion is used, including identification of:
   - 3.1. Individual components greater than 5 percent by volume in blends of polymers of different compositions
   - 3.2. Additives greater than 2 by volume
4. MSDS
5. Proposed methods for applying products
6. Application rates and number of passes
7. Required weather conditions for application, including ambient and surface temperatures, wind conditions, and allowable period before expected precipitation
8. Drying time or curing time required before traffic is allowed on the treated surface

Submit the manufacturer's instructions for the material to be used as an informational submittal.

Submit a certificate of compliance for the dust suppressant, dust control binders, and fibers.

For dust suppressants, include with the certificate of compliance:

1. Test results verifying compliance with the quality characteristic requirements in section 18-1.01D. The results must be from a test conducted within 6 months before the date of the certificate of compliance.
2. Test results from a test conducted within 2 years before the date of the certificate of compliance verifying compliance with the following environmental requirements:
   - 2.1. Maximum constituent concentration levels
   - 2.2. US EPA regulatory requirements for:
     - 2.2.1. Volatile organic compounds
     - 2.2.2. Semivolatile organic compounds
     - 2.2.3. Toxicity characteristic leaching
     - 2.2.3. Modified synthetic leaching procedure
   - 2.3. Aquatic toxicity

18-1.01D Quality Control and Assurance

Dust palliatives must comply with US EPA requirements and RWQCB requirements for soil stabilizers.

Dust suppressants must be tested by an EPA-accredited laboratory. Liquid chemical treatments must be tested before dilution. Solid products must be mixed with water to a 25 percent concentration before testing. The chemical constituent concentration for each dust suppressant must not exceed the maximum levels shown in the following table:

Maximum Constituent Concentration Levels

Constituent	Test method	Requirement maximum level (ppm)
Arsenic	EPA Method 200.7	5.0
Barium		100.0
Cadmium		0.2
Chromium		1.0
Copper		1.0
Lead		1.0
Mercury	EPA Method 245.1	0.05
Selenium	EPA Method 200.7	5.0
Zinc		10.0
Phosphorus	EPA Method 365.4	2500.0
Cyanide	EPA Method 335.4	0.2

Dust suppressants must comply with the US EPA requirements for the quality characteristics when tested under the test methods shown in the following table:

Quality characteristic	Test method
Volatile organic compounds (VOC)	EPA Method 8260
Semivolatile organic compounds (SVOC)	EPA Method 8270
Toxicity characteristic leaching procedure	EPA Method 1311
Modified synthetic leaching procedure	EPA Method 1312

The aquatic toxicity for dust suppressant must comply with the requirements shown in the following table:

Aquatic Toxicity Requirements

Quality characteristic	Test method	Requirement
Aquatic toxicity[a] (LC50 min, ppm)	ASTM E729 or EPA Method 600/4-90/027F and EPA Method 600/4-91/002	10
Aquatic toxicity[a] (rating)	ASTM E729 or EPA Method 600/4-90/027F and EPA Method 600/4-91/002	slightly toxic or better
Renewal toxicity[b] (LC50 min, ppm)	ASTM E1295	10
Renewal toxicity[b] (rating)	ASTM E1295	slightly toxic or better

[a]Using *Ceriodaphnia dubia* (water flea), *Oncorhynchus mykiss* (rainbow trout), *Pimephales promelas* (fathead minnow), and *Americamysis bahia* (mysid shrimp)
[b]Using *Ceriodaphnia dubia* (water flea)

18-1.02 MATERIALS

18-1.02A General

Dust suppressants and control binders must be either (1) miscible in water or (2) a material that is directly applied to the surface without mixing with water.

18-1.02B Dust Suppressants

18-1.02B(1) General

Dust suppressants must be one of the following:

1. Petroleum-based organic product
2. Nonpetroleum-based organic product

3. Hygroscopic product
4. Synthetic polymer emulsions

18-1.02B(2) Petroleum-Based Organic Products

Petroleum-based organic dust suppressants must be asphalt emulsion, petroleum resin, base oil, mineral oil, or synthetic fluid.

Asphalt emulsion must be Grade SS1h.

Petroleum resin must comply with the requirements shown in the following table:

Petroleum Resin Requirements

Quality characteristic	Test method	Requirement
Residue (min, %)	ASTM D6934	60
pH	ASTM D1429	4.0–7.0
Specific gravity at 16 °C (min)	ASTM D1298	1.00
Kinematic viscosity at 25 °C (min, Saybolt Furol seconds [a])	ASTM D2170	188
Flash point (min °C)	ASTM D92	205
Particle charge test	ASTM D7402	Positive

[a] Use ASTM D2161 to convert the mm^2/s value to Saybolt Furol seconds

Base and mineral oils must comply with the requirements shown in the following table:

Base and Mineral Oils Requirements

Quality characteristic	Test method	Requirement
Base and mineral oil content (min, %)	-	75
Specific gravity at 16 °C (min)	ASTM D1298	0.85–0.90
Brookfield absolute viscosity at 68 °C (max, cP)	ASTM D2196	250
Flash point (min, °C)	ASTM D93	150

Synthetic fluids must comply with 40 CFR 35 and the requirements shown in the following table:

Synthetic Fluids Requirements

Quality characteristic	Test method	Requirement
Base and mineral oil content (min, %)	--	75
Specific gravity at 16 °C (min)	ASTM D1298	0.85–0.90
Brookfield absolute viscosity at 68 °C (max, cP)	ASTM D2196	250
Flash point (min, °C)	ASTM D93	150

18-1.02B(3) Nonpetroleum-Based Organic Products

Nonpetroleum-based organic dust suppressants must be lignosulfonate, plant oil, or tall oil pitch rosin.

Lignosulfonate must comply with the requirements shown in the following table:

Lignosulfonate Requirements

Quality characteristic	Test method	Requirement
Lignin sulfonate content ready to use (min, %)	ASTM D4900	25
Residue total solids content (min %)	ASTM D4903 or D2834	52
Lignin sulfonate content of residue (min, %)	--	50
Reducing sugars content of residue (min, %)	ASTM D5896 or D6406	25
pH	ASTM D1293	6.0–9.0
Specific gravity (min)	ASTM D1429	1.20
Brookfield absolute viscosity at 25° C (max, cP)	ASTM D2196	1,000

Plant oil must comply with the requirements shown in the following table:

Plant Oil Requirements

Quality characteristic	Test method	Requirement
Residue active solids content (min, %)	ASTM D4903	50
Specific gravity (min)	ASTM D1429	0.93
Brookfield viscosity (cP)	ASTM D2196	48

Tall oil pitch rosin must comply with the requirements shown in the following table:

Tall Oil Pitch Rosin Requirements

Quality characteristic	Test method	Requirement
Rosin acid content (min, %)	ASTM D1240	10
Residue active solids content (min, %)	ASTM D2834	45
pH	ASTM D1293	3.0–9.0
Specific gravity (min)	ASTM D1429	1.00
Brookfield absolute viscosity at 25 °C (cP)	ASTM D2196	50–200

18-1.02B(4) Hygroscopic Products

Hygroscopic dust suppressants must be calcium chloride, calcium chloride flake, or magnesium chloride.

Calcium chloride must comply with the requirements shown in the following table:

Calcium Chloride[a] Requirements

Quality characteristic	Test method	Requirement
Calcium chloride content (%)	ASTM E449	28–42
Total magnesium as $MgCl_2$ (max, %)	ASTM E449	6.0
Total alkali chlorides as NaCl (max, %)	ASTM E449	6.0
Calcium hydroxide content (max, %)	ASTM E449	0.2
pH with 5 percent solution	ASTM D1293	7.0–9.0
Specific gravity	ASTM D1429	1.28–1.44

[a]ASTM D98 or AASHTO M144

Calcium chloride flake must comply with the requirements shown in the following table:

Calcium Chloride Flake[a] Requirements

Quality characteristic	Test method	Requirement
Calcium chloride content (%)	ASTM E449	28–42
Total magnesium as $MgCl_2$ (max, %)	ASTM E449	6.0
Total alkali chlorides as NaCl (max, %)	ASTM E449	6.0
Calcium hydroxide content (max, %)	ASTM E449	0.2
pH with 5 percent solution	ASTM D1293	7.0–9.0
Gradation percent passing 3/8–inch sieve #4 sieve #30 sieve	ASTM C136	 100 80–100 0–5

[a]ASTM D98 or AASHTO M144

Magnesium chloride must comply with the requirements shown in the following table:

Magnesium Chloride Requirements

Quality characteristic	Test method	Requirement
Magnesium chloride content (%)	ASTM D4691 or ASTM D511[a]	28–33
Sulfate content as magnesium sulfate (max, %)	ASTM D4691[a]	4.0
Potassium content as potassium chloride (max, %)	ASTM E449	0.5
Sodium chloride content (max, %)	ASTM E449	1.0
pH with 5% solution	ASTM D1293	7.0–9.0
Specific gravity	ASTM D1429	1.31 ± 0.02

[a]You may use another appropriate atomic absorption spectrophotometry method such as that in *Standard Methods for the Examination of Water and Waste Water* by APHA-AWWA-WPCF.

18-1.02B(5) Synthetic Polymer Emulsions

Synthetic polymer emulsions must comply with the requirements shown in the following table:

Synthetic Polymer Emulsion Requirements

Quality characteristic	Test method	Requirement
Residue active solids content (min, %)	ASTM D2834	40
pH	ASTM D1429	4.0–9.5
Specific gravity at 16 °C	ASTM D1298	1.00–1.15
Brookfield absolute viscosity (max, cP)	ASTM D2196	1,000
Polymer film tensile strength – dry (psi)	ASTM D412	500
Retained coagulum on #100 sieve (max, %)	ASTM D1417	0.1
Ash content (max, %)	ASTM D5040	2

18-1.02C Dust Control Binders

Dust control binders must comply with the specifications for a general purpose tackifier in section 21-1.02F(1).

Fibers must comply with section 21-1.02E.

18-1.03 CONSTRUCTION

18-1.03A General

Monitor dust conditions and apply dust palliative for dust control as described and as ordered. Reapply dust palliative at any time to control dust.

Apply a dust suppressant to:

1. Temporary haul roads
2. Construction staging, material storage, and layout areas
3. Compacted soil or aggregate base roads or driveways
4. Paved surfaces

Apply a dust control binder to:

1. Rough–graded soils
2. Completed slopes
3. Soil stockpiles unless another practice is already used

Do not use a dust suppressant or dust control binder within 100 feet of a wetland or body of water.

18-1.03B Equipment

Apply dust suppressants that are miscible in water with either (1) a pressure-type water distributor truck equipped with a spray system or (2) a pressure-type asphalt distributor truck as specified in section 93-1.03C.

Apply dust suppressant flakes to the surface using a spreader or spinner disk.

Apply dust control binders with either (1) a pressure-type water distributor truck equipped with a spray system or (2) hydraulic spray equipment as specified for applying hydromulch in section 21-1.03E.

18-1.03C Mixing and Application Rates

Use the mix proportions and application rate for the corresponding dust suppressant as shown in the following table:

Dust suppressant	Mix proportions	Application rate
Asphaltic emulsion, Grade SS1H	5 parts water to 1 part emulsion	0.20–1.0 gal/sq yd
Petroleum resin emulsion	5 parts water to 1 part emulsion	0.20–1.0 gal/sq yd
Base and mineral oil	Apply undiluted	0.30–0.35 gal/sy yd
Lignosulfonate	1 part water to 1 part concentrate	1.0 gal/sq yd
Plant oil	Apply undiluted	0.25–0.50 gal/sq yd
Tall oil pitch rosin	5 parts water to 1 part emulsion for clayey soil and 10 parts water to 1 part emulsion for sandy soil.	0.30–1.0 gal/sq yd
Calcium chloride solution (Hygroscopic)	Apply undiluted	0.20–0.35 gal/sq yd
Calcium chloride flakes (Hygroscopic)	--	1.0–1.5 lb/sq yd
Magnesium chloride (Hygroscopic)	Apply undiluted	0.30–0.50 gal/sq yd
Synthetic polymer emulsion	9 parts water to 1 part concentrate	0.50 gal/sq yd

Apply hygroscopic materials under the manufacturer's instructions.

Apply calcium chloride flakes to a moist surface.

Allow surfaces treated with a dust suppressant to cure before opening to traffic.

Use the mix proportions and application rate for the corresponding dust control binder as shown in the following table:

Dust control binder	Mix proportions	Application rate
Guar	11 to 15 pounds per 1,000 gallons of water	44–59 lb/acre
Psyllium	Enough water to allow for uniform slurry flow	80–200 lb/acre
Starch	Manufacturer's recommended mix proportions with water	150 lb/acre
Liquid acrylic copolymers and polymers[a]	10 parts water to 1 part polymer	1,175 gal/acre
Liquid methacrylate and acrylate polymers	Manufacturer's recommended mix proportions with water	20 gal/acre
Copolymers of sodium acrylates and acrylamides	Manufacturer's recommended mix proportions with water	3–10 lb/acre
Polyacrylamide and copolymer of acrylamide	10 pounds per 1,000 gallons of water	5 lb/acre
Hydro-colloid polymers	Manufacturer's recommended mix proportions with water	54–64 lb/acre

[a]Mix and handle the polymeric compound in a manner that will not cause foaming. You may add an antifoaming agent.

Do not allow stormwater runoff from polyacrylamide treated soils unless water passes through:

1. Sediment basin if the total drainage area is greater than or equal to 5 acres.
2. Sediment trap or a series of check dams if the total drainage area is less than 5 acres. Maximize the number of check dams used and space them evenly in the drainage channel so as to maximize sediment settlement.

You may add fibers to dust control binders at a rate of 2,000 lb/acre.

You may use reduced application rates when reapplying dust palliatives if authorized.

18-1.04 PAYMENT

Not Used

^^

19 EARTHWORK

07-19-13

Replace "20-3.03B(4)" in the 2nd paragraph of section 19-1.01A with:

07-19-13

20-2.02C(2)

Replace the 3rd paragraph in section 19-2.01A with:

07-19-13

Pavement removal within the limits of roadway excavation must comply with section 15-2.02B.

07-19-13

Delete the 2nd paragraph in section 19-2.03A.

Replace the 2nd paragraph of section 19-3.01A(2)(b) with:

07-01-11

For cofferdams on or affecting railroad property, allow 85 days for review.

Add to the list in the 1st paragraph of section 19-3.01A(2)(d):

01-20-12

9. Provisions for discontinuous rows of soil nails

Replace "sets" in the 3rd and 4th paragraphs of section 19-3.01A(2)(d) with:

04-19-13

copies

Add to section 19-3.01A(3)(b):

01-20-12

For soil nail walls, wall zones are specified in the special provisions.

For ground anchor walls, a wall zone is the entire wall unless otherwise specified in the special provisions.

01-20-12

Delete the 2nd sentence in the 4th paragraph of section 19-3.01A(3)(b).

Replace "90" in the paragraph of section 19-3.02G with:

01-18-13

90-1

Add to section 19-3.02:

07-19-13

19-3.02I Filter Fabric

Filter fabric must be Class A.

Replace the heading of section 19-3.03C with:

04-19-13

19-3.03B(4) Cofferdams

Replace the heading of section 19-3.03D with:

04-19-13

19-3.03B(5) Water Control and Foundation Treatment

Replace the 1st paragraph of section 19-3.03E(3) with:

01-20-12

Compact structure backfill behind lagging of soldier pile walls by hand tamping, mechanical compaction, or other authorized means.

Add to the end of section 19-3.03E(3):

07-19-13

If filter fabric is shown behind the lagging:

1. Immediately before placing the filter fabric, remove any loose or extraneous material and sharp objects from the surface to receive the filter fabric.
2. Handle and place the filter fabric under the manufacturer's instructions. Stretch, align, and place the fabric without wrinkling.
3. Stitch the adjacent borders of filter fabric or overlap the adjacent borders by 12 to 18 inches. If stitching the border, use yarn of a contrasting color. Yarn size and composition must be as recommended by the fabric manufacturer. Use 5 to 7 stitches per inch of seam.
4. Repair any damaged filter fabric by placing a piece of filter fabric large enough to cover the damaged area and comply with the overlapping or stitching requirements.

Replace the 2nd paragraph of section 19-3.03F with:

01-20-12

Do not backfill over or place material over slurry cement backfill until 4 hours after placement. When concrete sand is used as aggregate and the in-place material is free draining, you may start backfilling as soon as the surface water is gone.

Add between the 2nd and 3rd paragraphs of section 19-3.03K:

01-20-12

Before you excavate for the installation of ground anchors in a wall zone:

1. Complete stability testing
2. Obtain authorization of test data

Replace the 2nd sentence of the 7th paragraph of section 19-3.03K:

01-20-12

Stop construction in unstable areas until remedial measures have been taken. Remedial measures must be submitted and authorized.

Add between the 8th and 9th paragraphs of section 19-3.03K:

01-20-12

When your excavation and installation methods result in a discontinuous wall along any soil nail row, the ends of the structurally completed wall section must extend beyond the ends of the next lower excavation lift by a distance equal to twice the lift height. Maintain temporary slopes at the ends of each wall section to ensure slope stability.

Replace the 9th paragraph of section 19-3.03K:

01-20-12

Do not excavate to the next underlying excavation lift until the following conditions have been attained for the portion of the soil nail or ground anchor wall in the current excavation lift:

1. Soil nails or ground anchors are installed and grouted.
2. Reinforced shotcrete facing is constructed.

01-18-13

3. Grout and shotcrete have cured for at least 72 hours.

01-20-12

4. Specified tests are complete for that portion of wall and the results are authorized.
5. Soil nail facing anchorages are attached or ground anchors are locked off.

Replace the 2nd sentence in the 7th paragraph of section 19-3.04 with:

01-18-13

Structure excavation more than 0.5 foot from the depth shown is paid for as a work-character change if you request an adjustment or the Engineer orders an adjustment.

Replace "Contract completion time" in the 8th paragraph of section 19-6.03D with:

10-19-12

work completion date

Add to section 19:

01-18-13

19-10–19-20 RESERVED

^^

20 LANDSCAPE

05-30-14

Replace the headings and paragraphs in section 20 with:

07-19-13

20-1 GENERAL

20-1.01 GENERAL

20-1.01A Summary

Section 20-1 includes general specifications for performing landscaping.

If an irrigation system is to be installed in an existing planting area to be maintained, check for plant deficiencies under section 20-3.02A(4) before starting irrigation work.

Perform a functional test for each irrigation system under 20-2.01A(4)(d):

1. Before planting the plants
2. After planting the plants
3. Before the start of the plant establishment work

If a plant is to be transplanted or an irrigation component is to be relocated, transplant plant or protect irrigation components before performing other construction activities in the area.

Perform roadside clearing:

1. As required to prepare the job site for construction work
2. Until the start of the plant establishment work or Contract acceptance, whichever comes first

20-1.01B Definitions

Reserved

20-1.01C Submittals

At least 15 days before applying any pesticide, submit a copy of the licensed pest control adviser's recommendation.

At the end of each week, submit a report documenting the application of all pesticides as an informational submittal. Use form *Report of Chemical Spray Operations*.

Before mixing a pesticide, submit a copy of the registered label for the pesticide as an informational submittal. If unable to copy, allow the Engineer to read the label on the container.

20-1.01D Quality Control and Assurance

20-1.01D(1) General

Obtain a recommendation from a licensed pest control adviser for the use of all pesticides under the Food & Agri Code. The recommendation must include the pesticides to be used, rates of application, methods of application, and application areas.

The pesticide applicator must have an active and valid qualified applicator license or certificate from the Department of Pesticide Regulation.

20-1.01D(2) Progress Inspections

The Engineer will perform progress inspections before:

1. Cultivating work starts
2. Pressure testing of irrigation pipe on the supply side of control valves
3. Testing of low voltage conductors
4. Planting work starts
5. Completion of planting work

Notify the Engineer at least 4 business days before each inspection is required. Allow at least 3 business days for the Engineer's inspection.

Unless otherwise authorized, do not proceed with the next construction activity until the inspection has been completed and any required corrective work has been performed and authorized.

20-1.02 MATERIALS

20-1.02A General

Reserved

20-1.02B Water

Water available from an existing Department-owned facility within the project limits or an irrigation system to be installed under the Contract is furnished at no charge.

If water is not available, make arrangements for supplying water. Water must be of a quality that will promote plant growth.

20-1.02C Pesticides

Pesticides must comply with the Department of Pesticide Regulation.

Insecticide must be imidacloprid.

Rodenticides must be brodifacoum, bromadiolone, or diphacinone.

Do not use oil or pelleted forms of pesticides for weed control.

For weed control, use a pesticide with a photosensitive dye that produces a contrasting color when sprayed on the ground. The color must disappear between 2 to 3 days after being applied. The dye must not stain surfaces or injure plants or wildlife when applied at the manufacturer's recommended application rate.

20-1.03 CONSTRUCTION

20-1.03A General

Take precautions to prevent irrigation water from:

1. Wetting vehicles, pedestrians, and pavement
2. Eroding soil

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3. Causing excess runoff

Water plants under the Model Water Efficient Landscape Ordinance, 23 CA Code of Regs § 490 et seq., and local water agency requirements.

Water plants at night unless otherwise authorized.

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Dispose of removed, pruned, and damaged vegetative material.

You may reduce removed vegetative material to chips with a maximum thickness of 1/2 inch and spread within the job site at locations determined by the Engineer. Chipped material must not be substituted for wood mulch, nor must the chipped material be placed within areas to receive wood mulch.

20-1.03B Pesticides

Notify the Engineer of pesticide application times at least 24 hours before each application.

Mix and apply pesticides under the requirements of the Department of Pesticide Regulation and the instructions on the pesticide product label.

Do not apply pesticides:

1. On Saturdays and holidays unless authorized
2. Whenever weather and wind conditions are unsuitable for application
3. Within the plant basin
4. On the foliage and woody parts of the plant

If a granular preemergent is used, it must be covered with mulch on the same work day. Do not apply granular preemergent in plant basins.

Do not apply preemergents:

1. To groundcover plants before the plants have been planted a minimum of 3 days and have been thoroughly watered
2. Within 18 inches of trees, shrubs, and seeded areas

20-1.03C Roadside Clearing

20-1.03C(1) General

Perform roadside clearing by:

1. Removing and disposing of trash and debris
2. Controlling the following pests:
   - 2.1. Rodents
   - 2.2. Insects
   - 2.3. Weeds
3. Removing existing plants as described

Control rodents by using rodenticides or traps.

20-1.03C(2) Remove Existing Plants

Remove existing plants as described. Removal of existing plants includes removing their stumps and roots 2 inches or larger in diameter to a minimum depth of 12 inches below finished grade. Backfill holes resulting from stump removal to finished grade with material obtained from adjacent areas.

If a plant is to be planted within existing groundcover area, remove existing groundcover from within an area 6 feet in diameter centered at each plant location.

20-1.03C(3) Weed Control

Control weeds by the use of pesticides, hand pulling, or mowing.

If pesticides are used to control weeds, apply pesticides before the weeds reach the seed stage of growth or exceed 4 inches in length, whichever occurs first. Do not use pesticides at cutting plant locations.

Where cuttings are to be planted, control weeds by hand pulling within an area 2 feet in diameter centered at each plant location.

If weeds are to be controlled by hand pulling, hand pull weeds before they reach the seed stage of growth or exceed 4 inches in length, whichever occurs first.

Where liner, plug, or seedling plants are to be planted 10 feet or more apart, control weeds by the use of pesticides or hand pulling within an area 2 feet in diameter centered at each plant location. Where liner, plug, or seedling plants are to be planted less than 10 feet apart, control weeds by the use of pesticides within the entire area.

Control weeds by mowing outside of mulched areas, plant basins, groundcover areas, and within areas to be seeded. Mowing must extend to the edges of pavement, dikes, curbs, sidewalks, walls, and fences.

If mowing is to be performed within areas to be seeded, perform mowing as needed until the start of the seeding operation specified in section 21.

Mowing must be performed before the weeds reach the seed stage of growth or exceed 6 inches in length, whichever occurs first. Mow weeds to a height of 3 inches.

20-1.03C(4) Disposal of Removed Groundcover, Weeds, and Mowed Material

Dispose of hand pulled weeds the same day they are pulled. Dispose of removed groundcover within 3 days.

Dispose of mowed material from the initial mowing. Disposal of material from subsequent mowing is not required.

20-1.03D Cultivation

Cultivation must be by mechanical methods and performed until the soil is in a loose condition to a minimum depth of 6 inches. Soil clods must not be larger than 2 inches in maximum dimension after cultivation.

The areas to be cultivated must extend 12 inches beyond the outer limit of each planting area requiring cultivation.

After initial cultivation, place soil amendment and fertilizer at specified rates.

Recultivate to thoroughly mix native soil and amendments.

Do not drive on cultivated areas after cultivation.

Planting areas that have been cultivated and become compacted must be recultivated.

Rocks and debris encountered during soil preparation in planting areas must be brought to the surface of the ground.

Remove rocks and debris as ordered. This work is change order work.

20-1.03E Weed Germination

Reserved

20-1.04 PAYMENT

Items paid for by area are measured parallel to the ground surface.

Planting areas that do not require cultivation but are within the cultivation areas will not be deducted.

20-2 IRRIGATION

20-2.01 GENERAL

20-2.01A General

20-2.01A(1) Summary

Section 20-2 includes specifications for installing irrigation systems.

The irrigation systems shown are diagrammatic.

20-2.01A(2) Definitions

Reserved

20-2.01A(3) Submittals

20-2.01A(3)(a) General

Submit shop drawings for the electrical components of the irrigation system except electrical service 30 days before installation. The drawings must:

1. Include schematic wiring diagrams showing wire sizes and routes between electrical components
2. Show conduit sizes
3. Bear the written approval of the controller manufacturer or the manufacturer's authorized agent
4. Be accompanied by:
   - 4.1. Colored wire and splice samples
   - 4.2. Manufacturer's descriptive and technical literature

After the work shown on the drawing is complete, submit 3 copies of the as-built shop drawings including any wire modifications for each controller installed.

For each controller, laminate and place in an envelope 1 copy of:

1. As-built schematic wiring diagram including wiring modifications
2. 11 by 17 inches as-built irrigation plan

The laminate must be clear, mat-finished plastic that is at least 10 mils thick. The envelope must be heavy-duty plastic.

Attach the envelope to the inside of the controller enclosure or cabinet door. If the door is not large enough to secure the envelope, submit the envelope and its contents.

20-2.01A(3)(b) Manufacturer's Instructions

Submit as an informational submittal the manufacturer's installation instructions 15 days before installing:

1. Couplings for conduits used for irrigation conduits
2. Plastic pipe and fittings
3. Solvent cement for plastic pipe and flexible hose
4. Sprinklers
5. Flow sensors

20-2.01A(3)(c) Maintenance and Operation Manuals

Before Contract acceptance, submit as an informational submittal a manufacturer's maintenance and operation manual for each type of controller installed.

20-2.01A(4) Quality Control and Assurance

20-2.01A(4)(a) General

Reserved

20-2.01A(4)(b) Pressure Testing

20-2.01A(4)(b)(i) General

Perform pressure testing for leakage on irrigation supply lines:

1. In the Engineer's presence

2. On business days between 8 a.m. and 5 p.m. unless authorized
3. Before backfilling supply line trenches
4. With irrigation system gate valves open
5. With open ends of the supply line and fittings plugged or capped

Notify the Engineer at least 48 hours before performing a pressure test.

Choose either Method A or B to test supply lines installed by trenching and backfilling and supply lines that are completely visible after installation.

All other supply lines, including those installed in the ground by methods other than trenching and backfilling must be tested by Method A.

Test irrigation supply line in conduit by Method A with the testing period modified to 0.5 hour and no allowable pressure drop.

20-2.01A(4)(b)(ii) Method A

Method A pressure testing procedures for leakage must comply with the following:

1. Pressure gauge must be calibrated from 0 to 200 psi in 5 psi increments and be accurate to within a tolerance of 2 psi.
2. Supply line must be filled with water and connected to a pressure gauge. Place the pipeline under a pressure of 125 psi. Remove the source of pressure and leave the line under the required pressure.
3. Test the supply line under the required pressure for a period of 1 hour. The pressure gauge must remain in place until each test period is complete.
4. Leaks that develop in the tested portion of the system must be located and repaired after each test period if a drop of more than 5 psi is indicated by the pressure gauge. After the leaks have been repaired, repeat the 1 hour pressure test until the drop in pressure is 5 psi or less.

If a system consists of a new supply line connected to an existing line, the new supply line must be isolated from the existing line and tested.

20-2.01A(4)(b)(iii) Method B

Method B pressure testing procedures for leakage must comply with the following:

1. Before any portion of the supply line on the upstream side of a control valve is backfilled, water must be turned on for that portion of the line and maintained at full pressure from the water source for a period not less than 8 consecutive hours after all air has been expelled from the line. Before any portion of the supply line on the downstream side of the control valve is backfilled, perform the same test for a period not less than 1 hour.
2. Repair leaks that develop in the tested portion of the system. After the leaks have been repaired, repeat the pressure test until no leaks occur as determined by the Engineer.

20-2.01A(4)(c) Sprinkler Coverage Check

After installation of the sprinklers, check and adjust the entire sprinkler system for proper orientation and uniform coverage.

20-2.01A(4)(d) Irrigation System Functional Tests

The functional tests for each irrigation controller or group of controllers and associated irrigation system served by a single electric service point must consist of at least 1 complete cycle of operation. The Engineer determines the length of the cycle.

Notify the Engineer at least 10 days before performing each functional test.

20-2.01A(4)(e) Final Irrigation System Check

Perform the final check of the existing and new irrigation system between 20 and 30 days before Contract acceptance. The Engineer determines the length of the cycle.

Remote control valves connected to existing and new irrigation controllers must be checked for automatic operation when the controllers are in automatic mode.

20-2.01B Materials

20-2.01B(1) General

Use minor concrete for replacing removed concrete facilities.

HMA for replacing removed asphalt concrete surfacing and facilities must comply with section 39. You may use minor HMA if authorized.

20-2.01B(2) Garden Valves

Each garden valve must:

1. Be inverted nose type and of brass or bronze construction with female thread inlet
2. Have a replaceable seat washer, rising valve stem within a protective collar, and male thread hose outlet
3. Have a loose key handle

20-2.01B(3) Recycled Water Identification

Irrigation components used for recycled water must be manufactured or painted purple. Recycled water irrigation pipe and tubing must have a permanent label with the wording "CAUTION RECYCLED WATER" every 24 inches in 2 rows spaced approximately 180 degrees apart in the longitudinal direction of the pipe or tubing.

The recycled water warning sign must be a decal or a decal attached to a 1/16-inch thick aluminum plate or tag.

Each warning sign decal must:

1. Show the phrase "Recycled Water, Do Not Drink" and the drinking glass graphic symbol
2. Be UV fade and weather resistant and manufactured from flexible vinyl with or without mylar
3. Have a purple background, black text, and self-adhesive backing

Each warning tag must:

1. Show the phrase "RECYCLED WATER" and the drinking glass graphic symbol
2. Be UV fade and weather resistant
3. Be purple, double-sided, and manufactured from polyurethane
4. Have an integral neck attachment and attachment hole capable of withstanding 178 lb of pull-out resistance
5. Have hot-stamped black lettering

Posts and hardware for warning signs must comply with section 56-4.

Concrete sprinkler protectors used with recycled water must be painted purple.

20-2.01B(4) Location Markers

Location markers must be schedule 40 white PVC plastic pipe.

20-2.01B(5) Pull Boxes

Pull boxes must comply with section 86-2.06 and be no. 5 or larger unless otherwise shown. Pull boxes for low voltage conductors must not have side openings.

Pull box covers used solely for irrigation electrical service must be marked "IRRIGATION".

20-2.01B(6) Unions

Unions must be brass or malleable iron capable of withstanding the maximum required working pressure.

20-2.01B(7) Valve Boxes and Covers

Valve boxes must be precast concrete.

Covers must be:

1. Concrete, steel, or cast iron.

2. Marked "WATER" in cast-in letters not less than 1 inch high.
3. 1 piece, except 2 pieces are required when the weight of the valve box cover exceeds 35 lb.

The valve box covers must include a polyurethane label with the appropriate controller letter and station number as shown.

20-2.01B(8) Wye Strainers

Wye strainers must:

1. Have a cast iron or all bronze body
2. Have a removable stainless steel strainer screen:
   - 2.1. With an open area equal to at least 3 times the cross-sectional area of the pipe based on an iron pipe size
   - 2.2. With 40-mesh woven wire, except:
     - 2.2.1. For a backflow preventer assembly, the screen must be 20-mesh woven wire mesh or perforated sheet with 0.045-inch diameter holes
     - 2.2.2. For a valve assembly, the screen must be 80-mesh woven wire mesh
3. Be capable of withstanding a working pressure of 150 psi
4. Be equipped with a garden valve at the outlet

The wye strainer filter housing must:

1. Withstand a working pressure of 150 psi
2. Be manufactured of reinforced polypropylene plastic

20-2.01C Construction

20-2.01C(1) General

05-30-14

Immediately shut off water to broken supply lines, valves, or sprinkler assemblies. Repair irrigation systems within 24 hours after a malfunction or damage occurs.

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Connect underground metallic pipes, valves, or fittings made of dissimilar metals through a dielectric coupling or bushing.

You may install conduits, conductors, and supply lines by methods other than trenching provided that they are not damaged and are installed at the depths specified.

20-2.01C(2) Trenching and Backfilling

Trench and backfill under section 86-2.01.

Remove plants under 20-1.03C as necessary to perform trenching. If plants are to remain, adjust trench alignment to minimize damage.

If removal of:

1. Turf is required, remove to a maximum width of 12 inches.
2. Groundcover is required, remove to a maximum width of 6 feet. Existing *Carpobrotus* and *Delosperma* may be rototilled if the backfill for the trenches does not contain plants longer than 6 inches in length.

Make a 2-inch deep sawcut along neat lines around the perimeter of the pavement to be removed at locations determined by the Engineer.

The trench must have uniform bearing throughout the entire length and must be free of jagged rubble or sharp objects. Ensure conduit, supply line, and joints are not moved or damaged by backfill operations.

For a project with multiple water service points, excavate and backfill trenches for 1 service point at a time.

11-15-13

Trenches for irrigation supply lines and conduits 3 inches and larger must be 5 times the pipe or conduit diameter deep and 2 times the pipe or conduit diameter wide.

Trenches for irrigation supply lines and conduits 2-1/2 inches or less in diameter must be a minimum of 12 inches below finished grade, measured from the top of the installed pipe.

07-19-13

Trenches must be at least 4 feet from curbs, dikes, and paved shoulders.

Rocks and debris encountered during trenching operations must be brought to the surface of the ground. Remove rocks and debris as ordered. This work is change order work.

If trenching requires the removal of plants, in areas with:

1. Turf, replace turf with sod under section 20-3.03C(3)(e).
2. Groundcover, replace groundcover plants from flats and plant at 12 inches on center under section 20-3.03C. No replacement of *Carpobrotus* and *Delosperma* is required if removed by rototilling.

11-15-13

Where existing surfacing is removed, replace the structural section to match the materials removed. Replacement concrete must be of uniform smoothness, color, and texture equal to the adjacent concrete surface. Dispose of removed material. Install supply line and conduits at the bottom of trenches and backfill with sand to a depth of 2 inches over the top of the supply lines and conduits. Excluding the part of the trench backfilled with surfacing or pavement, the remainder of the trench must be backfilled with material that is excavated from the trench. Rock, broken concrete, asphalt concrete and other particles larger than 2 inches in greatest dimension must not be used.

07-19-13

20-2.01C(3) Pull Boxes

Install pull boxes under section 86-2.06 at the following locations:

1. At all conductor splices except splices made in valve boxes
2. Within 5 feet of irrigation controllers
3. At ends of electrical conduits
4. At other locations shown

20-2.01C(4) Valve Boxes and Covers

Install and identify each valve box as shown.

In walkways and paved areas, install the top of the valve box flush with the surrounding finished grade.

20-2.01C(5) Recycled Water Warning Signs

Install recycled water warning signs on irrigation facilities using recycled water.

Install sign decals directly to clean, smooth surfaces. Clean the surface with alcohol or an equivalent cleaner before applying the decal.

Install a 4 by 4 inch warning sign decal to each:

1. Backflow preventer assembly
2. Irrigation controller enclosure cabinet door

Install a 2 by 2 inch warning tag to the each remote control valve and valve box cover.

Install a 2-1/2 by 3 inches sign decal to each sprinkler riser.

Under local regulations, install a 12 by 12 inch warning sign decal on an aluminum plate and attach to gates, fences, and walls located in the vicinity of a recycled water irrigation system. On gates and fences, install signs with S hooks and C clips or 14-gauge galvanized steel wire. On concrete walls or other rough surfaces, install signs with a silicon-based adhesive.

20-2.01C(6) Garden Valves

Furnish 3 keys for each garden valve before Contract acceptance.

20-2.01D Payment

Not Used

20-2.02 EXISTING IRRIGATION FACILITIES

20-2.02A General

20-2.02A(1) Summary

Section 20-2.02 includes specifications for checking, testing, operating, replacing, and relocating existing irrigation facilities.

20-2.02A(2) Definitions

Reserved

20-2.02A(3) Submittals

Submit a list of irrigation system deficiencies within 7 days after checking the existing facilities.

20-2.02A(4) Quality Control and Assurance

After irrigation facilities have been relocated, demonstrate in the presence of the Engineer that the relocated facilities function properly.

Certify each existing backflow preventer under section 20-2.03A(4).

20-2.02B Materials

Valve box covers must be the same size as the covers they replace.

Control and neutral conductors must be the same size and color as the control and neutral conductors they replace.

20-2.02C Construction

20-2.02C(1) General

Notify the Engineer at least 4 business days before shutting off the water supply to any portion of the existing irrigation system and immediately after restoring the water supply to any portion of the existing irrigation system.

If an irrigation facility to be relocated is determined unsuitable by the Engineer, replace irrigation facility under section 20-2. This work is change order work.

20-2.02C(2) Check and Test Existing Irrigation Facilities

Before performing irrigation system work, check existing irrigation facilities to remain in place or to be relocated. The Engineer determines the test watering cycle lengths. Check for deficiencies including missing parts, damaged components, and improper operation. Correct deficiencies as ordered. The correction of deficiencies is change order work.

20-2.02C(3) Operate Existing Irrigation Facilities

If the Contract includes a bid item for operate existing irrigation facilities, after performing work under section 20-2.02C(2), operate existing irrigation facilities through Contract acceptance.

Operate existing irrigation facilities except for water meters, underground supply lines, control and neutral conductors, and electrical conduits.

Check for proper operation at least once every 30 days. Adjust, repair, or replace existing irrigation facilities within 7 days of finding any deficiency.

Operate irrigation systems using the automatic irrigation controller until Contract acceptance. You may operate irrigation controllers manually during plant replacement, fertilization, weed germination, and repair work.

Program the irrigation controllers for seasonal requirements.

20-2.02C(4) Replace Valve Box Covers

Existing valve box covers shown to be replaced must remain in place until the new covers are ready to be installed.

Dispose of removed valve box covers.

20-2.02C(5) Relocate Backflow Preventer Assemblies

Relocate backflow preventer assembly as shown and install under section 20-2.03C.

20-2.02C(6) Relocate Water Meters

Relocate water meter as shown.

20-2.02C(7) Relocate Irrigation Controllers

Relocate irrigation controller as shown and install under section 20-2.07C.

20-2.02D Payment

Not Used

20-2.03 BACKFLOW PREVENTER ASSEMBLIES

20-2.03A General

20-2.03A(1) Summary

Section 20-2.03 includes specifications for installing a backflow preventer assembly.

20-2.03A(2) Definitions

Reserved

20-2.03A(3) Submittals

Reserved

20-2.03A(4) Quality Control and Assurance

Each backflow preventer assembly must be certified by a backflow preventer tester. The tester must have an active and valid certification from the water purveyor having jurisdiction.

If the local water purveyor does not have a certification program, the tester must be certified by AWWA or a nearby county with a certification program.

Notify the Engineer at least 5 business days before certifying backflow preventer assembly.

Certify each backflow preventer assembly annually and within 10 days before Contract acceptance.

20-2.03B Materials

20-2.03B(1) General

Each backflow preventer assembly must include:

1. Backflow preventer including gate valve, wye strainer, brass or malleable iron unions, fittings, and supports
2. Blanket
3. Enclosure
4. Concrete pad

Concrete for the pad must be minor concrete, except the concrete must not contain less than 463 pounds of cementitious material per cubic yard. Hand mixing of the concrete is allowed.

20-2.03B(2) Backflow Preventers

Each backflow preventer must:

1. Be reduced-pressure principle type.
2. Comply with the requirements of the water purveyor that has jurisdiction.
3. Be factory-assembled with:
   3.1. 2 check valves

3.2. 1 pressure differential relief valve
3.3. 4 test cocks
3.4. 2 shut-off valves manufactured from iron or bronze. Shut-off valves must be one of the following:
    3.4.1. Resilient wedge gate valves
    3.4.2. Resilient seated and fully ported ball valves
    3.4.3. Resilient seated butterfly valves

Backflow preventer components must be capable of withstanding a working pressure of 150 psi.

20-2.03B(3) Backflow Preventer Blankets

Each backflow preventer blanket must:

1. Be polyester fabric coated with vinyl or polymeric resin
2. Be resistant to UV light, water, mildew, and fire
3. Have an R-value from R-30 to R-38

Blankets must have a securing mechanism that includes either zippers, hook-pile tape, grommets, snaps, buttons, or any combination of these. Wherever the backflow preventer is not in an enclosure, the securing mechanism must be capable of accepting a padlock.

20-2.03B(4) Backflow Preventer Enclosures

Each backflow preventer enclosure must:

1. Have expanded metal sides, ends, and top panels fabricated from 9-gauge minimum thickness stainless sheet steel with openings of approximately 3/4 by 1-3/4 inches
2. Have expanded metal panels attached to the 3/16-inch thick steel frame by a series of welds not less than 1/4 inch in length and spaced not more than 4 inches on center, along the edges of the enclosure
3. Have Type 304 stainless steel lock guards with a minimum thickness of 12 gauge.
4. Have hexagonal nuts and lock-type washers
5. Be powder coated by the manufacturer to match color no. 20450 of FED-STD-595.
6. Have padlock clasp or latch and lock mechanism

20-2.03C Construction

Finish exposed top surfaces of concrete pad with a medium broom finish applied parallel to the long dimension of pads.

Install hold-downs for the backflow preventer assembly enclosure when concrete is still plastic.

20-2.03D Payment

Not Used

20-2.04 CAM COUPLER ASSEMBLIES

20-2.04A General

Section 20-2.04 includes specifications for installing a cam coupler assembly.

20-2.04B Materials

Each cam coupler assembly must consist of a cam coupler, dust cap, check valve, pipes, fittings, concrete thrust block, and valve box with woven wire cloth and gravel.

Cam couplers and keys must be manufactured of brass or bronze and be able to withstand a working pressure of 150 psi.

Furnish 3 loose cam coupler keys before Contract acceptance.

20-2.04C Construction

Install cam coupler assemblies in valve boxes as shown.

20-2.04D Payment

Not Used

20-2.05 CONTROL AND NEUTRAL CONDUCTORS

20-2.05A General

20-2.05A(1) Summary

Section 20-2.05 includes specifications for installing control and neutral conductors.

20-2.05A(2) Definitions

Reserved

20-2.05A(3) Submittals

Reserved

20-2.05A(4) Quality Control and Assurance

Perform field tests on control and neutral conductors. Field tests must comply with the specifications for lighting circuits in section 86-2.14B.

Where the conductors are installed by trenching and backfilling, perform field tests after a minimum of 6 inches of backfill material has been placed and compacted over the conductors.

20-2.05B Materials

Control and neutral conductors must comply with the requirements in section 86-2.08.

For connections between 24-volt irrigation controllers and valve solenoids, use control and neutral conductors. Conductors must include a control conductor for each valve and a common neutral.

Conductor insulation color, except for the stripes, must be continuous throughout. The color of the conductors must be consistent from the controller to each valve. Neutral conductors must be white. Do not use white for control conductors. Do not use conductors with green insulation except as permitted by the NEC.

Conductors must be:

1. No. 12 AWG or larger or no. 14 AWG or larger for armor-clad
2. Rated for 36 V or 600 V for armor-clad
3. Rated for direct burial
4. Underground feeder cable Type UF and TWU
5. Solid, uncoated copper for armor-clad
6. Not less than 90 percent of the AWG diameter required

No. 10 and smaller conductors must be insulated with a minimum of 56 mils of PVC or a minimum of 41 mils of polyethylene. No. 8 and larger conductors must be insulated with a minimum of 70 mils of PVC.

No. 10 and smaller armor-clad conductors must be insulated with a minimum of 41 mils of polyethylene. No. 8 and larger armor-clad conductors must be insulated with 54 to 60 mils of PVC.

Armor-clad conductors must include:

1. Stainless steel tape armor, Type 304 and helically wrapped with a 33 percent minimum overlap. The tape must be 0.5 inch wide and at least 0.005 inch thick.
2. PVC outer conductor jacket that is UV resistant and complies with the ICEA S-61-402, NEMA standard WC5 and UL listing 1263. The jacket nominal thickness must be 24 to 30 mils thick.

20-2.05C Construction

20-2.05C(1) General

Reserved

20-2.05C(2) In Open Trenches

Do not install control and neutral conductors above each other in an open trench. Wrap conductors together with electrical tape at 5 foot intervals.

Where conductors are installed in the same trench as supply line, install at the same depth as the line. At other locations, install conductors not less than 12 inches below finished grade.

Where conductors are not in a supply line trench, install conductors at least 4 feet from curbs, dikes, and paved shoulders.

20-2.05C(3) In Conduits

Install conductors in electrical conduit if conductors are to be:

1. Surface mounted
2. Installed in or on structures
3. Installed under paved areas
4. Installed in irrigation conduits
5. Placed in concrete

20-2.05C(4) Splicing

Splice low voltage control and neutral conductors under sections 86-2.09C, 86-2.09D, and 86-2.09E, except do not use method B. Tape used for splice insulation must be PVC tape.

Leave at least 2 feet of slack for each conductor at each:

1. Pull box
2. Valve box for each conductor that is connected to other facilities within the box or spliced within the box

Do not splice conductors in irrigation controller cabinets.

Permanent splice connections must be made with freshly cut and skinned conductors. Do not use temporary splices made for testing valve circuits as permanent splices.

20-2.05C(5) Marking

Mark control and neutral conductors in pull boxes, valve boxes, at irrigation control terminals, and at splices.

Mark conductor terminations and splices with adhesive cloth wrap-around markers. Seal markers with clear, heat-shrinkable sleeves.

Mark nonspliced conductors with clip-on C-shaped white extruded PVC sleeves. Sleeves must have black indented legends of uniform depth with transparent overlays over the legends and chevron cuts for the alignment of 2 or more sleeves.

Identify markers for the control conductors with the appropriate irrigation controller and station number.

20-2.05D Payment

Not Used

20-2.06 FLOW SENSORS

20-2.06A General

Section 20-2.06 includes specifications for installing a flow sensor.

20-2.06B Materials

Each flow sensor must be an inline type with a nonmagnetic spinning impeller as the only moving part.

The electronics housing must:

1. Be schedule 80 PVC or cast 85-5-5-5 bronze
2. Include glass-filled polyphenylene sulfide
3. Be easily removable from the meter body and include 2 ethylene-propylene O-rings

The impeller must be tungsten carbide.

The electronics must be rated to withstand prolonged water immersion conditions and include 2 single conductor 18 AWG leads, 48 inches long.

The insulation must be direct burial UF type colored red for the positive lead and black for the negative lead.

The flow sensor must be capable of withstanding:

1. 100 to 400 psi operating pressure depending on sensor size shown
2. Liquid temperatures up to 220 degrees F
3. Flows from 1/2 to 15 ft/sec

20-2.06C Construction

Install flow sensor as shown.

20-2.06D Payment

Not Used

20-2.07 IRRIGATION CONTROLLERS

20-2.07A General

20-2.07A(1) Summary

Section 20-2.07 includes specifications for installing irrigation controllers.

20-2.07A(2) Definitions

irrigation controller: "Smart" irrigation controller as defined by the Irrigation Association.

remote irrigation control system (RICS): Centralized water management system that consists of a base station, centralized server, satellite controllers.

base station: Designated computer located at a Department maintenance facility or District Office that collects data from a series of satellite controllers through a centralized server.

centralized server: Designated server or web-based application that collects data from all base stations.

web-based application: Encrypted managing software that is coded in a browser-supported language and is executable via a common internet web browser (e.g., Microsoft Internet Explorer, Firefox, Safari, etc.).

satellite controller: Irrigation controller that communicates directly to a base station or centralized server.

network communication: Identified means through which satellite controllers, base stations, and a centralized server communicate to one another (i.e., fiber optics, spread spectrum, phone line, etc.).

remote access device: Device (i.e., FCC compliant radio remote, cell phone or wireless, etc.) used to communicate with satellite controllers from a remote location.

20-2.07A(3) Submittals

Submit as an informational submittal, a complete manufacturer's maintenance and operations manual for each type of controller installed. Submit the manual at the time the wiring plans and diagrams are placed inside the controller enclosure or cabinet door.

20-2.07A(4) Quality Control and Assurance

Provide training by a qualified person on the use and adjustment of the irrigation controllers installed 30 days before Contract acceptance.

Modifications to electrical components must be done by the manufacturer before shipment to the job site.

The installation date and expiration date of the manufacturer's guarantee for the controllers must be permanently marked on the inside face of the controller.

20-2.07B Materials

20-2.07B(1) General

Conventional A/C powered irrigation controllers must operate on 110/120 V, 60 Hz(ac) and supply 24 to 30 VAC, 60 Hz(ac) for operating electrical remote control valves.

Concrete for the pad and foundation must be minor concrete, except the concrete must not contain less than 463 pounds of cementitious material per cubic yard. Hand mixing of the concrete is allowed.

20-2.07B(2) Irrigation Controllers

20-2.07B(2)(a) General

The irrigation controllers must:

1. Be A/C, battery, solar, or 2-wire as shown
2. Be from a single manufacturer.
3. Be fully automatic and capable of operating a complete 30-day or longer irrigation program.
4. Have a switch or button on the face of the irrigation control panel showing that the irrigation controller can be turned on or off and provide for automatic or manual operation. Manual operation must allow cycle start at the desired station and allow for the minimum activation of a single station or have the option to operate multiple stations in sequential or simultaneous operation modes.
5. Have non-volatile memory.
6. Have a watering time display on the face of the control panel.
7. Have a panel and circuit board connected to the low voltage control and neutral conductors by means of a plug and receptacle connectors located within the cabinet enclosure.
8. Have a variable or incremental timing adjustment ranging from 1 minute to 360 minutes per station.
9. Be capable of operating at least 3 program schedules.
10. Be capable of having at least 4 start times per program schedule.
11. Have an output that can energize a pump start circuit or a remote control master valve.
12. Be protected by fuses and circuit breakers.
13. Display a program and station affected by a sensory alert without altering other watering schedules not affected by the alert.
14. Be capable of global manual and automatic seasonal adjustments to all valves in any given program.
15. Automatically alter watering schedule in accordance with evapotranspiration data provided by a local weather station or have an internal programmed default of historical evapotranspirational data for a given region.
16. Support a flow sensor, rain sensor, or weather station and have automatic shut-off capability.
17. Be capable of communicating with the remote access device.

If the irrigation controller is installed in an enclosure cabinet, the cabinet must be stainless steel and must comply with section 86-3.04A.

Irrigation controllers not installed in enclosure cabinets must be weatherproof, constructed of fiberglass or metal and have a door lock with 2 keys provided.

RICS must meet the requirements of an irrigation controller and be capable of being accessible only through a secured and encrypted server that is password and firewall protected by the Department or be accessible through a firewall secure remote server that is independent from any Department servers. The Department will set up and manage the network communication.

20-2.07B(2)(b) Battery Powered Irrigation Controllers

Reserved

20-2.07B(2)(c) Solar Powered Irrigation Controllers

Reserved

20-2.07B(2)(d) Two-wire Irrigation Controllers

Reserved

20-2.07B(3) Irrigation Controller Enclosure Cabinets

The irrigation controller enclosure cabinet must:

1. Be stainless steel.
2. Include a mounting panel. Fabricate mounting panels with one of the following:
   - 2.1. 3/4-inch exterior AC grade veneer plywood. Paint panels with 1 application of an exterior, latex based, wood primer and 2 applications of an exterior, vinyl acrylic enamel, white in color. Paint panels on all sides and edges before installation of the panels in the cabinets and the equipment on the panels.
   - 2.2. 3/16-inch thick aluminum sheets.
   - 2.3. 10-gauge cold-rolled steel sheets.
   - 2.4. 0.157-inch stainless steel metal sheets.
3. Provide cross ventilation, roof ventilation, or a combination of both. Ventilation must not compromise the weather resistance properties of the cabinet and must be fabricated by the cabinet manufacturer.
4. Include protection against lightning damage.
5. Have an area inside the cabinet doors for storage of the as-built schematic wiring diagram and irrigation plans.
6. Have padlock clasp or latch and lock mechanism.

20-2.07B(4) Rain Sensors

A rain sensor unit must be a solid state, automatic shut-off type, and compatible with the irrigation controller. The rain sensor unit must automatically interrupt the master remote control valves when approximately 1/8 inch of rain has fallen. The irrigation controller must automatically be enabled again when the accumulated rainfall evaporates from the rain sensor unit collection cup.

Rain sensor units must be one of the following:

1. Rated 24 V(ac) to 30 V(ac)
2. Wireless and FCC compliant

20-2.07C Construction

Finish exposed top surface of concrete pad with a medium broom finish applied parallel to the long dimension.

Locate irrigation controllers in pedestal or wall mounted enclosures as shown.

Install electrical components for automatic irrigation systems under section 86-1.02.

Install irrigation controllers under the manufacturer's instructions and as shown.

If 2 or more irrigation controllers operate the same remote master control valve, furnish and install an isolation relay under the controller manufacturer's instructions.

Where direct burial conductors are to be connected to the terminal strip, connect the conductors with the open-end-crimp-on wire terminals. Exposed wire must not extend beyond the crimp of the terminal and the wires must be parallel on the terminal strip.

Install rain sensor units for irrigation controllers on the irrigation controller enclosure cabinets. Provide protection against lightning damage.

20-2.07D Payment

Payment for electrical service for 120-volt or higher is not included in the payment for irrigation controller.

20-2.08 IRRIGATION CONDUIT

20-2.08A General

20-2.08A(1) Summary

Section 20-2.08 includes specifications for installing irrigation conduit under a roadway or other facility to accommodate electrical conduit for control and neutral conductors and irrigation supply lines.

Before performing work on irrigation systems, locate existing conduits shown to be incorporated into the new work.

Before removing or disturbing existing Type A pavement markers that show the location of the existing conduit, mark the location of the existing conduit on the pavement.

20-2.08A(2) Definitions

Reserved

20-2.08A(3) Submittals

Reserved

20-2.08A(4) Quality Control and Assurance

Demonstrate the conduits are free of obstructions after placement of base and surfacing.

Before and after extending the irrigation supply line in a conduit, pressure test the supply line under section 20-2.01A(4)(b).

After conductors are installed in a conduit, test the conductors under section 20-2.05A(4).

Assign a technical representative to direct and control the directional bore activities. The representative must be present during directional bore activities. Unless otherwise authorized, perform directional bore activities in the presence of the Engineer.

20-2.08B Materials

20-2.08B(1) General

Reserved

20-2.08B(2) ABS Composite Pipe Conduit

ABS composite pipe and couplings must comply with ASTM D 2680. Couplings must be solvent cement type.

20-2.08B(3) Corrugated High Density Polyethylene Pipe Conduit

Corrugated high density polyethylene pipe must comply with ASTM F 405 and F 667 or be Type S and comply with AASHTO M252 and M294. Couplings and fittings must be as recommended by the pipe manufacturer.

20-2.08B(4) Corrugated Steel Pipe Conduit

Corrugated steel pipe conduit must comply with section 66. The nominal thickness of metal sheets for pipe must be 0.064 inch for corrugated steel pipe and 0.060 inch for corrugated aluminum pipe. Coupling bands and hardware must comply with section 66.

20-2.08B(5) Polyvinyl Chloride Pipe Conduit

PVC pipe conduit must be schedule 40 and comply with ASTM D 1785.

Fittings must be schedule 80.

20-2.08B(6) Welded Steel Pipe Conduit

Welded steel pipe must comply with ASTM A 53. Pipe must be black and have either welded or threaded joints.

The minimum wall thickness for the various sizes of welded steel pipe must comply with the dimensions shown in the following table:

Pipe size, nominal (inch)	Minimum wall thickness (inch)
3	0.216
4	0.237
6	0.280
8	0.277
10	0.279
12	0.330

20-2.08C Construction

20-2.08C(1) General

When existing conduits are to be incorporated in new work, excavate exploratory holes for locating existing conduits at the locations indicated by existing markers or as directed. Excavate and backfill exploratory holes to a maximum size of 2-1/2 feet in width, 5 feet in depth, and 5 feet on each side of the marker or directed location parallel to the roadway. If the conduit is not found and if ordered, increase the size of the exploratory holes beyond the dimensions specified. The additional excavation and backfill is change order work.

If extending an existing conduit, remove conductors from the conduit.

Use a coupling band if the new conduit matches the existing conduit diameter, otherwise overlap the conduit at least 12 inches.

After extending existing conduits, install conductors that match the color and size of the existing conductors without splices. Splice conductors in adjacent pull boxes.

If installing a control and neutral conductor and electrical conduit through the irrigation conduit, install a no. 5 pull box at each end.

Remove debris found in the conduit before performing other work. Debris found more than 3 feet from the ends of the conduits is removed as change order work.

Extend conduit 2 feet beyond all paving unless otherwise shown.

Cap the ends of unused conduit.

Designate the location of each conduit by cementing a Type A pavement marker as shown. Type A pavement markers and adhesive must comply with section 85.

20-2.08C(2) Welded Steel Pipe Conduit

20-2.08C(2)(a) General

Install welded steel pipe by directional boring or jack and drill.

Install top of conduits:

1. 18 to 30 inches below the finished surface in sidewalk areas
2. 40 to 52 inches below the finished grade in other paved areas

20-2.08C(2)(b) Directional Boring

Notify the Engineer 2 business days before starting directional bore activities.

The diameter of the boring tool for directional boring must be only as large as necessary to install the conduit.

Mineral slurry or wetting solution may be used to lubricate the boring tool and to stabilize the soil surrounding the boring path. The mineral slurry or wetting solution must be water based.

The directional bore equipment must have directional control of the boring tool and have an electronic boring tool location detection system. During operation, the directional bore equipment must be able to determine the location of the tool both horizontally and vertically.

20-2.08C(2)(c) Jack and Drill

Notify the Engineer 2 business days before starting jack and drill activities.

Jacking or drilling pits must be no closer than 2 feet from pavement edge whenever possible.

If authorized, small holes may be cut in the pavement to locate or remove obstructions.

Do not use excessive water that will soften subgrade or undermine pavement.

20-2.08C(3) Schedule 40 Pipe Conduit

Where schedule 40 pipe conduit 2 inches or less in outside diameter is installed under surfacing, you may install by directional boring under section 20-2.08C(2)(b).

For conduit 2 inches or less in diameter, the top of the conduit must be a minimum of 18 inches below surfacing.

Extend schedule 40 pipe conduit 6 inches beyond surfacing. Cap ends of conduit until used.

20-2.08D Payment

Schedule 40 PVC pipe conduit is paid for as plastic pipe (schedule 40) (supply line).

20-2.09 IRRIGATION SUPPLY LINE

20-2.09A General

20-2.09A(1) Summary

Section 20-2.09 includes specifications for installing irrigation supply line.

If the supply line location interferes with the excavation of plant holes, relocate the plant hole to clear the supply line. Do not install supply lines through plant holes unless shown.

Supply lines, control and neutral conductors and electrical conduits installed in common trenches must not be installed above each other.

20-2.09A(2) Definitions

Reserved

20-2.09A(3) Submittals

Submit a certificate of compliance for polyethylene pipe and plastic pipe supply line.

20-2.09A(4) Quality Control and Assurance

Solvent cement must comply with the local Air Quality Management District requirements.

20-2.09B Materials

20-2.09B(1) General

Irrigation supply pipe must be metal or plastic as shown.

PCC for thrust blocks must be produced from commercial-quality aggregates. The concrete must contain at least 295 pounds of cementitious material per cubic yard.

20-2.09B(2) Copper Pipe Supply Line

Copper pipe must be Type K rigid pipe and comply with ASTM B 88. Fittings must be wrought copper or cast bronze either soldered or threaded.

Solder must be 95 percent tin and 5 percent antimony.

20-2.09B(3) Galvanized Steel Pipe Supply Line

Galvanized steel pipe supply line and couplings must be standard weight and comply with ASTM A 53, except that the zinc coating must not be less than 90 percent of the specified amount. Except for couplings, fittings must be galvanized malleable iron, banded and threaded, and comply with ANSI B16.3, Class 150.

Joint compound must be nonhardening and noncorrosive. Do not use pipe thread sealant tape.

20-2.09B(4) Drip Irrigation Tubing

Drip irrigation tubing must be virgin polyethylene plastic and comply with ASTM D 2737.

The drip irrigation tubing must be distribution tubing with preinstalled in-line emitters.

If preinstalled in-line drip irrigation tubing is not shown, you may install emitters that match the distribution requirements shown. The emitters must be barbed or threaded-type outlet devices with dual silicone diaphragms and installed under the manufacturer's instructions.

The emitters must meet the flow rate and operating pressure range shown.

The wall thickness of polyethylene tubing must comply with the following requirements when tested under ASTM D 2122:

Pipe size, nominal (inch)	Minimum wall thickness (inch)	Maximum wall thickness (inch)
1/2	0.050	0.070
5/8	0.055	0.075
3/4	0.060	0.080

The polyethylene tubing fittings must be leak-free, compression type and have female sockets with an internal barb to provide a positive pipe-to-fitting connection that will not separate at the designed pressure.

20-2.09B(5) Plastic Pipe Supply Line

Plastic pipe supply line must be PVC pipe that is NSF approved.

Schedule 40 plastic pipe supply line must comply with ASTM D 1785.

Class 315 plastic pipe supply line must comply with ASTM D 2241.

PVC gasketed bell joints must comply with ASTM D 2672, ASTM D2241, ASTM D 3139, and ASTM F 477.

For solvent-cemented type joints, the primer and solvent cement must be made by the same manufacturer. The primer color must contrast with the color of the pipe and fittings.

Solvent-cemented fittings must be injection molded PVC, schedule 40, and comply with ASTM D 2466.

Fittings for supply line placed in irrigation conduit must be schedule 80.

Fittings for plastic pipe supply line larger than 4 inches must be ductile iron under section 20-2.14C(2)(b).

If UV-resistant plastic pipe supply line is required, the pipe must be homogeneous, uniform color and be manufactured of:

1. At least 80 percent vinyl chloride resin with UV stabilizers
2. Non-PVC resin modifiers and coloring ingredients
3. Coloring ingredients with UV stabilizers

20-2.09C Construction

20-2.09C(1) General

Cut pipe straight and true. After cutting, ream out the ends to the full inside diameter of the pipe.

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Prevent foreign material from entering the irrigation system during installation. Immediately before assembling, clean all pipes, valves, and fittings. Flush lines before attaching sprinklers, emitters, and other terminal fittings. Reuse water from waterline flushing for landscape irrigation if practicable.

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Pipe supply lines installed between the water meter and backflow preventer assembly must be installed not less than 18 inches below finished grade measured to the top of the pipe.

Where a connection is made to existing supply lines, bell and gasketed fittings or compression fittings may be used.

Install a thrust block at each change in direction on the main supply line, terminus run, and at other locations shown.

Where supply lines cross paved ditches more than 3 feet deep at their flow line, install galvanized steel pipe for the entire span of the ditch.

Secure UV resistant plastic pipe supply line on grade as shown.

20-2.09C(2) Galvanized Steel Pipe Supply Line

Coat male pipe threads on galvanized steel pipe according to the manufacturer's instructions.

20-2.09C(3) Drip Irrigation Tubing

Install drip irrigation tubing on grade and under manufacturer's instructions.

Install a flush valve and an air-relief valve if recommended by the drip valve assembly manufacturer.

20-2.09C(4) Plastic Pipe Supply Line

For PVC pipe 1-1/2 inches in diameter or smaller, cut the pipe with PVC cutters.

For solvent-cemented type joints, apply primer and solvent-cement separately under the manufacturer's instructions.

Wrap the male portion of each threaded plastic pipe fitting with at least 2 layers of pipe thread sealant tape.

Install plastic pipe supply line mains with solvent-cemented type joints not less than 18 inches below finished grade measured to the top of the pipe.

Install plastic pipe supply line laterals with solvent-cemented type joints not less than 12 inches below finished grade measured to the top of the pipe.

Snake plastic pipe installed by trenching and backfilling methods.

20-2.09D Payment

Supply line pipe and drip irrigation tubing are measured along the slope.

20-2.10 SPRINKLER ASSEMBLIES

20-2.10A General

Section 20-2.10 includes specifications for installing sprinkler assemblies.

20-2.10B Materials

20-2.10B(1) General

Each sprinkler assembly must meet the characteristics shown in the irrigation legend.

Where shown, a sprinkler assembly must have a flow shut-off device that automatically stops the flow of water on the downstream side of the device when the assembly is broken. You may use a sprinkler assembly with a preinstalled flow shut-off device or you must install a flow shut-off device under the manufacturer's instructions.

Flexible hose for sprinkler assembly must be leak-free, nonrigid and comply with ASTM D 2287, cell Type 6564500. The hose wall thickness must comply with ASTM D 2122 for the hose diameters shown in the following table:

Hose diameter, nominal (inch)	Minimum wall thickness (inch)
1/2	0.127
3/4	0.154
1	0.179

Solvent cement and fittings for flexible hose must comply with section 20-2.09B(5).

20-2.10B(2) Pop-Up Sprinkler Assemblies

Each pop-up sprinkler assembly must include a body, nozzle, swing joint, pressure compensation device, check valve, sprinkler protector, and fittings as shown.

20-2.10B(3) Riser Sprinkler Assemblies

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Each riser sprinkler assembly must include a riser or flexible hose, threaded nipple, swing joint, check valve, and nozzle as shown. The riser must be UV resistant schedule 80, PVC 1120 or PVC 1220 pipe and comply with ASTM D 1785.

20-2.10B(4) Tree Well Sprinkler Assemblies

Each tree well sprinkler assembly must include a body, riser, swing joint, perforated drainpipe, and drain cap.

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The perforated drainpipe must be commercial grade, rigid, PVC pipe with holes spaced not more than 6 inches on center on 1 side of the pipe.

Drain cap must be commercially available, 1 piece, injection molded drain grate manufactured from structural foam polyolefins with UV light inhibitors. Drain grate must be black.

Gravel for filling the drainpipe must be graded such that 100 percent passes the 3/4-inch sieve and 100 percent is retained on the 1/2-inch sieve. Gravel must be clean, washed, dry, and free from clay or organic material.

20-2.10C Construction

Install pop-up and riser sprinkler assembly:

1. 6-1/2 to 8 feet from curbs, dikes, and sidewalks
2. 10 feet from paved shoulders
3. 3 feet from fences and walls

If sprinkler assembly cannot be installed within these limits, the location will be determined by the Engineer.

Set sprinkler assembly riser on slopes perpendicular to the plane of the slope.

Install tree well sprinkler assembly as shown.

20-2.10D Payment

Not Used

20-2.11 VALVES

20-2.11A General

Section 20-2.11 includes specifications for installing valves.

20-2.11B Materials

20-2.11B(1) General

Valves must:

1. Include a valve box and cover
2. Be the same size as the supply line that the valve serves unless otherwise shown
3. Be bottom, angled, or straight inlet configuration

20-2.11B(2) Ball Valves

Ball valve must be a two-piece brass or bronze body and comply with the requirements shown in the following table:

Property	Requirements
Nonshock working pressure, min	400 psi
Seats	PTFE
O-ring seals	PTFE

Ball valve must be the same size as the supply line that the valve serves.

20-2.11B(3) Check Valves

Each check valve must:

1. Be schedule 80 PVC and factory set to 5 psi for adjustable spring check valve
2. Be Class 200 PVC for swing check valves on non pressurized plastic irrigation supply line

20-2.11B(4) Drip Valve Assemblies

Each drip valve assembly must include:

1. Remote control valve
2. Wye filter with:
   - 2.1. Filter housing that:
     - 2.1.1. Can withstand a working pressure of 150 psi
     - 2.1.2. Is manufactured of reinforced polypropylene plastic
   - 2.2. Reusable stainless steel filter cartridge with a 200 mesh size filtration
3. Ball valve under 20-2.11B(2)
4. Schedule 80 PVC pipes and fittings
5. Pressure regulator

20-2.11B(5) Garden Valve Assemblies

Each garden valve assembly must have:

1. Garden valve
2. Location marker

20-2.11B(6) Gate Valves

Gate valves must be:

1. Flanged or threaded type
2. Iron or bronze body
3. Bronze trimmed with one of the following:
   - 3.1. Internally threading rising stem
   - 3.2. Nonrising stem
4. Able to withstand a working pressure of 150 psi
5. Same size as the pipeline that the valves serves unless otherwise shown

Gate valves smaller than 3 inches must have a cross handle.

Gate valves 3 inches or larger must be flanged type with a square nut. Furnish 3 long shank keys before Contract acceptance.

Gate valves attached to the outlets of a wye strainer must have seating rings on the discharge side of the gate valves must be PTFE. Valve wedges must be driven obliquely by cam action into the seating rings.

20-2.11B(7) Pressure Regulating Valves

Pressure regulating valve must be:

1. Flanged or threaded type
2. Brass, bronze, cast iron, or plastic body
3. Spring diaphragm type
4. Pilot controlled

Pressure regulating valve must have no internal filter screens.

20-2.11B(8) Pressure Relief Valves

Pressure relief valve must have a brass or bronze body, stainless steel springs, bronze nickel chrome seats, composition seat discs, female bottom inlets, and female side outlets.

20-2.11B(9) Quick Coupling Valves

Quick coupling valve must be 3/4 inch double slotted with a self-closing cap, 3/4-inch brass key and 3/4-inch brass hose swivel unless otherwise shown. Except for the cap, quick coupling valve must be brass or bronze construction. Furnish 3 loose quick coupling brass keys and brass hose swivels before Contract acceptance.

20-2.11B(10) Remote Control Valves

20-2.11B(10)(a) General

Each remote control valve must:

1. Be normally closed type.
2. Be glass filled nylon, brass, or bronze.
3. Be completely serviceable from the top without removing the valve body from the system.
4. Be equipped with a device that regulates and adjusts the flow of water and be provided with a manual shut-off. The manual shut-off for valves larger than 3/4 inch must be operated by a cross handle.
5. Have solenoids compatible with the irrigation controller.
6. Have a manual bleed device.
7. Be capable of withstanding a pressure of 200 psi
8. Have replaceable compression discs or diaphragms.
9. Have threaded fittings for inlets and outlets.
10. Have DC latching solenoids when used with solar or battery controllers. Solenoids must operate on 3.5 V.

20-2.11B(10)(b) Remote Control Valves with Flow Sensor

Reserved

20-2.11B(10)(c) Remote Control Valves with Pressure Regulator

Each remote control valve with pressure regulator must be factory assembled as 1 unit.

20-2.11B(11) Wye Strainer Assemblies

Each wye strainer assembly must include:

1. Wye strainer
2. Garden valve

20-2.11C Construction

20-2.11C(1) General

Install control valves:

1. 6-1/2 to 8 feet from curbs, dikes, and sidewalks
2. 10 feet from paved shoulders
3. 3 feet from fences, walls, or both

If a control valve cannot be installed within these limits, the location will be determined by the Engineer.

20-2.11C(2) Check Valves

Unless otherwise shown, install spring-action check valves as necessary to prevent low head drainage.

20-2.11C(3) Garden Valve Assemblies

Install a location marker 8 to 10 inches from the back of each garden valve.

20-2.11C(4) Pressure Regulating Valves

Install pressure regulating valves with threaded connections and a union on the inlet side of the valves.

20-2.11C(5) Wye Strainer Assemblies

Unless shown, install wye strainer assembly on the upstream side of the remote control valves.

Install garden valve so that when the system is flushed, the discharge sprays out of the valve box.

20-2.11D Payment

Not Used

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20-2.12–20-2.13 RESERVED

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20-2.14 SUPPLY LINE ON STRUCTURES

20-2.14A General

20-2.14A(1) General

20-2.14A(1)(a) Summary

Section 20-14 includes specifications for installing water supply lines through bridges and on the exterior of concrete structures.

20-2.14A(1)(b) Definitions

Reserved

20-2.14A(1)(c) Submittals

Submit a work plan for temporary casing support at the abutments as an informational submittal.

20-2.14A(1)(d) Quality Control and Assurance

20-2.14A(1)(d)(i) General

Before installing seismic expansion assemblies or expansion assemblies, the Engineer must authorize the extension setting.

20-2.14A(1)(d)(ii) Regulatory Requirements

Piping materials must bear the label, stamp, or other markings of the specified standards.

20-2.14A(1)(d)(iii) Site Tests

Test water supply lines before:

1. Backfilling
2. Beginning work on box girder cell decks
3. Otherwise covering the water supply lines

Furnish pipe anchorages to resist thrust forces occurring during testing.

Test the water supply lines as 1 unit. The limits of the unit must be 5 feet beyond the casing at each end of the bridge.

Cap each end of the water supply lines before testing. Caps must be rated for the test pressure.

Test water supply lines under section 20-2.01A(4)(b), except that the testing period must be 4 hours with no pressure drop.

For water supply lines 4 inches and larger testing must meet the following additional requirements:

1. Testing pressure must be at least 120 psi
2. Air relief valve must not be subjected to water pressure due to testing

If water supply lines fail testing, retest the lines after repair.

20-2.14A(2) Materials

20-2.14A(2)(a) General

Protect stored piping from moisture and dirt. Elevate piping above grade. Support piping to prevent sagging and bending.

Protect flanges, fittings, and assemblies from moisture and dirt.

20-2.14A(2)(b) Air Release Valve Assemblies

Air release valve assemblies include an air release valve, ball valve, tank vent, nipples, and pipe saddle. Assemblies must comply with the following:

1. Air release valves must have a cast iron body with stainless steel trim and float, 1-inch NPT inlet, 1/2-inch NPT outlet, and 3/16-inch orifice.
2. Ball valves must have a 2-piece bronze body with chrome plated or brass ball, 1-inch full-size port, and be rated for at least 400 psi.
3. Tank vents must have a 1/2-inch NPT inlet and downward-facing double openings with screened covers.
4. Nipples must be schedule 40 galvanized steel pipe.
5. Pipe saddle must be rated for at least 150 psi and compatible with water supply line. Pipe saddle must be (1) single strap pipe saddle for water supply lines smaller than 4 inches or (2) double strap pipe saddle for water supply lines 4 inches and larger. You may use a tee fitting for galvanized steel water supply lines.

20-2.14A(2)(c) Casings

Casings must be welded steel pipe casing complying with section 70-7.

20-2.14A(2)(d) Pipe Wrap Tape

Pipe wrap tape must be pressure sensitive tape made from PVC or polyethylene. Pipe wrap tape must be at least 50 mils thick and not wider than 2 inches.

20-2.14A(2)(e) Pipe Hangers

Pipe hangers must comply with section 70-7.02C.

The pipe hanger must be rated for the water supply line. If casings are shown, include the casings weight.

20-2.14A(2)(f) Epoxy Adhesives

Epoxy used for anchoring concrete pipe supports must comply with section 70-7.02D.

20-2.14A(2)(g) Concrete Pipe Supports

Concrete pipe supports must comply with section 70-7.02D.

20-2.14A(2)(h) Pipe Clamps and Anchors

Metal clamps must be commercial quality steel complying with section 75-1.02. Anchors must comply with the specifications for concrete anchorage devices in section 75-1.03C.

20-2.14A(2)(i) Pull Boxes

Pull boxes and covers must comply with section 20-2.01B(5).

20.2.14A(3) Construction

20-2.14A(3)(a) General

Support water supply lines as described.

Where water supply lines penetrate bridge superstructure concrete, either form or install pipe sleeves at least 2 pipe sizes larger than the pipe.

20-2.14A(3)(b) Preparation

Clean the interior of the pipe before installation. Cap or plug openings as pipe is installed to prevent the entrance of foreign material. Leave caps or plugs in place until the next pipe section is installed.

20-2.14A(3)(c) Installation

20-2.14A(3)(c)(i) General

Reserved

20-2.14A(3)(c)(ii) Casings

Install casings under section 70-7.03.

Seal casing end with 8 inches of polyurethane foam at dirt stop or pipe end seal.

20-2.14A(3)(c)(iii) Wrapping Water Supply Line

Wrap damaged supply line coatings with pipe wrap tape. Wrap field joints and fittings that are in contact with the earth.

Wrapping must comply with the following:

1. Clean and prime area as recommended by the tape manufacturer.
2. Tightly wrap tape with 1/2 uniform overlap, free from wrinkles and voids, to provide not less than a 100 mil thickness.
3. The tape must conform to joint or fitting contours.
4. Extend tape at least 6 inches over adjacent pipe.

20-2.14A(3)(c)(iv) Pipe Clamps and Anchors

Install water supply lines on the exterior surfaces of bridges or other concrete structures with metal clamps and anchors.

Drilling of holes for anchors must comply with the following:

1. Drill holes to manufacturers recommended depth.
2. Drilling tools must be authorized.
3. Do not drill holes closer than 6 inches to the edge of a concrete structure.
4. Relocate holes if reinforcing steel is encountered. Fill abandoned holes with mortar. Mortar must comply with section 51-1.02F.

Where water supply lines are mounted vertically for more than 2 feet, install clamps and anchors within 6 inches of the elbows.

Where water supply lines are mounted vertically for more than 10 feet, install additional clamps and anchors at 10 foot centers unless otherwise shown.

20-2.14A(3)(d) Sequences of Operation

If the bridge superstructure is to be prestressed do not place mortar around casings in abutments and hinges until bridge superstructure prestressing has been completed.

20-2.14A(4) Payment

Supply line on structures is measured from end to end, along the centerline.

The Department does not pay for failed tests.

20-2.14B Supply Line on Structures, Less than 4 Inches

20-2.14B(1) General

20-2.14B(1)(a) Summary

Section 20-2.14B includes specifications for installing water supply lines smaller than 4 inches.

20-2.14B(1)(b) Definitions

Reserved

20-2.14B(1)(c) Submittals

Product data for materials includes catalog cuts, performance data, and installation instructions.

Submit product data for:

1. Water supply line
2. Expansion assemblies
3. Casing insulators
4. Pipe end seals
5. Pipe anchorages
6. Air release valve assemblies
7. Casings
8. Pipe hangers
9. Epoxy adhesives

10. Concrete pipe supports

20-2.14B(1)(d) Quality Control and Assurance

Reserved

20-2.14B(2) Materials

20-2.14B(2)(a) General

Reserved

20-2.14B(2)(b) Water Supply Line

Water supply lines must comply with section 20-2.09.

20-2.14B(2)(c) Expansion Assemblies

Expansion assemblies must consist of a hose with ends, insulated flange connections, and elbows. Expansion assemblies must have the same nominal inside diameter as the water supply line. Working pressure must be at least 150 psi.

Hose must be medium or heavy weight, crush and kink resistant, rated for at least 150 psi. Cover must be flexible, oil resistant rubber or synthetic, reinforced with at least 2-ply synthetic yarn or steel wire. The inner tube must meet FDA and USDA Standards for potable water. Hose ends must be stainless steel flanged connections with stainless steel crimped bands or swaged end connectors. Do not use barbed ends with band clamps.

Elbows must be 45 degree, standard weight galvanized steel fittings.

20-2.14B(2)(d) Casing Insulators

Casing insulators must be:

1. 2-piece, high-density, injection-molded polyethylene, nonconductive inner liner, with cadmium-plated nuts and bolts.
2. Factory constructed to ensure the water supply line is centered in the casing. Insulators must not allow any contact between pipe and casing and have at least 2 runners seated on the bottom of the casing.
3. Sized for the casing and water supply line shown.

20-2.14B(2)(e) Pipe Anchorages

Pipe anchorages must consist of an I-beam, U-bolts, anchors, and double nuts.

Use concrete anchorage devices for anchors on existing bridges. Use L-anchor bolts for anchors on new bridges.

Fabricate the I-beam from 1/2-inch steel plate. Steel plate, U-bolts, L-anchors, and nuts must comply with section 75-1.02. Concrete anchorage devices must comply with section 75-1.03C.

20-2.14B(2)(f) Pipe End Seals

Pipe end seals must consist of a pipe end seal, stainless steel bands, and polyurethane foam.

Pipe end seal must be factory constructed from seamless neoprene and sized for the casing and water supply line shown. Neoprene must be at least 1/8 inch thick. Stainless steel bands must be crimped.

Polyurethane foam must be expanding foam spray that is water resistant and moisture cured.

20-2.14B(3) Construction

Locate pipe anchorage halfway between expansion assemblies.

Pipe end seal must be pulled onto the casing during pipe installation. Do not use wrap-around type end seals.

20-2.14B(4) Payment

Supply line on structures is paid for as galvanized steel pipe (supply line on bridge).

20-2.14C Supply Line on Structures, 4 Inches and Larger

20-2.14C(1) General

20-2.14C(1)(a) Summary

Section 20-2.14C includes specifications for installing water supply lines 4 inches and larger.

20-2.14C(1)(b) Definitions

Reserved

20-2.14C(1)(c) Submittals

Product data for materials includes catalog cuts, performance data, and installation instructions.

Submit product data for:

1. Water supply line
2. Expansion assemblies
3. Flange insulating gaskets
4. Casing insulators
5. Seismic expansion assemblies
6. Lateral restraint assemblies
7. Air release valve assemblies
8. Casings
9. Pipe hangers
10. Epoxy adhesives
11. Concrete pipe supports

Submit the maximum range and preset dimension for each expansion assembly or seismic expansion assembly as an informational submittal.

Submit at least 5 sets of product data to OSD, Documents Unit. Each set must be bound together and include an index stating equipment names, manufacturers, and model numbers. Two sets will be returned. Notify the Engineer of the submittal. Include in the notification the date and contents of the submittal.

20-2.14C(1)(d) Quality Control and Assurance

Reserved

20-2.14C(2) Materials

20-2.14C(2)(a) General

Reserved

20-2.14C(2)(b) Water Supply Line

Water supply lines must consist of ductile iron pipe and fittings. Pipe must comply with ANSI/AWWA C151/A21.51, Class 350. Fittings must comply with ANSI/AWWA C110/A21.10, rated for a working pressure of 350 psi.

Ductile iron pipe connections to expansion assemblies must be a flanged joint complying with ANSI/AWWA C115/A21.15. Flange gaskets must be rated for a working pressure of 350 psi. Fasteners must comply with section 75-1.02, except that stainless steel fasteners must not be used.

All other ductile iron pipe and fitting joints must be push-on, restrained type complying with ANSI/AWWA C111/A21.11. Push-on, restrained type joints may use proprietary dimensions and proprietary restrained joint locking systems.

Ductile iron pipe and fittings must have an asphaltic coating complying with ANSI/AWWA C151/A21.51, and a cement mortar lining complying with ANSI/AWWA C104/A21.4.

20-2.14C(2)(c) Expansion Assemblies

Expansion assemblies must be a sleeve type expansion joint. The expansion assembly must have:

1. Ductile iron body complying with ANSI/AWWA C153/A21.53

2. Flanged ends complying with ANSI/AWWA C110/A21.10
3. Fusion bonded epoxy internal lining complying with ANSI/AWWA C213 at least 15 mils thick
4. Internal expansion sleeve limiting stop collars and be pressure balanced
5. Working pressure of at least 350 psi for sizes 24 inches and smaller and 250 psi for sizes larger than 24 inches
6. NSF 61 certification

The expansion assembly must be factory set at 1/2 the extension capacity.

20-2.14C(2)(d) Flange Insulating Gaskets

Flange insulating gaskets must consist of a dielectric flange gasket, insulating washers and sleeves, and commercial quality steel bolts and nuts. Dielectric flange gasket must have a dielectric strength of at least 500 vpm.

20-2.14C(2)(e) Casing Insulators

Casing insulators must be:

1. 2-piece, 8-inch, 14-gauge epoxy-coated or galvanized steel band, four 2-inch-wide glass-reinforced polyester or polyethylene runners, with cadmium-plated nuts and bolts.
2. Coated with at least 15-mils heat-fused PVC to provide a nonconductive inner liner.
3. Factory constructed to ensure the water supply line is centered in the casing. Insulators must not allow any pipe to casing contact and have at least 2 runners seated on the bottom of the casing.
4. Sized for the casing and water supply line shown.

20-2.14C(2)(f) Dirt Stops

Dirt stops must consist of a redwood cover with polyurethane foam.

Use construction heart grade redwood complying with 57-2.01B(2). Construct cover to fit snugly around the water supply line. The cover must be 2 inches taller and 2 inches wider than the casing.

Polyurethane foam must be expanding foam spray that is water resistant and moisture cured.

20-2.14C(2)(g) Seismic Expansion Assemblies

Seismic expansion assemblies must be a sleeve type expansion joint with integral ball joints at each end.

Seismic expansion assemblies must have:

1. Ability to withstand at least 15 degree angular deflection at each end and maximum movement in all 3 planes at the same time
2. Ductile iron body complying with ANSI/AWWA C153/A21.53
3. Flanged ends complying with ANSI/AWWA C110/A21.10
4. Fusion bonded epoxy internal lining complying with ANSI/AWWA C213 at least 15 mils thick
5. Internal expansion sleeve limiting stop collars and pressure balanced
6. Ball joints contained in flanged retainers with seal gaskets
7. Working pressure of at least 350 psi for sizes 24 inches and smaller and 250 psi for sizes larger than 24 inches
8. NSF 61 certification

The seismic expansion assembly must be factory set at 1/2 the extension capacity.

20-2.14C(2)(h) Lateral Restraint Assemblies

Lateral restraint assemblies must be (1) constructed from commercial quality steel components complying with section 75-1.02, (2) adjustable, and (3) able to resist a horizontal force of 10 percent of the contributory dead load.

20-2.14C(3) Construction

Each ductile iron pipe must be connected and fully extended (pulled out) after joint assembly before the next pipe section is added.

Install flange insulating gaskets on the outside flange of seismic expansion assemblies and expansion assemblies.

20-2.14C(4) Payment

Supply line on structures is paid for as supply line (bridge).

20-2.15 TEMPORARY IRRIGATION SYSTEMS

Reserved

20-2.16–20-2.19 RESERVED

20-3 PLANTING

20-3.01 GENERAL

20-3.01A General

20-3.01A(1) Summary

Section 20-3 includes specifications for performing planting work in new and existing landscapes.

20-3.01A(2) Definitions

Reserved

20-3.01A(3) Submittals

20-3.01A(3)(a) General

Submit nursery invoices showing species or variety and inspection certificates for plants.

Submit documentation of clearance from the county agricultural commissioner for plants obtained from a county outside the project limits.

If a root stimulant is required, submit a copy of the root stimulant manufacturer's product sheet and instructions for the application of the root stimulant.

If cuttings are to be taken from outside the right-of-way, submit proof of permits and payment of associated fees. Notify the Engineer of the location at least 15 days before taking cuttings.

20-3.01A(3)(b) Vendor Statements

At least 60 days before planting the plants, submit a statement from the vendor that the order for the plants required, including sample plants used for inspection, has been received and accepted by the vendor. The statement from the vendor must include the plant names, sizes, and quantities and the anticipated delivery date.

20-3.01A(3)(c) Certificates of Compliance

Submit a certificate of compliance for:

1. Sod
2. Soil amendment

20-3.01A(4) Quality Control and Assurance

Plants must comply with federal and state laws requiring inspection for diseases and infestations. Inspection certificates required by law must accompany each shipment of plants.

Obtain clearance from the county agricultural commissioner before planting plants delivered from a county outside the project limits.

The Engineer inspects the roots of container-grown sample plants by removing earth from the rootball of not less than 2 plants, nor more than 2 percent of the total number of plants of each species or variety. If container-grown plants are purchased from several sources, the Engineer inspects the roots of not less than 2 of each sample plant species or variety from each source. The rootball of container grown plants must not show evidence of being underdeveloped, deformed, or having been restricted.

If the Engineer finds noncompliant plants, the entire lot represented by the noncompliant sample plants will be rejected.

Cuttings with mature or brown stems and cuttings that have been trimmed will be rejected.

20-3.01B Materials

20-3.01B(1) General

Notify the Engineer at least 10 days before the plants are shipped to the job site.

20-3.01B(2) Plants

20-3.01B(2)(a) General

Plants must be the variety and size shown and true to the type or name shown. Plants must be individually tagged or tagged in groups identifying the plants by species or variety. Tagging is not required for cuttings.

Plants must be healthy, well-formed, not root-bound, free from insect pests and disease, and grown in nurseries inspected by the Department of Food and Agriculture.

The plants must comply with the size and type shown in the following table:

Plant group designation	Description	Container size (cu in)
A	No. 1 container	152–251
B	No. 5 container	785–1242
C	Balled and burlapped	--
E	Bulb	--
F	In flats	--
H	Cutting	--
I	Pot	--
K	24-inch box	5775–6861
M	Liner[a]	--
O	Acorn	--
P	Plugs[a, b]	--
S	Seedling[c]	--
U	No. 15 container	2768–3696

[a]Do not use containers made of biodegradable material.
[b]Grown in individual container cells.
[c]Bare root.

Trucks used for transporting plants must be equipped with covers to protect plants from windburn.

Handle and pack plants in an authorized way for the species or variety.

20-3.01B(2)(b) Cuttings

20-3.01B(2)(b)(i) General

Take cuttings at random from healthy, vigorous plants. Make cuts with sharp, clean tools. Do not take more than 25 percent of an individual plant and not more than 50 percent of the plants in an area.

Keep cuttings covered and wet until planted. Do not allow cuttings to dry or wither.

Plant cuttings no more than 2 days after being cut.

20-3.01B(2)(b)(ii) *Carpobrotus* and *Delosperma* Cuttings

You may take cuttings for new *Carpobrotus* and *Delosperma* groundcover from the existing highway planting areas, but these areas may not provide enough material to complete the work. Contact the local District's encroachment permit office to obtain a permit to harvest cuttings, identify acceptable cutting harvest areas, and to determine acceptable quantities to take.

Take tip cuttings from healthy, vigorous Carpobrotus and Delosperma plants that are free of pests and disease.

Carpobrotus cuttings must be 10 inches or more in length and not have roots.

Delosperma cuttings must be 6 inches or more in length and not have roots.

20-3.01B(2)(b)(iii) Willow Cuttings

Take willow cuttings from areas shown or designated by the Engineer.

Willow cuttings must be:

1. Reasonably straight
2. 20 to 24 inches in length
3. 3/4 to 1-1/2 inch in diameter at the base of the cutting

Cut the top of each willow cutting square above a leaf bud. Cut the base below a leaf bud at approximately a 45 degree angle. Trim off leaves and branches flush with the stem of the cutting.

20-3.01B(2)(b)(iv) Cottonwood Cuttings

Cottonwood cuttings must comply with the requirements for willow cuttings in section 20-3.01B(2)(b)(iii).

20-3.01B(2)(b)(v)–20-3.01B(2)(b)(viii) Reserved

20-3.01B(2)(c) Sod

Sod must:

1. Be grown to comply with the Food & Agri Code
2. Be free from weeds and undesirable types of grasses and clovers
3. Be field-grown on soil containing less than 50 percent silt and clay
3. Have less than 1/2-inch-thick thatch
4. Not be less than 8 months or more than 16 months old
5. Be machine-cut to a uniform soil thickness of 5/8 ± 1/4 inch, not including top growth and thatch

Protect sod with tarps or other protective covers during delivery. Do not allow sod to dry out during delivery or before placement.

20-3.01B(3) Soil Amendment

Soil amendment must comply with the requirements in the Food & Agri Code. Soil amendment must be one or a combination of the following:

1. Sphagnum peat moss
2. Nitrolized fir bark
3. Vermiculite
4. Perlite

20-3.01B(4) Fertilizers

20-3.01B(4)(a) General

Deliver fertilizer in labeled containers showing weight, chemical analysis, and manufacturer's name.

Fertilizer must comply with the requirements of the Food & Agri Code.

20-3.01B(4)(b) Slow-release Fertilizers

Slow-release fertilizer must be a pelleted or granular form with a nutrient release over an 8 to 12 month period and must comply with the chemical analysis ranges shown in the following table:

Ingredient	Content (percent)
Nitrogen (N)	16–21
Phosphoric acid (P)	6–8
Water soluble potash (K)	4–10

20-3.01B(4)(c) Packet Fertilizers

Packet fertilizer must be a biodegradable packet with a nutrient release over a 12 month period. Each packet must have a weight of 10 ± 1 grams and must comply with the chemical analysis shown in the following table:

Ingredient	Content (percent)
Nitrogen(N)	20
Phosphoric acid (P)	10
Water soluble potash (K)	5

20-3.01B(4)(d) Organic Fertilizers

Organic fertilizer must be pelleted or granular with a cumulative nitrogen release rate of no more than 70 percent for the first 70 days after incubation at 86 degrees F with 100 percent at 350 days or more. Organic fertilizer must comply with the chemical analysis shown in the following table:

Ingredient	Content (percent)
Nitrogen (N)	5–7
Phosphoric acid (P)	1–5
Water soluble potash (K)	1–10

20-3.01B(5) Root Stimulants

Root stimulant must be a commercial quality product.

20-3.01B(6) Plaster Sand

Backfill material for the transplant palm tree planting holes must be 100 percent commercial quality washed plaster sand.

20-3.01B(7) Root Barrier

Root barrier must be an injection molded or extruded modular panel made of high-density polypropylene or polyethylene plastic.

Each panel must:

1. Be at least 1/16-inch thick
2. Have at least 4 molded root-deflecting vertical ribs 0.5- to 0.8-inch wide, 6 to 8 inches apart
3. Have a locking strip or an integral male-female sliding lock designed to resist slippage between panels
4. Be at least 2 feet wide and 2 feet in depth

20-3.01B(8) Root Protectors

Each root protector must be:

1. Fabricated from 1-inch, hexagonal pattern, 20-gauge mesh wire
2. Closed bottom design with a height and diameter that provides a minimum of 6 inches of clearance between the root ball and the sides and bottom of the wire cylinder

Wire edges at the top of the cylinder must be the uncut manufactured finished edge free of sharp points.

20-3.01B(9) Foliage Protectors

Each foliage protector must be:

1. Fabricated from 1-inch, hexagonal pattern, 20-gauge mesh wire
2. Approximately 4 feet high and 2 feet in diameter

Wire edges at the top of the cylinder must be the uncut manufactured finished edge free of sharp points. Other wire edges that are cut must be free of sharp points.

Support stakes must be one of the following:

1. 3/4-inch reinforcing steel bar a minimum of 5 feet long with an orange or red plastic safety cap that fits snugly onto the top of the reinforcing steel bar

2. 2 inch nominal diameter or 2 by 2 inch nominal size wood stakes a minimum of 5 feet long. Wood stakes must be straight

The jute mesh cover must comply with section 21-1.02O(2). Twine required to hold the jute mesh cover in place must be 1/8-inch diameter manila hemp twine.

20-3.01B(10) Wood Plant Stakes

Each plant stake must be nominal 2 by 2 inch or nominal 2-inch diameter and of sufficient length to keep the plant in an upright position.

Plant stakes for vines must be nominal 1 by 1 inch,18 inches long.

20-3.01B(11) Plant Ties

Plant ties must be extruded vinyl-based tape, 1 inch wide and at least 10 mils thick.

20-3.01C Construction

20-3.01C(1) General

Apply a root stimulant under the manufacturer's instructions to the plants specified in the special provisions.

Before transporting the plants to the planting area, thoroughly wet the root ball.

20-3.01C(2) Pruning

Prune plants under the latest edition of ANSI A300 part 1, *Pruning*, published by the Tree Care Industry Association.

Do not use tree seal compounds to cover pruning cuts.

20-3.01C(3) Watering

Water existing plants to be maintained, transplanted trees, and new plants as needed to keep the plants in a healthy growing condition.

20-3.01C(4) Replacement Plants

Plants that show signs of failure to grow at any time or are so injured or damaged as to render them unsuitable for the purpose intended, must be removed, replaced, and replanted. Replace unsuitable plants within 2 weeks after the Engineer marks or indicates that the plants must be replaced.

Replacement planting must comply with the original planting requirements, spacing, and size provisions described for the plants being replaced.

Replacement planting for transplanted trees must comply with the work plan and be planted in the same planting hole.

Replacement ground cover plants must be the same species specified for the ground cover being replaced. Other replacement plants must be the same species as the plants being replaced.

Place orders for replacement plants with the vendor at the appropriate time so that the replacement plants are not in a root-bound condition.

The Department does not pay for replacement plants or the planting of replacement plants.

20-3.01C(5) Maintain Plants

Maintain plants from the time of planting until Contract acceptance if no plant establishment period is specified or until the start of the plant establishment period.

20-3.01D Payment

Reserved

20-3.02 EXISTING PLANTING

20-3.02A General

20-3.02A(1) Summary

Section 20-3.02 includes specifications for pruning existing plants, transplanting trees, and maintaining existing planted areas.

Transplant palm trees between March 15 and October 15.

20-3.02A(2) Definitions

Reserved

20-3.02A(3) Submittals

Submit a work plan for:

1. Transplanting trees. The work plan must include methods for lifting, transporting, storing, planting, guying, and maintaining each tree to be transplanted. Include root ball size, method of root ball containment, and a maintenance program for each tree.
2. Maintaining existing planted areas. The work plan must include weed control, fertilization, mowing and trimming of turf areas, watering, and controlling rodents and pests.

Submit a copy of the manufacturer's product sheet for root stimulant including application instructions.

20-3.02A(4) Quality Control and Assurance

Inspect for deficiencies of existing planted areas in the presence of the Engineer. Complete the inspection within 15 days after the start of job site activities.

Deficiencies requiring corrective action include:

1. Weeds
2. Dead, diseased, or unhealthy plants
3. Missing plant stakes and tree ties
4. Inadequate plant basins and basin mulch
5. Other deficiencies needing corrective action to promote healthy plant life
6. Rodents and pests

20-3.02B Materials

Not Used

20-3.02C Construction

20-3.02C(1) General

Correct deficiencies of existing planted areas as ordered within 15 days of the order. Correction of deficiencies is change order work.

After deficiencies are corrected, perform work to maintain existing planted areas in a neat and presentable condition and to promote healthy plant growth through Contract acceptance.

20-3.02C(2) Prune Existing Plants

Prune existing plants as shown.

If no bid item for prune existing plants is included, prune existing plants as ordered. Pruning existing plants is change order work.

20-3.02C(3) Transplant Trees

Prune each tree to be transplanted immediately before lifting.

If the tree to be transplanted is a palm, prune by removing dead fronds and frond stubs from the trunk. Remove green fronds up to 2 rows of fronds away from the center of growth. Tie the remaining 2 rows of fronds in an upright position with light hemp or manila rope. Remove fronds and frond stubs at the trunk in a manner that will not injure the trunk. Remove fronds and frond stubs for *Phoenix dactylifera* (Date Palm) approximately 4 inches from the trunk.

Prepare each hole in the new location before lifting the tree to be transplanted.

Lift tree to be transplanted as described in the work plan.

Comply with section 20-3.03C(3) for handling and planting each tree to be transplanted.

Until replanted, cover exposed root ball with wet burlap or canvas and cover the crown with 90 percent shade cloth.

Replant each tree on the same day it is lifted if possible. If the transplant location is not ready to receive the tree, store and maintain the tree to be transplanted until the transplant location is authorized. Store tree in an upright position.

Replace damaged transplanted tree under 20-3.01C(4) and with the number of trees specified in the special provisions.

The replacement trees must be planted in individual plant holes at the location determined by the Engineer within the area of the tree being replaced. Comply with section 20-3.03C(2) for the planting of the replacement trees.

20-3.02C(4) Maintain Existing Planted Areas

If a bid item for maintain existing planted areas is included, the existing plant basins must be kept well-formed and free of sediment. If the existing plant basins need repairs, and the basins contain mulch, replace the mulch after the repairs are done.

Control weeds within the existing planted area and:

1. From the existing planted area limit to the adjacent edges of paving and fences if less than or equal to 12 feet
2. From the existing planted area limit to 6 feet beyond the outer limit of the existing planted area if the adjacent edge of paving or fence is more than 12 feet away
3. Within a 3-foot radius from each existing tree and shrub

If no bid item for maintain existing planted areas is included, maintain existing planted areas as ordered. Maintain existing planted areas is change order work.

20-3.02D Payment

Not Used

20-3.03 PLANTING WORK

20-3.03A General

Section 20-3.03 includes specifications for planting plants.

20-3.03B Materials

Not Used

20-3.03C Construction

20-3.03C(1) General

Do not begin planting until authorized.

If an irrigation system is required, do not begin planting in an area until the functional test has been completed and authorized for the irrigation system serving that area.

20-3.03C(2) Preparing Planting Areas

The location of each plant is as shown unless the Engineer designates otherwise. If the Engineer designates the location, it will be marked by a stake, flag, or other marker.

Conduct work so the existing flow line in drainage ditches is maintained. Material displaced by your operations that interferes with drainage must be removed.

Where a minimum distance to a drainage ditch is shown, locate the plant so that the outer edge of its basin wall is at least the minimum distance shown for each plant involved.

Excavate each planting hole by hand digging or by drilling. The bottom of each planting hole must be flat. Do not use water for excavating the hole.

Unless a larger planting hole is specified, the planting hole must be large enough to receive the root ball or the total length and width of roots, backfill, amendments, and fertilizer. Where rock or other hard material prohibits the hole from being excavated, a new hole must be excavated and the abandoned hole backfilled.

20-3.03C(3) Planting Plants

20-3.03C(3)(a) General

Do not plant plants in soil that is too wet, too dry, not properly conditioned as specified, or in an unsatisfactory condition for planting.

Do not distribute more plants than can be planted and watered on that day.

Water plants immediately after planting. Apply water until the backfill soil around and below the roots or ball of earth around the roots of each plant is thoroughly saturated. When watering with a hose, use a nozzle, water disbursement device, or pressure reducing device. Do not allow the full force of the water from the open end of the hose to fall within the basin around any plant. Groundcover plants in areas with an irrigation system must be watered by sprinklers. Several consecutive watering cycles may be necessary to thoroughly saturate the soil.

If shown, install root barriers between trees and concrete sidewalk or curb. Install panels flush with finished grade and join with locking strips or integral male-female sliding locks. Install barriers with root deflectors facing inward.

If a tree grate is shown, install root barrier panels 0.5 inch above finish grade or as shown.

Adjust planting locations so that each tree or shrub is at least 8 feet away from any sprinkler.

Where a tree, shrub, or vine is to be planted within a groundcover area or cutting planting area, plant it before planting groundcover or cuttings.

Where shrubs and groundcovers are shown to be planted in groups, the outer rows directly adjacent to the nearest roadway or highway fence must be parallel to the nearest roadway or highway fence. Stagger shrubs and groundcovers in adjacent rows. Adjust the alignment of the plants within the outer rows.

Core holes in concrete masonry block wall as shown.

Where a vine is to be planted against a wall or fence, plant it as close as possible to the wall or fence. If a vine planted next to a wall is to be staked, stake and tie the vine at the time of planting. A vine planted next to a fence must be tied to the fence at the time of planting.

Protect tree trunks from injury. Do not:

1. Drag tree
2. Use chains to move a tree
3. Lay tree on the ground

20-3.03C(3)(b) Trees, Shrubs, and Vines

After preparing holes, thoroughly mix soil amendment and granular fertilizer at the rate shown with native soil to be used as backfill material. Remove containers from plants in such a manner that the ball of earth surrounding the roots is not broken. Do not cut plant containers before delivery of the plants to the planting area. Plant and water plants immediately after removal from their containers.

Place packet fertilizer in the backfill within 6 to 8 inches of the ground surface and approximately 1 inch from the root ball. If more than 1 packet is required per plant, distribute the packets evenly around the root ball.

If a root stimulant is to be used, apply it according to the manufacturer's instructions.

If required, install root protectors in the plant holes as shown.

Ensure roots are not restricted or distorted.

Distribute backfill uniformly throughout the entire depth of the plant hole without clods or lumps. After the planting holes have been backfilled, jet water into the backfill with a pipe or tube inserted into the bottom of the hole until the backfill material is saturated for the full depth. If the backfill material settles below this level, add additional backfill to the required level. If a plant settles deeper than shown, replant it at the required level.

Remove nursery stakes after planting.

Install 2 plant stakes for each plant to be staked at the time of planting as shown. Ensure the rootball is not damaged.

Tie the plant to the stakes with 2 plant ties, 1 tie to each stake. Each tie must form a figure 8 by crossing the tie between the plant and the stake as shown. Install ties at the lowest position that will support the plant in an upright position. Ties must provide trunk flexibility but not allow the trunk to rub against the stakes. Wrap each end of the tie 1-1/2 turns around the stake and securely tie.

Construct a watering basin around each plant as shown.

If required, install a foliage protector:

1. Over the plant within 2 days after planting.
2. Vertically and centered over the plant as shown

If foliage protectors are required:

1. Cut the bottom of the wire cylinder to match the slope of the ground. Do not leave sharp points of wire after cutting. Sharp points must be bent over or blunted.
2. Install 2 support stakes for foliage protectors vertically and embed in the soil on opposite sides of the plant as shown and in a transverse direction to the prevailing wind.
3. Either weave the support stakes through the wire cylinder mesh at 6 inch maximum centers or fasten the wire cylinder to the support stakes at 6 inch maximum centers.
4. Wire cylinder must be snug against the support stakes but loose enough to be raised for pesticide application or to perform weeding within the plant basin.
5. Install jute mesh cover over the foliage protector and secure with twine as shown.

20-3.03C(3)(c) Groundcover Plants

Each groundcover planting area irrigated by a single control valve must be completely planted and watered before planting other groundcover planting areas.

Plant groundcover plants in moist soil, and in neat, straight rows, spaced as shown.

Apply fertilizer to groundcover plants and water into the soil immediately after planting.

20-3.03C(3)(d) Cuttings, Liners, Plugs, and Seedling Plants

20-3.03C(3)(d)(i) General

Apply fertilizer to cuttings, liners, plugs, and seedling plants and water immediately after planting.

Ensure the soil is moist to a minimum depth of 8 inches before planting cuttings.

If a root stimulant is to be used, apply it according to the manufacturer's instructions.

20-3.03C(3)(d)(ii) Willow Cuttings

Unless otherwise shown, for willow cuttings excavate planting holes perpendicular to the ground line by using a steel bar, auger, post hole digger, or similar tools. Holes must be large enough to receive the cuttings and fertilizer packet. Plant willow cuttings to the specified depths without damaging the bark.

Where rock or other hard material prohibits the excavation of the planting holes, excavate new holes and backfill the unused holes.

Plant willow cuttings during the period specified in the special provisions.

Apply root stimulant according to the manufacturer's instructions.

Plant the base of the cutting 10 to 12 inches deep with 3 to 5 bud scars exposed above the ground. If more than 5 bud scars are exposed, trim off the excess willow cutting length.

Place 1 fertilizer packet in the backfill of each cutting, 6 to 8 inches below the ground surface and approximately 1 inch from the cutting.

Backfill the plant holes with excavated material after planting. Distribute the excavated material evenly within the hole without clods, lumps, or air pockets. Compact the backfill so that the cutting cannot be easily removed from the soil. Do not damage the cutting's bark.

Dispose of trimmings and unused cuttings.

20-3.03C(3)(d)(iii) Cottonwood Cuttings

Reserved

20-3.03C(3)(d)(iv) *Carpobrotus* and *Delosperma* Cuttings

Plant *Carpobrotus* cuttings to a depth so that not less than 2 nodes are covered with soil. The basal end of *Delosperma* cuttings must not be less than 2 inches below the surface of the soil and the basal end of *Carpobrotus* cuttings must not be less than 4 inches below the surface of the soil.

Apply root stimulant to *Delosperma* cuttings before planting.

Do not plant *Carpobrotus* or *Delosperma* cuttings in soil that does not contain sufficient moisture at an average depth of 2 inches below the surface.

20-3.03C(3)(d)(v) Liner Plants

Plant liner plants during the period specified in the special provisions.

If a foliage protector is required, install under section 20-3.03C(3)(b).

20-3.03C(3)(d)(vi) Plug Plants

Plant plug plants during the period specified in the special provisions.

20-3.03C(3)(d)(vii) Seedling Plants

Plant seedling plants during the period specified in the special provisions.

20-3.03C(3)(e) Sod

After all other planting is performed, grade sod areas to drain and to a smooth and uniform surface. Fine grade and roll sod areas before placing sod.

Areas adjacent to sidewalks, edging, and other paved borders and surfaced areas must be 1 inch below the finished surface elevation of the facilities, after fine grading, rolling, and settlement of the soil.

Place sod such that the end of each adjacent strip is staggered a minimum of 2 feet. Place the edge and end of sod firmly against adjacent sod and against sidewalks, edging, and other paved borders and surfaced areas.

Lightly roll the entire sodded area to eliminate air pockets and ensure close contact with the soil after placement of sod. Water the sodded areas so that the soil is moist to a minimum depth of 4 inches after rolling. Do not allow the sod to dry out.

If irregular or uneven areas appear in the sodded areas, restore to a smooth and even appearance.

Trim sod to a uniform edge at sidewalks, edging, and other paved borders and surfaced areas. Trimming must be repeated whenever the edge of sod extends 1 inch beyond the edge of the edging, sidewalks, and other paved borders and surfaced areas. Remove and dispose of trimmed sod.

Mow sod when it has reached a height of 4 inches. Mow sod to a height of 2.5 inches.

20-3.03D Payment

Soil amendment is measured in the vehicle at the point of delivery.

Measurement for slow-release fertilizer, organic fertilizer, or iron sulfate is determined from marked weight or sack count.

Various sizes and types of plants are measured by either the product of the average plant density and the total area planted or by actual count of the living plants in place, determined by the Engineer. The average plant density is the number of living plants per sq yd determined from actual count of test areas chosen representing the total planted area. The size and location of the test areas is determined by you and the Engineer, except that the total area tested must be equal to not less than 3 percent nor more than 5 percent of the planted area being determined. The Engineer makes the final determination of the areas to be tested.

20-3.04–20-3.08 RESERVED

20-4 PLANT ESTABLISHMENT WORK

20-4.01 GENERAL

20-4.01A Summary

Section 20-4 includes specifications for performing plant establishment work.

Plant establishment consists of caring for the plants, including watering, fertilizing, pruning, replacing damaged plants, pest control, and operating and repairing of all existing irrigation facilities used and irrigation facilities installed as part of the new irrigation system.

Working days on which no work is required, as determined by the Engineer, will be credited as a plant establishment working day, regardless of whether or not you perform plant establishment work.

Working days whenever you fail to adequately perform plant establishment work will not be credited toward the plant establishment working days.

20-4.01B Definitions

Type 1 plant establishment: Plant establishment period with the number of working days specified for plant establishment beginning after all work has been completed except for plant establishment work and other bid items specified to be performed until Contract acceptance.

Type 2 plant establishment: Plant establishment period with the number of working days specified for plant establishment beginning after all planting work has been completed except for plant establishment work and other bid items specified to be performed until Contract acceptance, provided that the Contract must not be accepted unless the plant establishment work has been satisfactorily performed for at least the number of working days specified for plant establishment.

If maintenance and protection relief is granted for a completed portion of the work under section 5-1.38, Type 2 plant establishment period for the completed portion of the work is the time between completion of all planting work except for plant establishment work, and the granting of maintenance and protection relief, provided that the relief must not be granted unless the plant establishment work in the completed portion of the work has been satisfactorily performed for at least the number of working days specified for the plant establishment period.

20-4.01C Submittals

20-4.01C(1) General

Submit seasonal watering schedules for use during the plant establishment period within 10 days after the start of the plant establishment period. Remote irrigation control system watering schedule must utilize the remote irrigation control system software program.

Submit updated watering schedules within 5 business days after any changes have been made to the authorized schedules.

Submit a revised watering schedule for each irrigation controller not less than 30 days before completion of the plant establishment period.

20-4.01C(2) Notification

The Engineer will notify you in writing when the plant establishment period begins and will furnish statements regarding the number of working days credited to the plant establishment period after the notification.

Notify the Engineer at least 5 business days before applying each application of fertilizer.

20-4.01D Quality Control and Assurance

Provide training by a qualified person on the use and adjustment of the irrigation controllers installed, 30 days before completion of the plant establishment period.

Perform a final inspection of the plant establishment work in the presence of the Engineer between 20 and 30 days before Contract acceptance.

20-4.02 MATERIALS

20-4.02A General

Reserved

20-4.02B Fertilizers

Fertilizer must comply with section 20-3.01B(5).

20-4.03 CONSTRUCTION

20-4.03A General

Remove trash and debris.

Surplus earth accumulated in roadside clearing and planting areas must be removed.

Trim and mow turf areas as specified for sod in section 20-3.03C(3)(e). Dispose of trimmed and mowed material.

If irregular or uneven areas appear within turf areas, restore to a smooth and even appearance. Reseed turf seed areas.

Remove the tops of foliage protectors if plants become restricted.

Remove foliage protectors, including support stakes, within 30 days before the completion of the plant establishment period.

Keep plant basin walls well formed.

Clean new wye strainers and existing wye strainers that are a part of the new irrigation system annually until the completion of the plant establishment period. The last cleaning must be done within 15 days before the completion of the plant establishment period.

Remove, clean, and reinstall new filters and existing filters that are a part of the new irrigation system annually until the completion of the plant establishment period. The last cleaning must be done within 15 days before the completion of the plant establishment period.

20-4.03B Plant Growth Control

Prune plants planted as part of the Contract as authorized.

Remove plant growth that extends within 2 feet of sidewalks, curbs, dikes, shoulders, walls or fences.

Remove proposed and existing ground cover from within the plant basins, including basin walls, turf areas, and planting areas within edging.

Vines next to walls and fences must be kept staked and tied. Train vines on fences and walls or through cored holes in walls.

20-4.03C Fertilizers

Apply fertilizer to the plants as specified and water into the soil after each application.

Apply fertilizer at the rates shown and spread with a mechanical spreader, whenever possible.

20-4.03D Weed Control

Control weeds under section 20-1.03C(3).

20-4.03E Plant Staking

Replace the plant stakes that are inadequate to support plants with larger stakes.

Remove plant stakes when the Engineer determines they are no longer needed.

20-4.03F Replacement Plants

Replacement plants must comply with section 20-3.01C(4).

Replacement of plants up to and including the 125th plant establishment working day must be with a plant of the same size as originally specified. Plants of a larger container size than those originally specified for replacement plants may be used during the first 125 working days of the plant establishment period.

Replacement of plants after the 125th plant establishment working day must comply with the following size requirements:

Plant size (Original)	Plant size (Replacement)
Pot/liner/plug/ seedling	No. 1 container
No. 1 container	No. 5 container
No. 5 container	No. 15 container

Other replacement plants must be the same size as originally specified.

Replacement ground cover plants must comply with the following spacing requirements:

Original spacing (inches)	On center spacing of replacement ground cover plants (inches)		
	Number of completed plant establishment working days		
	1–125	126–190	191–End of plant establishment period
9	9	6	6
12	12	9	6
18	18	12	9
24	24	18	12
36	36	24	18

20-4.03G Watering

Operate the electric automatic irrigation systems in the automatic mode unless authorized.

If any component of the electric automatic irrigation system is operated manually, the day will not be credited as a plant establishment working day unless the manual operation is authorized.

Water plants utilizing the remote irrigation control system software program unless authorized.

Implement the watering schedule at least 10 days before completion of the plant establishment period.

20-4.04 PAYMENT

Not Used

20-5 LANDSCAPE ELEMENTS

20-5.01 GENERAL

20-5.01A General

Section 20-5 includes specifications for constructing and installing landscape elements.

20-5.01B Materials

Not Used

20-5.01C Construction

Earthwork must comply with section 19.

20-5.01D Payment

Not Used

20-5.02 EDGING

20-5.02A General

Section 20-5.02 includes specifications for constructing landscape edging.

20-5.02B Materials

20-5.02B(1) General

Reserved

20-5.02B(2) Header Board Edging

Lumber for header board edging must be one of the following types:

1. Construction grade cedar
2. Pressure-treated Douglas fir
3. Construction heart grade redwood complying with section 57-2.01B(2)

Lumber must be:

1. Rough cut from sound timber.
2. Straight. Sweep must not exceed 1 inch in 6 feet.
3. Free from loose or unsound knots. Knots must be sound, tight, well spaced, and not to exceed 2 inches in size on any face.
4. Free of shakes in excess of 1/3 the thickness of the lumber.
5. Free of splits longer than the thickness of the lumber.
6. Free of other defects that would render the lumber unfit structurally for the purpose intended.

Edging anchors for header board edging must be stakes of the size and shape shown.

20-5.02B(3) Metal Edging

Metal edging must be commercial quality, made of aluminum or steel, and have an L-shaped design. Edging must be a minimum of 4 inches in height. The thickness must be as recommended by the manufacturer for the use intended.

Edging anchors must be from the same manufacturer as the metal edging.

20-5.02B(4) High Density Polyethylene Edging

HDPE edging must be commercial quality and a minimum of 4 inches in height. The thickness must be as recommended by the manufacturer for commercial installation for the use intended.

Edging anchors must be from the same manufacturer as HDPE edging.

20-5.02B(5) Concrete Edging

Concrete for edging must be minor concrete.

20-5.02B(6)–20-5.02B(10) Reserved

20-5.02C Construction

20-5.02C(1) General

Where edging is used to delineate the limits of inert ground cover or mulch areas, install edging before installing inert ground cover or mulch areas.

Saw cut surfaces where (1) asphalt concrete or concrete surfacing must be removed to permit the installation of edging and (2) no joint exists between the surfacing to be removed and the surfacing to

remain in place. The surfacing must be cut in a straight line to a minimum depth of 2 inches with a power-driven saw before the surfacing is removed. Spike or stake spacing must comply with the manufacturer's instructions for use and site conditions.

20-5.02C(2) Header Board Edging

Each stake must be driven flush with the top edge of the header board edging and the stake top must be beveled away from the header board at a 45 degree angle. Attach stake to header board with a minimum of two 12-penny hot dipped galvanized nails per stake.

20-5.02C(3) Metal and High Density Polyethylene Edging

Spike or stake spacing must comply with the manufacturer's instructions for use and site conditions.

20-5.02C(4) Concrete Edging

Construct and finish minor concrete edging under section 73-2.

20-5.02C(5)–20-5.02C(9) Reserved

20-5.02D Payment

Edging is measured parallel to the ground surface.

20-5.03 INERT GROUND COVERS AND MULCHES

20-5.03A General

20-5.03A(1) General

20-5.03A(1)(a) Summary

Section 20-5.03 includes specifications for installing inert ground covers and mulches.

20-5.03A(1)(b) Definitions

Reserved

20-5.03A(1)(c) Submittals

Submit:

1. Filter fabric product data including the manufacturer's product sheet and installation instructions
2. Certificate of compliance for filter fabric at least 5 business days before delivery of the material to the job site

20-5.03A(1)(d) Quality Control and Assurance

Reserved

20-5.03A(2) Materials

Soil sterilant must be oxadiazon granular preemergent and must comply with section 20-1.02C.

Filter fabric must be Class A. Staples for filter fabric must comply with section 21-1.02R.

20-5.03A(3) Construction

20-5.03A(3)(a) General

Before performing inert ground cover and mulch work, remove plants and weeds to ground level.

20-5.03A(3)(b) Earthwork

Excavate areas to receive inert ground cover or mulch to the depth shown. Maintain the planned flow lines, slope gradients, and contours of the job site. Grade subgrade to a smooth and uniform surface and compact to not less than 90 percent relative compaction.

20-5.03A(3)(c) Treatment of Soil

After compaction, apply soil sterilant at the maximum label rate. Do not apply soil sterilant more than 12 inches beyond the inert ground cover or mulch limits. The soil sterilant application and inert ground cover or mulch placement must be completed within the same work day.

20-5.03A(3)(d) Filter Fabric

Immediately before placing filter fabric, surfaces to receive filter fabric must be free of loose or extraneous material and sharp objects that may damage the filter fabric during installation.

Align fabric and place in a wrinkle-free manner.

Overlap adjacent rolls of the fabric from 12 to 18 inches. Spread each overlapping roll in the same direction. Fasten fabric with staples flush with the adjacent fabric to prevent movement of fabric by placement of inert ground cover or mulch.

Repair or replace fabric damaged during placement of inert ground cover or mulch with sufficient fabric to comply with overlap requirements.

20-5.03A(4) Payment

Not Used

20-5.03B Rock Blanket

20-5.03B(1) General

20-5.03B(1)(a) Summary

Section 20-5.03B includes specifications for placing rock blanket.

20-5.03B(1)(b) Definitions

Reserved

20-5.03B(1)(c) Submittals

Submit a 1 sq yd sample of the various rock sizes.

20-5.03B(1)(d) Quality Control and Assurance

Reserved

20-5.03B(2) Materials

20-5.03B(2)(a) General

Do not use filter fabric.

20-5.03B(2)(b) Concrete

Concrete must be minor concrete.

20-5.03B(2)(c) Rock

Rock must be clean, smooth, and obtained from a single source and must comply with the following grading requirements:

Grading Requirements

Screen size (inches)	Percentage passing
8	100
6	50-85
4	0-50

20-5.03B(2)(d) Mortar

Mortar must comply with section 51-1.02F.

20-5.03B(3) Construction

Place concrete as shown.

Rock must be placed while concrete is still plastic. Remove concrete adhering to the exposed surfaces of the rock.

Loose rocks or rocks with a gap greater than 3/8 inch must be reset by an authorized method. The rock gap is measured from the edge of the rock to the surrounding concrete bedding.

Place mortar as shown.

20-5.03B(4) Payment

Rock blanket is measured parallel to the rock blanket surface.

20-5.03C Gravel Mulch

20-5.03C(1) General

20-5.03C(1)(a) Summary

Section 20-5.03C includes specifications for placing gravel mulch.

20-5.03C(1)(b) Definitions

Reserved

20-5.03C(1)(c) Submittals

Submit a 5-lb sample of the gravel mulch.

20-5.03C(1)(d) Quality Control and Assurance

Reserved

20-5.03C(2) Materials

Gravel mulch must be:

1. Uniform gray color
2. From a single source only
3. Crushed rock that complies with the following grading requirements:

Grading Requirements

Sieve size	Percent passing
1-1/4 inch	100
3/4 inch	60-80
1/2 inch	45-65
No. 40	5-20

20-5.03C(3) Construction

Place gravel and compact by rolling.

The finished gravel mulch surface must be smooth and uniform, maintaining original flow lines, slope gradients, and contours of the job site.

20-5.03C(4) Payment

Gravel mulch is measured parallel to the gravel mulch surface.

20-5.03D Decomposed Granite

20-5.03D(1) General

20-5.03D(1)(a) Summary

Section 20-5.03D includes specifications for placing decomposed granite.

20-5.03D(1)(b) Definitions

Reserved

20-5.03D(1)(c) Submittals

Five business days before delivery of the materials to the job site, submit:

1. Solidifying emulsion product data including the manufacturers' product sheets and installation instructions
2. Certificate of compliance for solidifying emulsion
3. 5-lb sample of the decomposed granite

20-5.03D(1)(d) Quality Control and Assurance

Test plot must be:

1. Constructed at an authorized location
2. At least 3 by 12 feet
3. Constructed using the materials, equipment, and methods to be used in the work
4. Authorized before starting work

Notify the Engineer not less than 7 days before constructing the test plot.

The Engineer uses the authorized test plot to determine acceptability of the work.

If ordered, prepare additional test plots. Additional test plots are change order work.

If the test plot is not incorporated into the work, the Engineer may order you to remove it.

20-5.03D(2) Materials

20-5.03D(2)(a) General

Decomposed granite must be:

1. Uniform gray or tan color
2. From one source only
3. Crushed granite rock that complies with grading requirements shown in the following table:

Grading Requirements

Sieve size	Percent passing
3/8 inch	100
No. 4	95–100
No. 8	75–80
No. 16	55–65
No. 30	40–50
No. 50	25–35
No. 100	20–25
No. 200	5–15

Note:
Grading based upon AASHTO T11-82 and T27-82

20-5.03D(2)(b) Solidifying Emulsion

Solidifying emulsion must be either a water-based polymer or nontoxic organic powdered binder specifically manufactured to harden decomposed granite. The solidifying emulsion must not alter the decomposed granite color.

20-5.03D(3) Construction

Do not place decomposed granite during rainy conditions.

Mix solidifying emulsion thoroughly and uniformly throughout the decomposed granite and under the manufacturer's instructions. Mix the material in the field using portable mixing equipment, or delivered in mixer trucks from a local ready-mixed plant.

Place decomposed granite uniformly in layers no more than 1-1/2 inch thick. Compact each layer of decomposed granite to a relative compaction of not less than 90 percent. Begin compaction within 6 to 48 hours of placement.

If the material was mixed in the field, apply an application of solidifying emulsion after compaction as recommended by the manufacturer. Prevent runoff or overspray of solidifying emulsion onto adjacent paved or planting areas.

The finished decomposed granite surface must be smooth and uniform, compacted to a relative compaction of not less than 90 percent, maintaining original flow lines, slope gradients, and contours of the job site.

20-5.03D(4) Payment

Not Used

20-5.03E Wood Mulch

20-5.03E(1) General

20-5.03E(1)(a) Summary

Section 20-5.03E includes specifications for placing wood mulch.

20-5.03E(1)(b) Definitions

Reserved

20-5.03E(1)(c) Submittals

Submit a certificate of compliance for mulch.

Submit a 2 cu ft mulch sample with the mulch source listed on the bag and obtain approval before delivery of mulch to the job site.

20-5.03E(1)(d) Quality Control and Assurance

Reserved

20-5.03E(2) Materials

20-5.03E(2)(a) General

Mulch must not contain more than 0.1 percent of deleterious materials such as rocks, glass, plastics, metals, clods, weeds, weed seeds, coarse objects, sticks larger than the specified particle size, salts, paint, petroleum products, pesticides or other chemical residues harmful to plant or animal life.

Do not use filter fabric.

20-5.03E(2)(b) Tree Bark Mulch

Tree bark mulch must be derived from cedar, Douglas fir, or redwood species.

Tree bark mulch must be ground so that at least 95 percent of the material by volume is less than 2 inches and no more than 30 percent by volume is less than 1 inch.

20-5.03E(2)(c) Wood Chip Mulch

Wood chip mulch must:

1. Be derived from clean wood
2. Not contain leaves or small twigs
3. Contain at least 95 percent wood chips by volume with average thickness of 1/16 to 3/8 inch in any direction and 1/2 to 3 inches in length

20-5.03E(2)(d) Shredded Bark Mulch

Shredded bark mulch must:

1. Be derived from trees
2. Be a blend of loose, long, thin wood, or bark pieces
3. Contain at least 95 percent wood strands by volume with average thickness of 1/8 to 1-1/2 inches in any direction and 2 to 8 inches in length

20-5.03E(2)(e) Tree Trimming Mulch

Tree trimming mulch must:

1. Be derived from chipped trees and may contain leaves and small twigs.
2. Contain at least 95 percent material by volume less than 3 inches and no more than 30 percent by volume less than 1 inch

20-5.03E(2)(f)–20-5.03E(2)(j) Reserved

20-5.03E(3) Construction

Spread mulch placed in areas outside of plant basins to a uniform thickness as shown.

Mulch must be placed at the rate described and placed in the plant basins or spread in areas as shown after the plants have been planted. Mulch placed in plant basins must not come in contact with the plant crown and stem.

Spread mulch from the outside edge of the proposed plant basin or plant without basin to the adjacent edges of shoulders, paving, retaining walls, dikes, edging, curbs, sidewalks, walls, fences, and existing plantings. If the proposed plant or plant without basin is 12 feet or more from the adjacent edges of shoulders, paving, retaining walls, dikes, edging, curbs, sidewalks, walls, fences, and existing plantings, spread the mulch 6 feet beyond the outside edge of the proposed plant basin or plant without basin.

Do not place mulch within 4 feet of:

1. Flow line of earthen drainage ditches
2. Edge of paved ditches
3. Drainage flow lines

20-5.03E(4) Payment

Mulch is measured in the vehicle at the point of delivery.

20-5.03F–20-5.03J Reserved

20-5.04 RESERVED

Reserved

20-5.05 SITE FURNISHINGS

20-5.05A General

Section 20-5.05 includes specifications for installing site furnishings.

20-5.05B–20-5.05Z Reserved

20-5.06–20-5.10 RESERVED

^^

21 EROSION CONTROL

07-19-13

Replace ", bonded fiber matrix, and polymer-stabilized fiber matrix" in the 1st paragraph of section 21-1.01B with:

04-20-12

and bonded fiber matrix

04-20-12

Delete the last paragraph of section 21-1.02E.

Replace section 21-1.02F(2) with:

04-20-12

21-1.02F(2) Reserved

Replace "20-7.02D(1)" in the 1st paragraph of section 21-1.02H with:

07-19-13

20-3.01B(4)

Replace section 21-1.02J with:

04-20-12

21-1.02J Reserved

Replace the row for organic matter content in the table in the 4th paragraph of section 21-1.02M with:

01-18-13

Organic matter content	TMECC 05.07-A Loss-on-ignition organic matter method (LOI) % dry weight basis	30–100

Replace the paragraph in section 21-1.02P with:

10-19-12

Fiber roll must be a premanufactured roll filled with rice or wheat straw, wood excelsior, or coconut fiber. Fiber roll must be covered with biodegradable jute, sisal, or coir fiber netting secured tightly at each end and must be one of the following:

1. 8 to 10 inches in diameter and at least 1.1 lb/ft
2. 10 to 12 inches in diameter and at least 3 lb/ft

Fiber roll must have a minimum functional longevity of 1 year.

Add between the 1st and 2nd paragraphs of section 21-1.03A:

01-18-13

Remove and dispose of trash, debris, and weeds in areas to receive erosion control materials.

Remove and dispose of loose rocks larger than 2-1/2 inches in maximum dimension unless otherwise authorized.

Protect the traveled way, sidewalks, lined drainage channels, and existing vegetation from overspray of hydraulically-applied material.

Replace section 21-1.03B with:

01-18-13

21-1.03B Reserved

Replace "3 passes" in item 2 in the list in the 2nd paragraph of section 21-1.03G with:

04-19-13

2 passes

Replace section 21-1.03I with:

04-20-12

21-1.03I Reserved

Add between the 4th and 5th paragraphs of section 21-1.03P:

10-19-12

If soil conditions do not permit driving the stakes into the soil, drill pilot holes to facilitate driving of the

stakes.

01-18-13

Delete the 1st and 2nd sentences of the 3rd paragraph in section 21-1.04.

^^^

4. Type and source of ingredients used
5. Age and strength from compression strength results

Field qualification test reports must be signed by the official in responsible charge of the laboratory performing the tests.

28-2.01D Quality Control and Assurance

28-2.01D(1) General

Stop LCB activities and immediately notify the Engineer whenever:

1. Any quality control or acceptance test result does not comply with the specifications
2. Visual inspection shows noncompliant LCB

If LCB activities are stopped, before resuming activities:

1. Inform the Engineer of the adjustments you will make
2. Remedy or replace the noncompliant LCB
3. Obtain authorization

Molds for compressive strength testing under ASTM C 31 or ASTM C 192 must be 6 by 12 inches.

Quality control and assurance for cementitious materials and admixtures must comply with section 90-1.01D(1)

28-2.01D(2) Aggregate Qualification Testing

Qualify the aggregate for each proposed aggregate source and gradation. Qualification tests include (1) sand equivalent and (2) average 7-day compressive strength under ASTM C 39 on 3 specimens manufactured under ASTM C 192. The cement content for this test must be 300 lb/cu yd, and the 7-day average compressive strength must be at least 610 psi. Cement must be Type II portland cement under section 90-1.02B(2).

LCB must have from 3 to 4 percent air content during aggregate qualification testing.

28-2.01D(3) Field Qualification Testing

Before placing LCB, you must perform field qualification testing and obtain authorization for each mix design. Retest and obtain authorization for changes to authorized mixed designs.

Proposed mix designs must be field qualified before you place the LCB represented by those mix designs. Use an American Concrete Institute (ACI) certified "Concrete Laboratory Technician, Grade I" to perform field qualification tests and calculations.

Notify the Engineer at least 5 days before field qualification. Perform field qualification within the job site or a location authorized by the Engineer.

Field qualification testing includes compressive strength, air content, and penetration or slump in compliance with the table titled "Quality Control Requirements."

Field qualification testing for compressive strength must comply with the following:

1. Manufacture 12 cylinders under ASTM C 31 from a single batch
2. Perform 3 tests; each test consists of determining the average compressive strength of 2 cylinders at 7 days under ASTM C 39
3. The average compressive strength for each test must be at least 530 psi

If you submitted a notice to produce LCB qualifying for a transverse contraction joint waiver, manufacture additional specimens and test LCB for compressive strength at 3 days. Prepare compressive strength cylinders under ASTM C 31 at the same time using the same material and procedures as the 7-day compressive strength cylinders except do not submit 6 additional test cylinders. The average 3-day compressive strength for each test must be not more than 500 psi.

28-2.01D(4) Quality Control Testing

Provide a testing laboratory to perform quality control tests. Maintain sampling and testing equipment in proper working condition. Perform sampling under California Test 125.

Testing laboratories and testing equipment must comply with the Department's Independent Assurance Program.

Perform quality control sampling, testing, and inspection throughout LCB production and placement. LCB must comply with the requirements for the quality characteristics shown in the following table:

Quality Control Requirements

Quality characteristic	Test method	Minimum sampling and testing frequency	Requirement
Sand equivalent (min)	ASTM D 2419	1 per 500 cubic yards but at least 1 per day of production	18
Aggregate gradation	ASTM C 136		Note a
Air content (max, percent) [b]	ASTM C 231		4
Penetration (inches)	ASTM C 360		0 to 1-1/2 nominal [c, d]
Slump (inches)	ASTM C 143		0–3 nominal [c, d]
Compressive strength (min, psi at 7 days)	ASTM C 39 [e]		530
Compressive strength (max, psi at 3 days) [f]	ASTM C 39 [e]		500

[a] Comply with the table titled "Aggregate Grading" in section 28-2.02C.
[b] If no single test in the first 5 air content tests exceeds 1-1/2 percent, no further air content tests are required.
[c] Maximum penetration must not exceed 2 inches and maximum slump must not exceed 4 inches
[d] Test for either penetration or slump
[e] Prepare cylinders under ASTM C 31
[f] Only applicable if you (1) submitted a notice stating intent to produce LCB qualifying for a transverse contraction joint waiver and (2) successfully field qualified the LCB for 3-day compressive strength. Make cylinders at the same time using the same material and procedures as QC testing for 7-day compressive strength.

28-2.01D(5) Acceptance Criteria

For acceptance, properties of LCB must comply with values shown in the following table:

Acceptance Criteria Testing

Property	Test method	Value
Compressive strength (min, psi at 7 days)	ASTM C 39 [a]	530 [b]

[a] Cylinders prepared under ASTM C 31
[b] A compressive strength test represents up to (1) 1,000 cu yd or (2) 1 day's production if less than 1,000 cu yd.

28-2.02 MATERIALS

28-2.02A General

Water must comply with section 90-1.02D.

The air content in LCB must not exceed 4 percent. If the aggregate used for LCB is produced from processed reclaimed asphalt concrete or other material that may cause the air content to exceed 4 percent, reduce the air content with an admixture.

A water-reducing chemical admixture may be used. Water-reducing chemical admixture must comply with ASTM C 494, Type A or Type F.

Air-entraining admixtures must comply with section 90-1.02E.

28-2.02B Cementitious Material

Portland cement must comply with section 90-1.02B. Portland cement content must not exceed 300 lb/cu yd.

SCM must comply with section 90-1.02B except the equations for SCM content under 90-1.02B(3) do not apply.

For aggregate qualification testing, use Type II portland cement under section 90-1.02B(2) without SCM.

28-2.02C Aggregate

Aggregate must be clean and free from decomposed material, organic material, and other deleterious substances. Aggregate samples must not be treated with lime, cement, or chemicals before testing for sand equivalent.

Use either 1-1/2 inch or 1 inch grading. Do not change your selected aggregate grading without authorization.

When tested under ASTM C 136, the percentage composition by weight of the aggregate must comply with the grading requirements for the sieve sizes shown in the following table:

Aggregate Grading

Sieve sizes	Percentage passing			
	1-1/2" maximum		1" maximum	
	Operating range	Contract compliance	Operating range	Contract compliance
2"	100	100	--	--
1-1/2"	90-100	87-100	100	100
1"	--	--	90-100	87-100
3/4"	50-85	45-90	50-100	45-100
3/8"	40-75	35-80	40-75	35-80
No. 4	25-60	20-65	35-60	30-65
No. 30	10-30	6-34	10-30	6-34
No. 200	0-12	0-15	0-12	0-15

Aggregate must comply with the quality requirements shown in the following table:

Aggregate Quality

Property	Test Method	Operating range	Contract compliance
Sand equivalent (min)	ASTM D 2419	21	18
Compressive strength (min, psi at 7 days)	ASTM C 192 ASTM C 39	--	610 at 300 lb/cu yd cement content

Note: Cement must be Type II portland cement under section 90-1.02B(2).

If the aggregate grading or the sand equivalent test results, or both comply with contract compliance requirements but not operating range requirements, you may continue placing LCB for the remainder of the work day. Do not place additional LCB until you demonstrate the LCB to be placed complies with the operating range requirements.

28-2.03 CONSTRUCTION

28-2.03A General

Do not allow traffic or equipment on the LCB for at least 72 hours after the 1st application of the curing compound and completion of contraction joints. Limit traffic and equipment on the LCB to that is required for placing additional layers of LCB or paving.

28-2.03B Subgrade

Immediately before spreading LCB, the subgrade must:

1. Comply with the specified compaction and elevation tolerance for the material involved

2. Be free from loose or extraneous material
3. Be uniformly moist

Areas of subgrade lower than the grade established by the Engineer must be filled with LCB. The Department does not pay for filling low areas of subgrade.

28-2.03C Proportioning, Mixing, and Transporting

Proportion LCB under section 90-1.02F except aggregate does not have to be separated into sizes.

Mix and transport LCB under section 90-1.02G except the 5th and 7th paragraphs in section 90-1.02G(6) do not apply.

28-2.03D Placing

Place LCB under section 40-1.03H(1) except the 3rd paragraph does not apply.

Unless otherwise described, construct LCB in minimum widths of 12 feet separated by construction joints. For LCB constructed monolithically in widths greater than 26 feet, construct a longitudinal contraction joint offset no more than 3 feet from the centerline of the width being constructed.

Contraction joints must comply with section 40-1.03D(3).

Construct transverse contraction joints in intervals that result in LCB areas where the lengths and widths are within 20 percent of each other. Measure the widths from any longitudinal construction or longitudinal contraction joints.

The Engineer waives the requirement for transverse contraction joints if you:

1. Submitted a notice under 28-2.01C(1)
2. Successfully field qualified LCB for 3-day compressive strength testing
3. Submit QC test results for 3-day compressive strength under section 28-2.01D(4).

If concrete pavement will be placed on LCB, construct longitudinal construction and longitudinal contraction joints in the LCB. Provide at least 1 foot horizontal clearance from planned longitudinal construction and longitudinal contraction joints in the concrete pavement.

Do not mix or place LCB when the atmospheric temperature is below 35 degrees F. Do not place LCB on frozen ground.

28-2.03E Finishing

Place LCB under section 40-1.03H(4) or under section 40-1.03H(5) except where there are confined work areas and when authorized:

1. Spread and shape LCB using suitable powered finishing machines and supplement with hand work as necessary
2. Consolidate LCB using high-frequency internal vibrators within 15 minutes after LCB is deposited on the subgrade
3. Vibrate with care such that adequate consolidation occurs across the full paving width and do not use vibrators for extensive weight shifting of the LCB

For LCB to be paved with HMA, before curing operation texture the LCB finished surface by dragging a broom, burlap, or a spring steel tine device. If using a spring steel tine device, the device must produce a scored surface with scores parallel or transverse to the pavement centerline. Texture at a time and in a manner that produces the coarsest texture for the method used.

For LCB to be paved with HMA, the finished surface must not vary more than 0.05 foot from the grade established by the Engineer.

Do not texture LCB that will be covered with concrete pavement. Before applying curing compound, finish LCB to a smooth surface free from mortar ridges and other projections.

For LCB to be paved with concrete pavement, the finished surface must not be above the grade, or more than 0.05 foot below the grade established by the Engineer.

The finished surface must be free from porous areas.

28-2.03F Curing

After finishing LCB, cure LCB with pigmented curing compound under section 90-1.03B(3) and 40-1.03K except for LCB to be paved with concrete pavement, comply with section 36-2. Apply curing compound to the area to be paved with concrete pavement:

1. In 2 separate applications
2. Before the atmospheric temperature falls below 40 degrees F
3. At a rate of 1 gal/150 sq ft for the first application
4. At a rate of 1 gal/200 sq ft for the second application. Within 4 days after the first application, clean the surface and apply the second application.

Immediately repair damage to the curing compound or LCB.

28-2.03G Surfaces Not Within Tolerance

Where LCB will be paved with concrete pavement, remove the base wherever the surface is higher than the grade established by the Engineer and replace it with LCB. Where LCB will not be paved with concrete pavement, remove the base wherever the surface is higher than 0.05 foot above the grade established by the Engineer and replace it with LCB. If authorized, grind the surface with either a diamond or carborundum blade to within tolerance. After grinding LCB to be paved with concrete pavement and after all free water has left the surface, clean foreign material and grinding residue from the surface. Apply curing compound to the ground area at a rate of approximately 1 gal/150 sq ft.

Where the surface of LCB is lower than 0.05 foot from the grade established by the Engineer, remove the base and replace it with LCB or, if authorized, fill low areas according to the pavement material as follows:

1. For HMA pavement, fill low areas with HMA that complies with the specifications for the lowest layer of pavement. Do not fill low areas concurrently with the paving operation.
2. For concrete pavement, fill low areas with pavement concrete concurrent with the paving operation.

28-2.04 PAYMENT

LCB is measured from the dimensions shown.

Replace section 28-3 with:

07-19-13

28-3 RAPID STRENGTH CONCRETE BASE

Reserved

Replace section 28-4 with:

07-19-13

28-4 LEAN CONCRETE BASE RAPID SETTING

Reserved

Replace section 28-5 with:

07-19-13

28-5 CONCRETE BASE

Reserved

Add to section 28:

07-19-13

28-6–28-14 RESERVED

28-15 REPLACE BASE

Reserved

^^^

Replace section 30 with:

04-20-12

30 RECLAIMED PAVEMENTS

04-20-12

30-1 GENERAL

30-1.01 GENERAL

Section 30 includes specifications for reclaiming the pavement section and constructing a base.

30-2 FULL DEPTH RECLAIMED—FOAMED ASPHALT

Reserved

30-3–30-6 RESERVED

^^^

Prepaving conference attendees must sign an attendance sheet provided by the Engineer. The prepaving conference must be attended by your:

1. Project superintendent
2. Paving construction foreman
3. Traffic control foreman

Be prepared to discuss:

1. Quality control
2. Acceptance testing
3. Placement
4. Training on placement methods
5. Checklist of items for proper placement
6. Unique issues specific to the project, including:
   6.1. Weather
   6.2. Alignment and geometrics
   6.3. Traffic control issues
   6.4. Haul distances
   6.5. Presence and absence of shaded areas
   6.6. Any other local issues

37-1.02 MATERIALS

Not Used

37-1.03 CONSTRUCTION

Not Used

37-1.04 PAYMENT

Not Used

Replace section 37-2 with:

07-19-13

37-2 SEAL COATS

37-2.01 GENERAL

37-2.01A General

37-2.01A(1) Summary

Section 37-2 includes specifications for applying seal coats.

37-2.01A(2) Definitions

Reserved

37-2.01A(3) Submittals

Reserved

37-2.01A(4) Quality Control and Assurance

The following personnel must attend the prepaving conference:

1. Aggregate suppliers
2. Chip spreader operators
3. Emulsion and binder distributor
4. Coated chips producer if coated chips are used

37-2.01B Materials

Screenings must be broken stone, crushed gravel, or both. At least 90 percent of screenings by weight must be crushed particles as determined under California Test 205.

Screenings for seal coats must have the properties specified in the following table:

Seal Coat Screenings

Properties	Test method	Specification
Los Angeles Rattler, %, max Loss at 100 revolutions. Loss at 500 revolutions.	California Test 211	 10 40
Film stripping, %, max	California Test 302	25

37-2.01C Construction

37-2.01C(1) General

Wherever final sweeping or brooming of the seal coat surface is complete, place permanent traffic stripes and pavement markings within 10 days.

If you fail to place the permanent traffic stripes and pavement markings within the specified time, the Department withholds 50 percent of the estimated value of the seal coat work completed that has not received permanent traffic stripes and pavement markings.

37-2.01C(2) Equipment

Equipment for seal coats must include and comply with the following:

1. Screenings haul trucks. Haul trucks must have:
   - 1.1. Tailgates that discharge screenings
   - 1.2. Devices to lock onto the rear screenings spreader hitch
   - 1.3. Dump beds that will not push down on the spreader when fully raised
   - 1.4. Dump beds that will not spill screenings on the roadway when transferred to the spreader hopper
   - 1.5. Tarpaulins to cover precoated screenings when haul distance exceeds 30 minutes or ambient temperature is less than 65 degrees F
2. Self-propelled screenings spreader. The spreader must have:
   - 2.1. Screenings hopper in the rear
   - 2.2. Belt conveyors that carry the screenings to the front
   - 2.3. Spreading hopper capable of providing a uniform screening spread rate over the entire width of the traffic lane in 1 application.
3. Self-propelled power brooms. Do not use gutter brooms or steel-tined brooms. Brooms must be capable of removing loose screenings adjacent to barriers that prevent screenings from being swept off the roadway, including curbs, gutters, dikes, berms, and railings.
4. Pneumatic-tired rollers. Pneumatic-tired rollers must be an oscillating type at least 4 feet wide. Each roller must be self-propelled and reversible. Pneumatic tires must be of equal size, diameter, type, and ply. The roller must carry at least 3,000 lb of load on each wheel and each tire must have an air pressure of 100 ± 5 psi.

37-2.01C(3) Surface Preparation

Before applying seal coat, cover manholes, valve and monument covers, grates, or other exposed facilities located within the area of application, using a plastic or oil resistant construction paper secured by tape or adhesive to the facility being covered. Reference the covered facilities with a sufficient number of control points to relocate the facilities after the application of the seal coat.

After completion of the seal coat operation, remove covers from the facilities.

Immediately before applying seal coat, clean the surface to receive seal coat by removing extraneous material and drying. Cleaning the existing pavement includes the use of brooms.

37-2.01C(4) Applying Emulsion and Asphalt Binder

Prevent spray on existing pavement not intended for seal coat or on previously applied seal coat using a material such as building paper. Remove the material after use.

Align longitudinal joints between seal coat applications with designated traffic lanes.

For emulsion, overlap longitudinal joints by not more than 4 inches. You may overlap longitudinal joints up to 8 inches if authorized.

For areas not accessible to a truck distributor bar, apply the emulsion with a squeegee or other authorized means. For asphalt binder, hand spray nonaccessible areas. You may overlap the emulsion or asphalt binder applications before the application of screenings at longitudinal joints.

Do not apply the emulsion or asphalt binder unless there are sufficient screenings at the job site to cover the emulsion or asphalt binder.

Discontinue application of emulsion or asphalt binder early enough to comply with lane closure specifications and darkness. Apply to 1 lane at a time and cover the lane entirely in 1 operation.

37-2.01C(5) Spreading Screenings

Prevent vehicles from driving on asphaltic emulsion or asphalt binder before spreading screenings.

Spread screenings at a uniform rate over the full lane width in 1 application.

Broom excess screenings at joints before spreading adjacent screenings.

Operate the spreader at speeds slow enough to prevent screenings from rolling over after dropping.

If the spreader is not moving, screenings must not drop. If you stop spreading and screenings drop, remove the excess screenings before resuming activities.

37-2.01C(6) Finishing

Remove piles, ridges, or unevenly distributed screenings. Repair permanent ridges, bumps, or depressions in the finished surface. Spread additional screenings and roll if screenings are picked up by rollers or vehicles.

Seal coat joints between adjacent applications of seal coat must be smooth, straight, uniform, and completely covered. Longitudinal joints must be at lane lines and not overlap by more than 4 inches. Blend the adjacent applications by brooming.

A coverage is the number of passes a roller needs to cover the width. A pass is 1 roller movement parallel to the seal coat application in either direction. Overlapping passes are part of the coverage being made and are not part of a subsequent coverage. Do not start a coverage until completing the previous coverage.

Before opening to traffic, finish seal coat in the following sequence:

1. Perform initial rolling consisting of 1 coverage with a pneumatic-tired roller
2. Perform final rolling consisting of 3 coverages with a pneumatic-tired roller
3. Broom excess screenings from the roadway and adjacent abutting areas
4. Apply flush coat if specified

The Engineer may order salvaging of excess screenings.

Dispose of excess screenings the Engineer determines are not salvageable. Dispose of screenings in any of the following ways or locations:

1. Under section 14-10
2. On embankment slopes
3. In authorized areas

Salvaging and stockpiling excess screenings is change order work.

37-2.01C(7) Seal Coat Maintenance

Seals coat surfaces must be maintained for 4 consecutive days from the day screenings are applied. Maintenance must include brooming to maintain a surface free of loose screenings, to distribute screenings over the surface so as to absorb any free asphaltic material, to cover any areas deficient in cover coat material, and to prevent formation of corrugations.

After 4 consecutive days, excess screenings must be removed from the paved areas. Brooming must not displace screenings set in asphaltic material.

The exact time of brooming will be determined by the Engineer. As a minimum, brooming will be required at the following times:

1. On 2-lane 2-way roadways, from 2 to 4 hours after traffic, controlled with pilot cars, has been routed on the seal coat
2. On multilane roadways, from 2 to 4 hours after screenings have been placed
3. In addition to previous brooming, immediately before opening any lane to public traffic, not controlled with pilot cars
4. On the morning following the application of screenings on any lane that has been open to public traffic not controlled with pilot cars and before starting any other activities

For 2-lane 2-way roadways under 1-way traffic control, upon completion of secondary rolling, public traffic must be controlled with pilot cars and routed over the new seal coat for a period of 2 to 4 hours. The Engineer will determine the exact period of time.

Schedule the operations so that seal coat is placed on both lanes of the traveled way each work shift and so that 1-way traffic control is discontinued 1 hour before darkness. At the end of the work shift, the end of the seal coat on both lanes must generally match.

On multilane roadways, initial brooming must begin after the screenings have been in place for a period of 2 to 4 hours. If the initial brooming is not completed during the work shift in which the screenings were placed, the initial brooming must be completed at the beginning of the next work shift.

Public traffic must be controlled with pilot cars and be routed on the new seal coat surface of the lane for a minimum of 2 hours after completion of the initial brooming and before opening the lane to traffic not controlled with pilot cars. When traffic is controlled with pilot cars, a maximum of 1 lane in the direction of travel must be open to public traffic. Once traffic controlled with pilot cars is routed over the seal coat at a particular location, continuous control must be maintained at that location until the seal coat placement and brooming on adjacent lanes to receive seal coat is completed.

37-2.01D Payment

If there is no bid item for a traffic control system, furnishing and using a pilot car is included in the various items of the work involved in applying the seal coat.

If test results for the screenings grading do not comply with specifications, you may remove the seal coat represented by these tests or request that it remain in place with a payment deduction. The deduction is $1.75 per ton for the screenings represented by the test results.

37-2.02 FOG SEAL

37-2.02A General

37-2.02A(1) Summary

Fog seal coat includes applying a slow-setting asphaltic emulsion.

37-2.02A(2) Definitions

Reserved

37-2.02A(3) Submittals

Submit a 1/2-gallon sample of the asphaltic emulsion in a plastic container. Take the sample from the distributor truck spray bar at mid-load.

37-2.02A(4) Quality Control and Assurance

Reserved

37-2.02B Material

The Engineer selects the grade of slow-setting asphaltic emulsion to be used.

If additional water is added to the asphaltic emulsion, the resultant mixture must not be more than 1 part asphaltic emulsion to 1 part water. The Engineer determines the exact amount of additional water.

37-2.02C Construction

Apply asphaltic emulsion for fog seal coat at a residual asphalt rate from 0.02 to 0.06 gal/sq yd. The Engineer determines the exact rate.

Apply fog seal coat when the ambient air temperature is above 40 degrees F.

Sprinkle water on fog seal coat that becomes tacky in an amount determined by the Engineer.

If fog seal coat and seal coat with screenings are specified on the same project, apply fog seal coat at least 4 days before applying the adjoining seal coat with screenings. The joint between the seal coats must be neat and uniform.

37-2.02D Payment

The Department does not adjust the unit price for an increase or decrease in the asphaltic emulsion (fog seal coat) quantity.

37-2.03 FLUSH COATS

37-2.03A General

03-21-14

Flush coat includes applying a fog seal coat to the surface, followed by sand.

07-19-13

37-2.03B Material

The Engineer selects the grade of slow-setting or quick-setting asphaltic emulsion to be used.

Sand for flush coat must comply with the material specifications for fine aggregate grading in section 90-1.02C(3). Sand must not include organic material or clay.

37-2.03C Construction

Apply asphaltic emulsion for flush coat at a residual asphalt rate from 0.02 to 0.06 gal/sq yd. The Engineer determines the exact rate.

During flush coat activities, close adjacent lanes to traffic. Do not track asphaltic emulsion on existing pavement surfaces.

Apply sand immediately after the asphaltic emulsion application.

Spread sand with a self-propelled screenings spreader equipped with a mechanical device that spreads sand at a uniform rate over the full width of a traffic lane in a single application. Spread sand at a rate from 2 to 6 lb/sq yd. The Engineer determines the exact rate.

37-2.03D Payment

The Department does not adjust the unit price for an increase or decrease in the sand cover for the flush coat quantity.

37-2.04 ASPHALTIC EMULSION SEAL COAT

37-2.04A General

37-2.04A(1) General

37-2.04A(1)(a) Summary

Section 37-2.04 includes specifictions for applying asphaltic emulsion seal coat. Asphaltic emulsion seal coat includes applying asphaltic emulsion, followed by screenings, and then a flush coat.

Asphaltic emulsion seal coat includes one or more of the following types:

1. Nonpolymer asphaltic emulsion seal coat
2. Polymer asphaltic emulsion seal coat

A double asphaltic emulsion seal coat is the application of asphaltic emulsion, followed by screenings applied twice in sequence.

37-2.04A(1)(b) Definitions

Reserved

37-2.04A(1)(c) Submittals

At least 10 days before starting asphaltic emulsion seal coat application, submit the name of an authorized laboratory that will be performing asphaltic emulsion QC testing.

03-21-14

Submit a sample of asphaltic emulsion in a 1/2-gallon plastic container to the Engineer and to the authorized laboratory. Each sample must be submitted in an insulated shipping container within 24 hours of sampling.

07-19-13

Within 7 days after taking samples, submit the authorized laboratory's test results for asphaltic emulsion.

37-2.04A(1)(d) Quality Control and Assurance

Samples for the screenings grading and cleanness value must be taken from the spreader conveyor belt.

03-21-14

Within 3 business days of sampling, the authorized laboratory must test the asphaltic emulsion for:

1. Viscosity under AASHTO T 59
2. Sieve test under AASHTO T 59
3. Demulsibility under AASHTO T 59
4. Torsional recovery under California Test 332 for polymer asphaltic emulsion
5. Elastic recovery under AASHTO T 301 for polymer asphaltic emulsion

Circulate asphaltic emulsion in the distributor truck before sampling. Take samples from the distributor truck at mid load or from a sampling tap or thief. Before taking samples, draw and dispose of 1 gallon. In the presence of the Engineer take two 1/2-gallon samples every 55 tons or at least 1 day's production.

07-19-13

37-2.04A(2) Materials

Not Used

37-2.04A(3) Construction

The Engineer determines the exact application rate.

At the time of application, the temperature of the asphaltic emulsion must be from 130 to 180 degrees F.

When tested under California Test 339, the application rate for asphaltic emulsion must not vary from the average by more than:

1. 15 percent in the transverse direction
2. 10 percent in the longitudinal direction

37-2.04A(4) Payment

Not Used

37-2.04B Nonpolymer Asphaltic Emulsion Seal Coat

37-2.04B(1) General

37-2.04B(1)(a) Summary

Section 37-2.04B includes specifications for applying a nonpolymer asphaltic emulsion seal coat.

37-2.04B(1)(b) Definitions

Reserved

37-2.04B(1)(c) Submittals

Reserved

37-2.04B(1)(d) Quality Control and Assurance

For nonpolymer asphaltic emulsion seal coat, if a test result for the screenings cleanness value is from 75 to 80, you may request that the asphaltic emulsion seal coat represented by the test remain in place. A payment deduction is made as specified in section 37-2.04D. If the screenings cleanness value is less than 75, remove the asphaltic emulsion seal coat.

37-2.04B(2) Materials

Screenings for nonpolymer asphaltic emulsion seal coat must have the gradation as determined under California Test 202 in the following table.

Nonpolymer Asphaltic Emulsion Seal Coat Screenings Gradation

	Percentage passing			
Sieve sizes	Coarse 1/2" max	Medium 3/8" max	Medium fine 5/16" max	Fine 1/4" max
3/4"	100	--	--	--
1/2"	95–100	100	--	--
3/8"	50–80	90–100	100	100
No. 4	0–15	5–30	30–60	60–85
No. 8	0–5	0–10	0–15	0–25
No. 16	--	0–5	0–5	0–5
No. 30	--	--	0–3	0–3
No. 200	0–2	0–2	0–2	0–2

03-21-14

The cleanness value determined under California Test 227 must be 80 or greater.

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37-2.04B(3) Construction

Asphaltic emulsion must be applied within the application rate ranges shown in the following table:

Asphaltic Emulsion Application Rates

Screenings	Application rate range(gallons per square yard)
Fine	0.15–0.30
Medium fine	0.25–0.35
Medium	0.25–0.40
Coarse	0.30–0.40

Apply asphaltic emulsion when the ambient air temperature is from 65 to 110 degrees F and the pavement surface temperature is at least 80 degrees F.

Do not apply asphaltic emulsion when weather forecasts predict the ambient air temperature will fall below 39 degrees F within 24 hours after application.

For double asphaltic emulsion seal coat, the asphaltic emulsion must be applied within the application rates shown in the following table:

Asphaltic Emulsion Application Rates

Screenings	Application rate range (gal/sq yd)
Double	
1st application	0.20–0.35
2nd application	0.20–0.30

You may stockpile screenings for asphaltic emulsion seal coat if you prevent contamination. Screenings must have damp surfaces at spreading. If water visibly separates from the screenings, do not spread. You may redampen them in the delivery vehicle.

Spread screenings before the asphaltic emulsion sets or breaks.

Spread screenings within 10 percent of the rate determined by the Engineer. Screenings must have a spread rate within the ranges shown in the following table:

Screening Spread Rates

Seal coat type	Range (lb/sq yd)
Fine	12–20
Medium fine	16–25
Medium	20–30
Coarse	23–30

Do not spread screenings more than 2,500 feet ahead of the completed initial rolling.

For double asphaltic emulsion seal coat, screenings must have a spread rate within the ranges shown in the following table:

Screening Spread Rates

Seal coat type	Range (lb/sq yd)
Double	
1st application	23–30
2nd application	12–20

Remove excess screenings on the 1st application before the 2nd application of asphaltic emulsion.

37-2.04B(4) Payment

If asphaltic emulsion seal coat with screenings does not comply with the cleanness value specifications, you may request that the seal coat remain in place with a pay deduction corresponding to the cleanness value shown in the following table:

Asphaltic Emulsion Seal Coat Cleanness Value Deductions

Cleanness value	Deduction
80 or over	None
79	$2.00 /ton
77–78	$4.00 /ton
75–76	$6.00 /ton

37-2.04C Polymer Asphaltic Emulsion Seal Coat

37-2.04C(1) General

37-2.04C(1)(a) Summary

Section 37-2.04C includes specifications for applying a polymer asphaltic emulsion seal coat.

37-2.04C(1)(b) Definitions

Reserved

37-2.04C(1)(c) Submittals

At least 10 days before starting polymer asphaltic emulsion seal coat application, submit a signed copy of the test result report of the Vialit test method for aggregate retention in chip seals (french chip) to the Engineer and to:

DEPARTMENT OF TRANSPORTATION
Division of Maintenance, Roadway Maintenance Office
1120 N Street, MS 31
Sacramento, CA 95814

37-2.04C(1)(d) Quality Control and Assurance

The authorized laboratory must test screenings for retention under the Vialit test method for aggregate in chip seals (french chip). The Vialit test results are not used for acceptance. The Vialit test is available at the METS Web site.

If the test results for polymer asphaltic emulsion do not comply with the specifications, the Engineer assesses a pay factor value for the following properties and increments:

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Polymer Asphaltic Emulsion Pay Factor Table

Test method and property	Increment	Pay factor
Test on polymer asphaltic emulsion		
AASHTO T 59 (Viscosity, sec Saybolt Furol, at 50 °C)	Each 10 seconds above max or below min	1
AASHTO T 59 (settlement, 5 days, percent)	Each 1.5 percent above max	1
AASHTO T 59 (sieve test, percent max)	Each 0.2 percent above max	1
AASHTO T 59 (demulsibility percent)	Each 2 percent below min	1
Test on residue from evaporation test		
AASHTO T 49 (penetration, 25 °C)	Each 2 dm above max or below min	1
ASTM D 36 (field softening point °C)	2 °C below min	1
California Test 332 (torsional recovery [a])	For each 1 increment below the min value of 18	1
	For each 2 increments below the min value of 18	3
	For each 3 or more increments below the min value of 18	10
ASTM T 301 (elastic recovery [a])	For each 1 increment below the min value of 60	1
	For each 2 increment below the min value of 60	3
	For each 3 increment below the min value of 60	10

[a] The highest pay factor applies

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The Engineer assesses a pay factor of 1 for sampling not performed in compliance with the specifications, including shipping and sampling containers.

For polymer asphaltic emulsion seal coat, if a test result for the screenings cleanness value is from 75 to 86, you may request that the asphaltic emulsion seal coat represented by the test remain in place. A payment deduction is made as specified in section 37-2.04D. If the screenings cleanness value is less than 75, remove the asphaltic emulsion seal coat.

37-2.04C(2) Materials

Polymer asphaltic emulsion must include elastomeric polymer.

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Polymer asphaltic emulsion must comply with section 94, Table 3, under the test on residue from evaporation test for Grades PMRS2, PMRS2h, PMCRS2, and PMCRS2h and the following:

1. The penetration at 39.2 degrees F (200g for 60 seconds) determined under AASHTO T 49 must be at least 6.
2. Elastic recovery determined under AASHTO T 301 must be at least 60 percent.

3. Polymer content in percent by weight does not apply.
4. The ring and ball softening point temperature determined under AASHTO T 53 for Test on Residue from Evaporation Test must comply with the following minimum temperature requirement:
   4.1. 126 degrees F for a geographical ambient temperature from 32 to 104 degrees F
   4.2. 129 degrees F for a geographical ambient temperature from 18 to 104 degrees F
   4.3. 135 degrees F for a geographical ambient temperature from 18 to greater than 104 degrees F

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Screenings for polymer asphaltic emulsion seal coat must have the gradation as determined under California Test 202 in the following table:

Polymer Asphaltic Emulsion Seal Coat Screenings Gradation

	Percentage passing			
Sieve sizes	Coarse 1/2" max	Medium 3/8" max	Medium fine 5/16" max	Fine 1/4" max
3/4"	100	--	--	--
1/2"	85–100	100	--	--
3/8"	0–30	85–100	100	100
No. 4	0–5	0–15	0–50	60–85
No. 8	--	0–5	0–15	0–25
No. 16	--	--	0–5	0–5
No. 30	--	--	0–3	0–3
No. 200	0–2	0–2	0–2	0–2

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The cleanness value determined under California Test 227 must be 86 or greater.

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37-2.04C(3) Construction

Polymer asphaltic emulsion must be applied within the application rate ranges shown in the following table:

Polymer Asphaltic Emulsion Application Rates

Screenings	Application rate range(gallons per square yard)
Fine	0.15–0.30
Medium fine	0.25–0.35
Medium	0.25–0.40
Coarse	0.30–0.40

Apply polymer asphaltic emulsion when the ambient air temperature is from 60 to 105 degrees F and the pavement surface temperature is at least 55 degrees F.

Do not apply polymer asphaltic emulsion when weather forecasts predict the ambient air temperature will fall below 39 degrees F within 24 hours after application.

For double asphaltic emulsion seal coat, polymer asphaltic emulsion must be applied within the application rates shown in the following table:

Polymer Asphaltic Emulsion Application Rates

Screenings	Application rate range (gal/sq yd)
Double	
1st application	0.20–0.35
2nd application	0.20–0.30

You may stockpile screenings for polymer emulsion seal coat if you prevent contamination. Screenings must have damp surfaces at spreading. If water visibly separates from the screenings, do not spread. You may redampen them in the delivery vehicle.

Spread screenings before the polymer emulsion sets or breaks.

Spread screenings within 10 percent of the rate determined by the Engineer. Screenings must have a spread rate within the ranges shown in the following table:

Screening Spread Rates

Seal coat type	Range (lb/sq yd)
Fine	12–20
Medium fine	16–25
Medium	20–30
Coarse	23–30

Do not spread screenings more than 2,500 feet ahead of the completed initial rolling.

For double seal coat, screenings must have a spread rate within the ranges shown in the following table:

Screening Spread Rates

Seal coat type	Range (lb/sq yd)
Double	
1st application	23–30
2nd application	12–20

Remove excess screenings on the 1st application before the 2nd application of asphaltic emulsion.

37-2.04C(4) Payment

If polymer asphaltic emulsion seal coat with screenings does not comply with the specifications for cleanness value you may request that the seal coat remain in place with a pay deduction corresponding by the cleanness value shown in the following table:

Polymer Asphaltic Emulsion Seal Coat Cleanness Value Deductions

Cleanness value	Deduction
86 or over	None
81–85	$2.20/ton
77–80	$4.40/ton
75–76	$6.60/ton

If test results for polymer asphaltic emulsion aggregate grading and cleanness value test results do not comply with the specifications, all deductions are made. A test for polymer asphaltic emulsion represents the smaller of 55 tons or 1 day's production. A test for the screenings grading or cleanness value represents the smaller of 300 tons or 1 day's production.

The payment deduction for noncompliant polymer asphaltic emulsion is based on the total pay factor value determined from the table titled, "Polymer Asphaltic Emulsion Pay Factor Deduction." You must remove polymer asphaltic emulsion seal coat with a pay factor value greater than 20. You may request seal coat with noncompliant polymer asphaltic emulsion to remain in place with a pay deduction for the total pay factor value shown in the following table:

Polymer Asphaltic Emulsion Pay Factor Deductions

Total pay factor value	Deduction
0	none
1–2	$5.00/ton
3–5	$10.00/ton
6–9	$15.00/ton
10–14	$25.00/ton
15–20	$50.00/ton

37-2.05 ASPHALT BINDER SEAL COATS

37-2.05A General

Reserved

37-2.05B Asphalt Rubber Binder Seal Coats

37-2.05B(1) General

37-2.05B(1)(a) Summary

Section 37-2.05B includes specifications for applying asphalt rubber binder seal coat. Asphalt rubber seal coat includes applying heated asphalt rubber binder, followed by heated screenings precoated with asphalt binder, followed by a flush coat.

37-2.05B(1)(b) Definitions

crumb rubber modifier: Ground or granulated high natural crumb rubber or scrap tire crumb rubber.

descending viscosity reading: Subsequent viscosity reading at least 5 percent lower than the previous viscosity reading.

high natural crumb rubber: Material containing 40 to 48 percent natural rubber.

scrap tire crumb rubber: Any combination of:

1. Automobile tires
2. Truck tires
3. Tire buffing

37-2.05B(1)(c) Submittals

For each delivery of asphalt rubber binder ingredients and asphalt rubber binder to the job site, submit a certificate of compliance and a copy of the specified test results.

Submit MSDS for each asphalt rubber binder ingredient and the asphalt rubber binder.

At least 15 days before use, submit:

1. Four 1-quart cans of mixed asphalt rubber binder
2. Samples of each asphalt rubber binder ingredient
3. Asphalt rubber binder formulation and data as follows:
   - 3.1. For asphalt binder and asphalt modifier submit:
     - 3.1.1. Source and grade of asphalt binder
     - 3.1.2. Source and type of asphalt modifier
     - 3.1.3. Percentage of asphalt modifier by weight of asphalt binder
     - 3.1.4. Percentage of combined asphalt binder and asphalt modifier by weight of asphalt rubber binder
     - 3.1.5. Test results for the specified quality characteristics
   - 3.2. For crumb rubber modifier submit:
     - 3.2.1. Each source and type of scrap tire crumb rubber and high natural rubber
     - 3.2.2. Percentage of scrap tire crumb rubber and high natural rubber by total weight of asphalt rubber binder
     - 3.2.3. Test results for the specified quality characteristics

3.3. For asphalt rubber binder submit:
   3.3.1. Test results for the specified quality characteristics
   3.3.2. Minimum reaction time and temperature

At least 5 business days before use, submit the permit issued by the local air quality agency for asphalt rubber binder:

1. Field blending equipment
2. Application equipment

If an air quality permit is not required by the local air quality agency for producing asphalt rubber binder or spray applying asphalt rubber binder, submit verification from the local air quality agency that an air quality permit is not required for this Contract.

Submit a certified volume or weight slip for each delivery of asphalt rubber binder ingredients and asphalt rubber binder.

Submit a certificate of compliance and accuracy verification of test results for viscometers.

When determined by the Engineer, submit notification 15 minutes before each viscosity test or submit a schedule of testing times.

Submit the log of asphalt rubber binder viscosity test results each day of asphalt rubber seal coat work.

37-2.05B(1)(d) Quality Control and Assurance

Equipment used in producing asphalt rubber binder must be permitted for use by the local air quality agency. Equipment used in spreading asphalt rubber binder must be permitted for use by the local air quality agency.

Each asphalt rubber binder ingredient must be sampled and tested for compliance with the specifications by the manufacturer.

Test and submit results at least once per project or the following, whichever frequency is greater:

1. For crumb rubber modifier except for grading, at least once per 250 tons. Samples of scrap tire crumb rubber and high natural crumb rubber must be sampled and tested separately. Test each delivery of crumb rubber modifier for grading.
2. For asphalt binder, test and submit at least once per 200 tons of asphalt binder production.
3. For asphalt modifier, test and submit at least once per 25 tons of asphalt modifier production.

Scrap tire crumb rubber and high natural crumb rubber must be delivered to the asphalt rubber production site in separate bags.

Take viscosity readings of asphalt rubber binder under ASTM D7741 during asphalt rubber binder production. Start taking viscosity readings of samples taken from the reaction vessel at least 45 minutes after adding crumb rubber modifier and continue taking viscosity readings every 30 minutes until 2 consecutive descending viscosity readings have been obtained and the final viscosity meets the specification requirement. After meeting the 2 descending viscosity readings requirement, continue to take viscosity readings hourly and within 15 minutes before use. Log the test results, including time of testing and temperature of the asphalt rubber binder.

37-2.05B(2) Material

37-2.05B(2)(a) General

Reserved

37-2.05B(2)(b) Asphalt Binder

Asphalt binder must comply with the specifications for asphalt binder. Do not modify asphalt binder with polymer.

37-2.05B(2)(c) Asphalt Modifier

Asphalt modifier must be a resinous, high flash point, and aromatic hydrocarbon. Asphalt modifier must have the values for the quality characteristics shown in the following table:

Asphalt Modifier for Asphalt Rubber Binder

Quality characteristic	Test method	Value
Viscosity, m^2/s (x 10^{-6}) at 100 °C	ASTM D 445	X ± 3 [a]
Flash point, CL.O.C., °C	ASTM D 92	207 min
Molecular analysis		
Asphaltenes, percent by mass	ASTM D 2007	0.1 max
Aromatics, percent by mass	ASTM D 2007	55 min

[a] "X" denotes the proposed asphalt modifier viscosity from 19 to 36. A change in "X" requires a new asphalt rubber binder submittal.

37-2.05B(2)(d) Crumb Rubber Modifier

Crumb rubber modifier must be ground or granulated at ambient temperature.

Scrap tire crumb rubber and high natural crumb rubber must be delivered to the asphalt rubber binder production site in separate bags.

Steel and fiber must be separated. If steel and fiber are cryogenically separated, it must occur before grinding and granulating. Cryogenically-produced crumb rubber modifier particles must be large enough to be ground or granulated.

Wire must not be more than 0.01 percent by weight of crumb rubber modifier. Crumb rubber modifier must be free of contaminants except fabric, which must not exceed 0.05 percent by weight of crumb rubber modifier. Method for determining the percent weight of wire and fabric is available under Laboratory Procedure 10 at the following METS Web site:

http://www.dot.ca.gov/hq/esc/Translab/ofpm/fpmlab.htm

The length of an individual crumb rubber modifier particle must not exceed 3/16 inch.

Crumb rubber modifier must be dry, free-flowing particles that do not stick together. A maximum of 3 percent calcium carbonate or talc by weight of crumb rubber modifier may be added. Crumb rubber modifier must not cause foaming when combined with the asphalt binder and asphalt modifier.

Specific gravity of crumb rubber modifier must be from 1.1 to 1.2 determined under California Test 208.

When tested under ASTM D 297, crumb rubber modifier must comply with the requirements shown in the following table:

Crumb Rubber Modifier

Quality characteristic	Scrap tire crumb rubber (percent)		High natural rubber (percent)	
	Min	Max	Min	Max
Acetone extract	6.0	16.0	4.0	16.0
Rubber hydrocarbon	42.0	65.0	50.0	--
Natural rubber content	22.0	39.0	40.0	48.0
Carbon black content	28.0	38.0	--	--
Ash content	--	8.0	--	--

Scrap tire crumb rubber must have the gradation shown in the following table:

Scrap Tire Crumb Rubber Gradation
Percentage passing

Sieve size	Gradation limit	Operating range	Contract compliance
No. 8	100	100	100
No. 10	98–100	95–100	90–100
No. 16	45–75	35–85	32–88
No. 30	2–20	2–25	1–30
No. 50	0–6	0–10	0–15
No. 100	0–2	0–5	0–10
No. 200	0	0–2	0–5

High natural crumb rubber must have the gradation shown in the following table:

High Natural Crumb Rubber Gradation
Percentage passing

Sieve size	Gradation limit	Operating range	Contract compliance
No. 10	100	100	100
No. 16	95–100	92–100	85–100
No. 30	35–85	25–95	20–98
No. 50	10–30	6–35	2–40
No. 100	0–4	0–7	0–10
No. 200	0–1	0–3	0–5

Test the crumb rubber modifier gradation under ASTM C 136 except

1. Split or quarter 100 ± 5 g from the crumb rubber modifier sample and dry to a constant mass at a temperature from 57 to 63 degrees C and record the dry sample mass. Place the crumb rubber modifier sample and 5 g of talc in a 1/2-liter jar. Seal the jar, then shake the jar by hand for at least 1 minute to mix the crumb rubber modifier and the talc. Continue shaking or open the jar and stir until the particle agglomerates and clumps are broken and the talc is uniformly mixed.
2. Place 1 rubber ball on each sieve. Each ball must weigh 8.5 ± 0.5 g, measure 24.5 ± 0.5 mm in diameter, and have a Shore Durometer "A" hardness of 50 ± 5 determined under ASTM D 2240. After sieving the combined material for 10 ± 1 minutes, disassemble the sieves. Brush material adhering to the bottom of a sieve into the next finer sieve. Weigh and record the mass of the material retained on the 2.36-milimeter sieve and leave this material (do not discard) on the scale or balance. Fabric balls must remain on the scale or balance and be placed together on the side to prevent them from being covered or disturbed when the material from finer sieves is placed onto the scale or balance. The material retained on the 2.00-milimeter sieve must be added to the scale or balance. Weigh and record that mass as the accumulative mass retained on the 2.00-milimeter sieve. Continue weighing and recording the accumulated masses retained on the remaining sieves until the accumulated mass retained in the pan has been determined. Before discarding the crumb rubber modifier sample, separately weigh and record the total mass of fabric balls in the sample.
3. Determine the mass of material passing the 75-micrometer sieve by subtracting the accumulated mass retained on the 75-micrometer sieve from the accumulated mass retained in the pan. If the material passing the 75-micrometer sieve has a mass of 5 g or less, cross out the recorded number for the accumulated mass retained in the pan and copy the number recorded for the accumulated mass retained on the 75-micrometer sieve and record that number, next to the crossed out number, as the accumulated mass retained in the pan. If the material passing the 75-micrometer sieve has a mass greater than 5 g, cross out the recorded number for the accumulated mass retained in the pan, subtract 5 g from that number and record the difference next to the crossed out number. The adjustment to the accumulated mass retained in the pan accounts for the 5 g of talc added to the sample. For calculation purposes, the adjusted total sample mass is the same as the adjusted

accumulated mass retained in the pan. Determine the percent passing based on the adjusted total sample mass and record to the nearest 0.1 percent.

37-2.05B(2)(e) Asphalt Rubber Binder

Asphalt rubber binder must be a combination of:

1. Asphalt binder
2. Asphalt modifier
3. Crumb rubber modifier

Asphalt rubber binder blending equipment must be authorized under the Department's material plant quality program.

The blending equipment must allow the determination of weight percentages of each asphalt rubber binder ingredient.

Asphalt rubber binder must be 79 ± 1 percent by weight asphalt binder and 21 ± 1 percent by weight of crumb rubber modifier. The minimum percentage of crumb rubber modifier must be 20.0 percent and lower values may not be rounded up.

Crumb rubber modifier must be 76 ± 2 percent by weight scrap tire crumb rubber and 24 ± 2 percent by weight high natural rubber.

Asphalt modifier and asphalt binder must be blended at the production site. Asphalt modifier must be from 2.5 to 6.0 percent by weight of the asphalt binder in the asphalt rubber binder. The asphalt rubber binder supplier determines the exact percentage.

If blended, the asphalt binder must be from 375 to 440 degrees F when asphalt modifier is added and the mixture must circulate for at least 20 minutes. Asphalt binder, asphalt modifier, and crumb rubber modifier may be proportioned and combined simultaneously.

The blend of asphalt binder and asphalt modifier must be combined with crumb rubber modifier at the asphalt rubber binder production site. The asphalt binder and asphalt modifier blend must be from 375 to 440 degrees F when crumb rubber modifier is added. Combined ingredients must be allowed to react at least 45 minutes at temperatures from 375 to 425 degrees F except the temperature must be at least 10 degrees F below the flash point of the asphalt rubber binder.

After reacting, the asphalt rubber binder must have the values for the quality characteristics shown in the following table:

Asphalt Rubber Binder

Quality characteristic	Test method	Requirement	
		Min	Max
Cone penetration @ 25 °C, 1/10 mm	ASTM D 217	25	60
Resilience @ 25 °C, percent rebound	ASTM D 5329	18	50
Field softening point, °C	ASTM D 36	55	88
Viscosity @190 °C, Pa • s ($x10^{-3}$)	ASTM D 7741	1500	2500

Maintain asphalt rubber binder at a temperature from 375 to 415 degrees F.

Stop heating unused asphalt rubber binder 4 hours after the 45-minute reaction period. Reheating asphalt rubber binder that cools below 375 degrees F is a reheat cycle. Do not exceed 2 reheat cycles. If reheating, asphalt rubber binder must be from 375 to 415 degrees F before use.

During reheating, you may add scrap tire crumb rubber. Scrap tire crumb rubber must not exceed 10 percent by weight of the asphalt rubber binder. Allow added scrap tire crumb rubber to react for at least 45 minutes. Reheated asphalt rubber binder must comply with the specifications for asphalt rubber binder.

37-2.05B(2)(f) Screenings

Before precoating with asphalt binder and when tested under California Test 202, screenings for asphalt rubber seal coat must have the gradation shown in the following table:

Asphalt Rubber Seal Coat Screenings Gradation

Sieve sizes	Percentage passing by weight		
	Coarse 1/2" max	Medium 1/2" max	Fine 3/8" max
3/4"	100	100	100
1/2"	75–90	85–90	95–100
3/8"	0–20	0–30	70–85
No. 4	0–2	0–5	0–15
No. 8	--	--	0–5
No. 200	0–1	0–1	0–1

Screenings must have the values for the properties shown in the following table:

Seal Coat Screenings

Properties	Test method	Value
Cleanness value, min	California Test 227	80
Durability, min	California Test 229	52

37-2.05B(3) Construction

37-2.05B(3)(a) General

Reserved

37-2.05B(3)(b) Equipment

Self-propelled distributor truck for applying asphalt rubber binder must have the following features:

1. Heating unit
2. Internal mixing unit
3. Pumps that spray asphalt rubber binder within 0.05 gal/sq yd of the specified rate
4. Fully circulating spray bar that applies asphalt rubber binder uniformly
5. Tachometer
6. Pressure gages
7. Volume measuring devices
8. Thermometer
9. Observation platform on the rear of the truck for an observer on the platform to see the nozzles and unplug them if needed

37-2.05B(3)(c) Precoating Screenings

For asphalt rubber seal coat, do not recombine fine materials collected in dust control systems except cyclone collectors or knock-out boxes with any other aggregate used in the production of screenings.

For asphalt rubber seal coat, screenings must be preheated from 260 to 325 degrees F. Coat with any of the asphalts specified in the table titled "Performance Graded Asphalt Binder" in section 92. Coat at a central mixing plant. The asphalt must be from 0.5 to 1.0 percent by weight of dry screenings. The Engineer determines the exact rate.

Plant must be authorized under the Department's material plant quality program.

Do not stockpile preheated or precoated screenings.

37-2.05B(3)(d) Asphalt Rubber Binder Application

Apply asphalt rubber binder immediately after the reaction period. At the time of application, the temperature of asphalt rubber binder must be from 385 to 415 degrees F.

Apply asphalt rubber binder at a rate from 0.55 to 0.65 gal/sq yd. The Engineer determines the exact rate.

Apply asphalt rubber binder when the atmospheric temperature is from 60 to 105 degrees F and the pavement surface temperature is at least 55 degrees F.

Do not apply asphalt rubber binder unless there are sufficient screenings available to cover the asphalt rubber binder within 2 minutes. Intersections, turn lanes, gore points, and irregular areas must be covered within 15 minutes.

Do not apply asphalt rubber binder when weather or road conditions are unsuitable, including high wind or when the pavement is damp. In windy conditions you may adjust the distributor bar height and distribution speed, and use shielding equipment, if the Engineer authorizes your request.

37-2.05B(3)(e) Screenings Application

During transit, cover precoated screenings for asphalt rubber seal coat with tarpaulins if the ambient air temperature is below 65 degrees F or the haul time exceeds 30 minutes.

At the time of application, screenings for asphalt rubber seal coat must be from 225 to 325 degrees F.

Spread screenings at a rate from 28 to 40 lb/sq yd. The exact rate is determined by the Engineer. Spread to within 10 percent of the determined rate.

37-2.05B(3)(f) Rolling and Sweeping

Perform initial rolling within 90 seconds of spreading screenings. Do not spread screenings more than 200 feet ahead of the initial rolling.

For final rolling, you may request use of a steel-wheeled roller weighing from 8 to 10 tons, static mode only.

Perform a final sweeping before Contract acceptance. The final sweeping must not dislodge screenings.

Dispose of swept screenings at least 150 feet from any waterway.

37-2.05B(4) Payment

Screenings for asphalt rubber seal coat are measured by coated weight after they are preheated and precoated with asphalt binder. The weight of screenings must be the coated weight.

If recorded batch weights are printed automatically, the bid item for screenings for asphalt-rubber seal coat are measured using the printed batch weights, provided:

1. Total aggregate weight for screenings per batch is printed
2. Total asphalt binder weight per batch is printed
3. Each truckload's zero tolerance weight is printed before weighing the first batch and after weighing the last batch
4. Time, date, mix number, load number and truck identification are correlated with a load slip
5. A copy of the recorded batch weights is certified by a licensed weighmaster and submitted to the Engineer

Screenings for asphalt rubber seal coat is paid for as precoated screenings.

Asphalt-rubber binder is measured under the specifications for asphalts.

If test results for gradation tests do not comply with the specifications, deductions are taken.

Each gradation test for scrap tire crumb rubber represents 10,000 lbs or the amount used in that day's production, whichever is less.

Each gradation test for high natural rubber represents 3,400 lbs or the amount used in that day's production, whichever is less.

For each gradation test, the following pay deductions will be taken from the asphalt rubber bid item:

Gradation Test

Material	Test result [a]	Deduction
Scrap tire crumb rubber	Operating range < TR < Contract compliance	$250
Scrap tire crumb rubber	TR > Contract compliance	$1,100
High natural crumb rubber	Operating range < TR < Contract compliance	$250
High natural crumb rubber	TR > Contract compliance	$600

[a] Test Result = TR

37-2.05C Modified Asphalt Binder Seal Coat

Reserved

03-21-14

37-2.06 STRESS ABSORBING MEMBRANE INTERLAYER

37-2.06A General

Section 37-2.06 applies where a stress absorbing membrane interlayer (SAMI) is shown.

Comply with section 37-2.05B except a flush coat is not required.

37-2.06B Materials

For SAMI, screenings must comply with the 3/8-inch maximum gradation.

37-2.06C Construction

For SAMI, section 37-2.01C(7) does not apply.

Final rolling and sweeping are not required for SAMI.

37-2.06D Payment

Not Used

37-2.07–37-2.10 RESERVED

Add to section 37-3.01D(1):

01-18-13

Micro-surfacing spreader operators must attend the prepaving conference.

^^

39 HOT MIX ASPHALT

04-18-14

Replace the headings and paragraphs in section 39 with:

04-18-14

39-1 GENERAL

39-1.01 GENERAL

39-1.01A Summary

Section 39-1 includes general specifications for producing and placing hot mix asphalt.

HMA includes one or more of the following types:

1. Type A HMA
2. RHMA-G

3. OGFC
4. BWC
5. Minor HMA

If a warm mix asphalt technology is specified, the warm mix asphalt technology to be used must be authorized. For Department-authorized warm mix asphalt technologies, go to the METS website.

39-1.01B Definitions

binder replacement: Binder from RAP expressed as a percent of the total binder in the mix.

coarse aggregate: Aggregate retained on a no. 4 sieve.

fine aggregate: Aggregate passing the no. 4 sieve.

leveling course: Thin layer of HMA used to correct minor variations in the longitudinal and transverse profile of the pavement before placement of other pavement layers.

lower course: Layer of HMA below 0.2 feet from finished grade exclusive of OGFC.

miscellaneous areas: Areas outside the traveled way such as:

1. Median areas not including inside shoulders
2. Island areas
3. Sidewalks
4. Gutters
5. Ditches
6. Overside drains
7. Aprons at ends of drainage structures

processed RAP: RAP that has been fractionated.

substitution rate: Percent of RAP by dry weight of aggregate substituted for virgin aggregate.

supplemental fine aggregate: Aggregate passing the no. 30 sieve, including hydrated lime, portland cement, and fines from dust collectors.

surface course: Upper 0.2 feet of HMA exclusive of OGFC.

top layer: Final riding surface.

39-1.01C Submittals

39-1.01C(1) General

Reserved

39-1.01C(2) Job Mix Formula

39-1.01C(2)(a) General

Except for the HMA to be used in miscellaneous areas and dikes, submit your proposed JMF for each type of HMA to be used. The JMF must be submitted on the Contractor Job Mix Formula Proposal form along with:

1. Mix design documentation on Contractor Hot Mix Asphalt Design Data form dated within 12 months of submittal
2. JMF verification on a Caltrans Hot Mix Asphalt Verification form, if applicable
3. JMF renewal on a Caltrans Job Mix Formula Renewal form, if applicable
4. MSDS for:
   4.1. Asphalt binder
   4.2. Supplemental fine aggregate except fines from dust collectors
   4.3. Antistrip additives

The Contractor Hot Mix Asphalt Design Data form must show documentation on aggregate quality.

If you cannot submit a Department-verified JMF on a Caltrans Hot Mix Asphalt Verification form dated within 12 months before HMA production, the Engineer verifies the JMF.

Submit a new JMF if you change any of the following:

1. Target asphalt binder percentage greater than ±0.2 percent
2. Asphalt binder supplier
3. Combined aggregate gradation
4. Aggregate sources
5. Liquid antistrip producer or dosage
6. Average binder content in a new fractionated RAP stockpile by more than ±2.0 percent from the average RAP binder content reported on page 4 of your Contractor Hot Mix Asphalt Design Data form
7. Average maximum specific gravity in a new fractionated RAP stockpile by more than ±0.060 from the average maximum specific gravity value reported on page 4 of your Contractor Hot Mix Asphalt Design Data form
8. Any material in the JMF

Allow the Engineer 5 business days from a complete JMF submittal for document review of the aggregate qualities, mix design, and JMF. The Engineer notifies you if the proposed JMF submittal is accepted.

If your JMF fails verification testing, submit an adjusted JMF based on your testing. An adjusted JMF requires a new Contractor Job Mix Formula Proposal form and Contractor Hot Mix Asphalt Design Data form and verification of a plant-produced sample.

You may submit an adjusted aggregate gradation TV on a Contractor Job Mix Formula Proposal form before verification testing. Aggregate gradation TV must be within the TV limits specified.

39-1.01C(2)(b) Job Mix Formula Renewal

You may request a JMF renewal by submitting:

1. Proposed JMF on a Contractor Job Mix Formula Proposal form
2. Previously verified JMF documented on a Caltrans Hot Mix Asphalt Verification form dated within 12 months
3. Mix design documentation on a Contractor Hot Mix Asphalt Design Data form used for the previously verified JMF

39-1.01C(2)(c) Job Mix Formula Modification

For an authorized JMF, submit a modified JMF if you change any of the following:

1. Asphalt binder supplier
2. Liquid antistrip producer
3. Liquid antistrip dosage

You may change any of the above items only once during the Contract.

Submit your modified JMF request a minimum of 15 days before production. Each modified JMF submittal must consist of:

1. Proposed modified JMF on Contractor Job Mix Formula Proposal form, marked *Modified*.
2. Mix design records on Contractor Hot Mix Asphalt Design Data form for the authorized JMF to be modified.
3. JMF verification on Hot Mix Asphalt Verification form for the authorized JMF to be modified.
4. Test results for the modified JMF in compliance with the mix design specifications. Perform tests at the mix design OBC as shown on the Contractor Asphalt Mix Design Data form.

With an accepted modified JMF submittal, the Engineer verifies each modified JMF within 10 days of receiving all verification samples.

39-1.01C(3) Quality Control Plan

With your proposed JMF submittal, submit a QC plan for HMA.

The QC plan must describe the organization and procedures for:

1. Controlling HMA quality characteristics
2. Taking samples, including sampling locations
3. Establishing, implementing, and maintaining QC
4. Determining when corrective actions are needed
5. Implementing corrective actions
6. Methods and materials for backfilling core locations

The QC plan must address the elements affecting HMA quality including:

1. Aggregate
2. Asphalt binder
3. Additives
4. Production
5. Paving

The QC plan must include aggregate QC sampling and testing during lime treatment.

The Engineer reviews the QC plan within 5 business days from the submittal. Do not start HMA production until the Engineer authorizes the plan.

If QC procedures, personnel, tester qualifications, sample testing locations, or lab accreditation status change, submit a QC plan supplement at least 3 business days before implementing the proposed change. Do not implement the change without authorization.

39-1.01C(4) Test Results

For mix design, JMF verification, production start-up, and each 10,000 tons, submit AASHTO T 283 and AASHTO T 324 (Modified) test results to the Engineer and electronically to:

Moisture_Tests@dot.ca.gov

Submit all QC test results, except AASHTO T 283 and AASHTO T 324 (Modified), within 3 business days of a request. Submit AASHTO T 283 QC tests within 15 days of sampling.

For tests performed under AASHTO T 324 (Modified), submit test data and 1 tested sample set within 5 business days of sampling.

If coarse and fine durability index tests are required, submit test results within 2 business days of testing.

If tapered notched wedge is used, submit test result values within 24 hours of testing.

39-1.01C(5) Reserved

39-1.01C(6) Liquid Antistrip Treatment

If liquid antistrip treatment is used, submit the following with your proposed JMF submittal:

1. One 1-pint sample
2. Infrared analysis including copy of absorption spectra
3. Certified copy of test results
4. Certificate of compliance for each liquid antistrip shipment. On each certificate of compliance, include:
   - 4.1. Your signature and printed name
   - 4.2. Shipment number
   - 4.3. Material type
   - 4.4. Material specific gravity
   - 4.5. Refinery
   - 4.6. Consignee
   - 4.7. Destination
   - 4.8. Quantity
   - 4.9. Contact or purchase order number
   - 4.10. Shipment date
6. Proposed proportions for liquid antistrip

For each delivery of liquid antistrip to the HMA production plant, submit a 1-pint sample to METS. Submit shipping documents. Label each liquid antistrip sampling container with:

1. Liquid antistrip type
2. Application rate
3. Sample date
4. Contract number

At the end of each day's production shift, submit production data in electronic and printed media. Present data on electronic media in tab delimited format. Use line feed carriage return with 1 separate record per line for each production data set. Allow sufficient fields for the specified data. Include data titles at least once per report. For each HMA mixing plant type, submit the following information in the order specified:

1. For batch plant mixing:
   - 1.1. Production date
   - 1.2. Time of batch completion
   - 1.3. Mix size and type
   - 1.4. Each ingredient's weight
   - 1.5. Asphalt binder content as a percentage of the total weight of mix
   - 1.6. Liquid antistrip content as a percentage of the asphalt binder weight
2. For continuous mixing plant:
   - 2.1. Production date
   - 2.2. Data capture time
   - 2.3. Mix size and type
   - 2.4. Flow rate of wet aggregate collected directly from the aggregate weigh belt
   - 2.5. Aggregate moisture content as percentage of the dry aggregate weight
   - 2.6. Flow rate of asphalt binder collected from the asphalt binder meter
   - 2.7. Flow rate of liquid antistrip collected from the liquid antistrip meter
   - 2.8. Asphalt binder content as percentage of the total weight of mix calculated from:
     - 2.8.1. Aggregate weigh belt output
     - 2.8.2. Aggregate moisture input
     - 2.8.3. Asphalt binder meter output
   - 2.9. Liquid antistrip content as percentage of the asphalt binder weight calculated from:
     - 2.9.1. Asphalt binder meter output
     - 2.9.2. Liquid antistrip meter output

39-1.01C(7) Lime Treatment

If aggregate lime treatment is used, submit the following with your proposed JMF submittal and each time you produce lime-treated aggregate:

1. Exact lime proportions for fine and coarse virgin aggregate
2. If marination is required, the averaged aggregate quality test results within 24 hours of sampling
3. For dry lime aggregate treatment, a treatment data log from the dry lime and aggregate proportioning device in the following order:
   - 3.1. Treatment date
   - 3.2. Time of day the data is captured
   - 3.3 Aggregate size being treated
   - 3.4. HMA type and mix aggregate size
   - 3.5. Wet aggregate flow rate collected directly from the aggregate weigh belt
   - 3.6. Aggregate moisture content, expressed as a percent of the dry aggregate weight
   - 3.7. Flow rate of dry aggregate calculated from the flow rate of wet aggregate
   - 3.8. Dry lime flow rate
   - 3.9. Lime ratio from the authorized JMF for each aggregate size being treated
   - 3.10. Lime ratio from the authorized JMF for the combined aggregate
   - 3.11. Actual lime ratio calculated from the aggregate weigh belt output, the aggregate moisture input, and the dry lime meter output, expressed as a percent of the dry aggregate weight
   - 3.12. Calculated difference between the authorized lime ratio and the actual lime ratio
4. For lime slurry aggregate treatment, a treatment data log from the slurry proportioning device in the following order:
   - 4.1. Treatment date

4.2. Time of day the data is captured
4.3. Aggregate size being treated
4.4. Wet aggregate flow rate collected directly from the aggregate weigh belt
4.5. Moisture content of the aggregate just before treatment, expressed as a percent of the dry aggregate weight
4.6. Dry aggregate flow rate calculated from the wet aggregate flow rate
4.7. Lime slurry flow rate measured by the slurry meter
4.8. Dry lime flow rate calculated from the slurry meter output
4.9. Authorized lime ratio for each aggregate size being treated
4.10. Actual lime ratio calculated from the aggregate weigh belt and the slurry meter output, expressed as a percent of the dry aggregate weight
4.11. Calculated difference between the authorized lime ratio and the actual lime ratio
4.12. Dry lime and water proportions at the slurry treatment time

Each day during lime treatment, submit the treatment data log on electronic media in tab delimited format on a removable CD-ROM storage disk. Each continuous treatment data set must be a separate record using a line feed carriage return to present the specified data on 1 line. The reported data must include data titles at least once per report.

39-1.01C(8) Warm Mix Asphalt Technology

If a warm mix asphalt technology is used, submit the following with your proposed JMF submittal:

1. MSDS for warm mix asphalt technology
2. For warm mix asphalt water injection foam technology:
   2.1. Name of technology
   2.2. California Test 386 test result for foamed bitumen expansion ratio dated within 12 months of submittal
   2.3. California Test 386 test result for foamed bitumen half-life dated within 12 months of submittal
   2.4. Optimum foaming water content
   2.5. Proposed HMA production temperature range
3. For warm mix asphalt additive technology:
   3.1. Name of technology
   3.2. Percent admixture by weight of binder and percent admixture by total weight of HMA as recommended by the manufacturer
   3.3. Methodology for inclusion of admixture in laboratory-produced HMA
   3.4. Proposed HMA production temperature range

Collect and hold data for the duration of the contract and submit the electronic media, daily and upon request. The snapshot of production data must include the following:

1. Date of production
2. Production location
3. Time of day the data is captured
4. HMA mix type being produced and target binder rate
5. HMA additive type, brand, and target rate
6. Temperature of the binder and HMA mixture
7. For a continuous mixing plant, the rate of flow of the dry aggregate calculated from the wet aggregate flow rate as determined by the conveyor scale
8. For a continuous mixing plant, the rate of flow of the asphalt meter
9. For a continuous mixing plant, the rate of flow of HMA additive meter
10. For batch plant mixing, actual batch weights of all ingredients
11. Dry aggregate to binder ratio calculated from metered ingredient output
12. Dry aggregate to HMA additive ratio calculated from metered output

At the end of each day's production shift, submit electronic and printed media from the HMA plant process controller. Present data on electronic media in comma-separated values or tab-separated values format. The captured data for the ingredients represented by production snapshot must have allowances for sufficient fields to satisfy the amount of data required by these specifications and include data titles at least once per report.

39-1.01C(9) Samples

For the samples taken for JMF verification, submit 3 parts to the Engineer and use 1 part for your testing.

At production start-up and within 1000 tons of the halfway point of production of HMA, submit samples split from your HMA production sample for AASHTO T 283 and AASHTO T 324 (Modified) tests to the Engineer.

For production samples taken, submit 3 parts to the Engineer and use 1 part for your testing.

39-1.01C(10)–39-1.01C(11) Reserved

39-1.01C(12) Data Cores

Section 39-1.01C(12) applies if a bid item for data core is shown on the Bid Item List.

Submit a summary of data cores taken and a photograph of each data core to the Engineer and to:

Coring@dot.ca.gov

For each data core, the summary must include:

1. Project identification number
2. Date cored
3. Core identification number
4. Type of materials recovered
5. Type and approximate thickness of unstabilized material not recovered
6. Total core thickness
7. Thickness of each individual material to within:
   - 7.1. For recovered material, 1/2 inch
   - 7.2. For unstabilized material, 1.0 inch
8. Location including:
   - 8.1. County
   - 8.2. Route
   - 8.3. Post mile
   - 8.4. Lane number
   - 8.5. Lane direction
   - 8.6. Station

Each data core digital photograph must include a ruler laid next to the data core. Each photograph must include:

1. Core
2. Project identification number
3. Core identification number
4. Date cored
5. County
6. Route
7. Post mile
8. Lane number
9. Lane direction

39-1.01C(13) Pavement Smoothness

39-1.01C(13)(a) General

Reserved

39-1.01C(13)(b) Straightedge Measurements

Within 2 business days of performing straightedge measurements, submit areas requiring smoothness correction. Identify locations of smoothness correction by:

1. Location Number
2. District-County-Route
3. Beginning station or post mile to the nearest 0.01 mile

4. For correction areas within a lane:
   4.1. Lane direction as NB, SB, EB, or WB
   4.2. Lane number from left to right in direction of travel
   4.3. Wheel path as "L" for left, "R" for right, or "B" for both
5. For correction areas not within a lane:
   5.1. Identify pavement area (i.e., shoulder, weight station, turnout)
   5.2. Direction and distance from centerline as "L" for left or "R" for right
6. Estimated size of correction area

39-1.01C(13)(c) Inertial Profiler Certification

At least 5 business days before the start of initial profiling or changing profiler or operator, submit:

1. Inertial profiler certification issued by the Department.
2. Operator certification for the inertial profiler issued by the Department.
3. List of manufacturer's recommended test procedures for the inertial profiler calibration and verification.

Within 2 business days after cross-correlation testing, submit ProVAL profiler certification analysis report for cross-correlation test results performed on test section to the Engineer and to the electronic mailbox address:

smoothness@dot.ca.gov

39-1.01C(13)(d) Inertial Profiler Data

Within 2 business days after each day of inertial profiling, submit profile data to the Engineer and to the electronic mailbox address:

smoothness@dot.ca.gov

The profiling data must include:

1. Raw profile data for each lane.
2. ProVAL ride quality analysis report for the International Roughness Index of left and right wheel paths of each lane. Submit this report in pdf file format.
3. ProVAL ride quality analysis report for the Mean Roughness Index of each lane. Submit this report in pdf file format.
4. ProVAL smoothness assurance analysis report for the International Roughness Index of left wheel path. Submit this report in pdf file format.
5. ProVAL smoothness assurance analysis report for the International Roughness Index of right wheel path. Submit this report in pdf file format.
6. GPS data file for each lane in GPS eXchange. Submit data file in GPS eXchange file format.
7. Manufacturer's recommended inertial profiler calibration and verification test results.
8. Inertial profiler calibration and verification test results including bounce, block, and distance measurement instrument.

Submit the raw profile data in unfiltered electronic pavement profile file (PPF) format. Name the PPF file using the following naming convention:

YYYYMMDD_TTCCCRRR_D_L_W_S_E_X_PT.PPF

where:
YYYY = year
MM = Month, leading zero
DD = Day of month, leading zero
TT = District, leading zero
CCC = County, 2 or 3 letter abbreviation as shown in section 1-1.08
RRR = Route number, no leading zeros
D = Traffic direction as NB, SB, WB, or EB
L = Lane number from left to right in direction of travel
W = Wheel path as "L" for left, "R" for right, or "B" for both

S = Beginning station to the nearest foot (i.e., 10+20) or beginning post mile to the nearest hundredth (i.e., 25.06) no leading zero
E = Ending station to the nearest foot (i.e., 14+20) or ending post mile to the nearest hundredth (i.e., 28.06) no leading zero
X = Profile description as "EXIST" for existing pavement, "INTER" for after prepaving smoothness correction, "PAVE" for after paving, and "CORR" for after final surface pavement correction
PT = HMA pavement type

39-1.01C(13)(e) Reserved

39-1.01C(14)–39-1.01C(15) Reserved

39-1.01D Quality Control and Assurance

39-1.01D(1) General

When testing under AASHTO T 324 (Modified), test under AASHTO T 324 with the following parameters:

1. Target air voids must equal 7 ± 1 percent
2. Specimen height must be 60 ± 1 mm
3. Number of test specimens must be 4 (2 test sets)
4. Do not average test sets
5. Test specimen must be a 150 mm gyratory compacted specimen
6. Test temperature must be set at:
   6.1. 113 ± 2 degrees F for PG 58
   6.2. 122 ± 2 degrees F for PG 64
   6.3. 131 ± 2 degrees F for PG 70 and above
7. Measurements for impression must be taken at every 100 passes
8. Inflection point defined as the number of wheel passes at the intersection of the creep slope and the stripping slope
9. Testing shut off must be set at 25,000 passes

Take samples under California Test 125.

HMA samples may be heated a maximum of 2 times for up to 4 hours each.

39-1.01D(2) Job Mix Formula Verification

The Engineer verifies the JMF from samples taken from HMA produced by the plant to be used. Notify the Engineer at least 2 business days before sampling materials. Samples may be taken from a different project including a non-Department project, if you make arrangements for the Engineer to be present during sampling.

In the Engineer's presence and from the same production run, take samples of:

1. Aggregate. Coarse, fine, and supplemental fine aggregate must be taken from the combined cold feed belt, or hot bins. If lime treatment is required, samples must be taken from individual stockpiles before lime treatment. Samples must be at least 120 lb for each coarse aggregate, 80 lb for each fine aggregate, and 10 lb for each type of supplemental fines. For hot bin samples, the Department combines these aggregate samples to comply with the TV submitted on a Contractor Job Mix Formula Proposal form.
2. Asphalt binder. Take 2 samples minimum. Each sample must be in a 1-quart cylindrical-shaped can with an open top and friction lid. If the asphalt binder is modified or rubberized, the asphalt binder must be sampled with the components blended in the proportions to be used.
3. RAP. RAP samples must be at least 50 lb from each fractionated stockpile used or 100 lb from the belt.
4. Plant-produced HMA. The HMA samples must be at least 250 lb.

For aggregate, RAP, and HMA, split the samples into at least 4 parts and label their containers. Three parts are for the Department's verification testing and 1 part is for your testing.

After acceptance of the JMF submittal, the Engineer verifies each proposed JMF within 20 days of receiving all verification samples.

For JMF verification, the Engineer tests the following for compliance with the specifications:

1. Aggregate quality
2. Aggregate gradation
3. Voids in mineral aggregate on laboratory-produced HMA must comply with the mix design specifications for voids in mineral aggregate
4. HMA quality characteristics for Department acceptance

To verify the HMA for air voids, voids in mineral aggregate, and dust proportion, the Engineer uses an average of 3 briquettes. The Engineer tests plant-produced material.

If the Engineer verifies the JMF, the Engineer furnishes you a Hot Mix Asphalt Verification form.

If the Engineer's test results on plant-produced samples do not show compliance with the specifications, the Engineer notifies you. Adjust your JMF based on your testing unless the Engineer authorizes reverification without adjustments. JMF adjustments may include a change in:

1. Asphalt binder content target value up to ±0.2 percent from the OBC value submitted on Contractor Hot Mix Asphalt Design Data form
2. Aggregate gradation target values within the target value limits specified in the aggregate gradation table

You may adjust the JMF only once due to a failed verification test.

For each HMA type and aggregate size specified, the Engineer verifies up to 2 proposed JMF submittals including a JMF adjusted after verification failure. If you submit more than 2 JMFs for each type of HMA and aggregate size, the Engineer deducts $3,000 from payments for each verification exceeding this limit. This deduction does not apply to verifications initiated by the Engineer or if a JMF expires while HMA production is stopped longer than 30 days.

A verified JMF is valid for 12 months.

39-1.01D(3) Job Mix Formula Authorization

You may start HMA production if:

1. The Engineer's review of the JMF shows compliance with the specifications
2. The Department has verified the JMF within 12 months before HMA production
3. The Engineer authorizes the verified JMF

39-1.01D(4) Job Mix Formula Renewal

For a JMF renewal and upon request, in the Engineer's presence and from the same production run, take samples of:

1. Aggregate. Coarse, fine, and supplemental fine aggregate must be taken from combined cold-feed belt, or hot bins. If lime treatment is required, samples must be taken from individual stockpiles before lime treatment. Samples must be at least 120 lb for each coarse aggregate, 80 lb for each fine aggregate, and 10 lb for each type of supplemental fines. For hot bins, the Department combines these aggregate samples to comply with the TV submitted on a Contractor Job Mix Formula Proposal form.
2. Asphalt binder. Take 2 samples minimum. Each sample must be in a 1-quart cylindrical-shaped can with an open top and friction lid. If the asphalt binder is modified or rubberized, the asphalt binder must be sampled with the components blended in the proportions to be used.
3. RAP. RAP samples must be at least 50 lb from each fractionated stockpile.
4. Plant-produced HMA. The HMA samples must be at least 250 lb.

Notify the Engineer at least 2 business days before sampling materials. For aggregate, RAP, and HMA, split samples into at least 4 parts. Submit 3 parts to the Engineer and use 1 part for your testing.

Allow the Engineer 5 business days from a complete JMF reverification submittal for document review of the aggregate qualities, mix design, and JMF.

The most recent aggregate quality test results within the past 12 months may be used for verification of JMF renewal or upon request, the Engineer may perform aggregate quality tests for verification of JMF renewal.

The Engineer verifies the JMF for renewal under section 39-1.01D(2) except:

1. The Engineer keeps the samples until you provide test results for your part on a Contractor Job Mix Formula Renewal form.
2. The Department tests samples of materials obtained from the HMA production unit after you submit test results that comply with the mix design specifications.
3. After completion of the JMF verification renewal document review, the Engineer verifies each proposed JMF within 20 days of receiving the verification renewal samples and the complete Contractor Job Mix Formula Renewal form.
4. You may not adjust the JMF due to a failed verification.
5. For each HMA type and aggregate gradation specified, the Engineer verifies at no cost to you 1 proposed JMF renewal within a 12-month period.

If the Engineer verifies the JMF renewal, the Engineer furnishes you a Hot Mix Asphalt Verification form. The Hot Mix Asphalt Verification form is valid for 12 months.

39-1.01D(5) Job Mix Formula Modification

The Engineer verifies the modified JMF after the modified JMF HMA is placed on the project and verification samples are taken within the first 750 tons. The Engineer tests verification samples for compliance with:

1. Hamburg wheel track mix design specifications
2. Air void content
3. Voids in mineral aggregate on plant-produced HMA mix design specifications
4. Dust proportion mix design specifications

The Engineer may test for moisture susceptibility for compliance with the mix design specifications.

If the modified JMF is verified, the Engineer revises your Hot Mix Asphalt Verification form to include the new asphalt binder source, new liquid antistrip producer, or new liquid antistrip dosage. Your revised form will have the same expiration date as the original form.

If a modified JMF is not verified, stop production and any HMA placed using the modified JMF is rejected.

The Engineer deducts $2,000 from payments for each JMF modification.

39-1.01D(6) Certifications

39-1.01D(6)(a) General

Laboratories testing aggregate and HMA qualities used to prepare the mix design and JMF must be qualified under AASHTO Materials Reference Laboratory program and the Department's Independent Assurance Program.

39-1.01D(6)(b) Hot Mix Asphalt Plants

Before production, the HMA plant must have a current qualification under the Department's Material Plant Quality Program.

39-1.01D(6)(c) Inertial Profiler Certifications

The inertial profiler equipment must display a current certification decal with expiration date.

The inertial profiler operator and device certifications must be not more than 12 months old.

The operator must be certified for each different model of inertial profiler device operated.

39-1.01D(6)(d)–39-1.01D(6)(e) Reserved

39-1.01D(7) Prepaving Meeting

Meet with the Engineer at a prepaving meeting at a mutually agreed time and place. Discuss the QC plan and the methods of performing HMA production and paving work.

The following personnel must attend the prepaving meeting:

1. Project manager

2. Superintendent
3. HMA plant manager
4. HMA paving foreman

If a warm mix asphalt technology is used, a technical representative for warm mix asphalt technology must attend the prepaving meeting.

39-1.01D(8) Quality Control

39-1.01D(8)(a) General

QC test results must comply with the specifications for Department acceptance.

Prepare 3 briquettes for air voids content and voids in mineral aggregate determination. Report the average of 3 tests.

Except for smoothness, if 2 consecutive QC test results or any 3 QC test results for 1 day's production do not comply with the materials specifications:

1. Stop HMA production
2. Notify the Engineer
3. Take corrective action
4. Demonstrate compliance with the specifications before resuming production and placement

For QC tests performed under AASHTO T 27, results are considered 1 QC test regardless of number of sieves out of compliance.

Do not resume production and placement until the Engineer authorizes your corrective action proposal.

39-1.01D(8)(b) Reserved

39-1.01D(8)(c) Aggregate

39-1.01D(8)(c)(i) General

Reserved

39-1.01D(8)(c)(ii) Aggregate Lime Treatments

If lime treatment is required, sample coarse and fine aggregate from individual stockpiles before lime treatment. Combine aggregate in the JMF proportions. Test the aggregate under the test methods and frequencies shown in the following table:

Aggregate Quality Control During Lime Treatment

Quality characteristic	Test method	Minimum sampling and testing frequency
Sand equivalent[a, b]	AASHTO T 176	1 per 750 tons of untreated aggregate
Percent of crushed particles	AASHTO T 335	1 per 10,000 tons or 2 per project whichever is greater
Los Angeles Rattler	AASHTO T 96	
Fine aggregate angularity	AASHTO T 304 Method A	
Flat and elongated particles	ASTM D4791	

[a]Report test results as the average of 3 tests from a single sample.
[b]Use of a sand reading indicator is required as shown in AASHTO T 176, Figure 1. Sections 4.7, 4.8, 7.1.2, abd 8.4.3 do not apply.

For lime slurry aggregate treatment, determine the aggregate moisture content at least once every 2 hours of treatment. Calculate moisture content under AASHTO T 329 and report it as a percent of dry aggregate weight. Use the moisture content calculations as a set point for the proportioning process controller.

The device controlling lime and aggregate proportioning must produce a treatment data log. The log consists of a series of data sets captured at 10-minute intervals throughout daily treatment. The data must be a treatment activity register and not a summation. The material represented by a data set is the quantity produced 5 minutes before and 5 minutes after the capture time. For the duration of the Contract, collected data must be stored by the controller.

If 3 consecutive sets of recorded treatment data indicate a deviation of more than 0.2 percent above or below the lime ratio in the accepted JMF, stop treatment and take corrective action.

If a set of recorded treatment data indicates a deviation of more than 0.4 percent above or below the lime ratio in the accepted JMF, stop treatment and do not use the material represented by that set of data in HMA.

If 20 percent or more of the total daily treatment indicates a deviation of more than 0.2 percent above or below the lime ratio in the accepted JMF, stop treatment and do not use that day's treated aggregate in HMA.

The Engineer may order you to stop aggregate treatment activities for any of following:

1. You fail to submit treatment data log
2. You fail to submit aggregate QC data for marinated aggregate
3. You submit incomplete, untimely, or incorrectly formatted data
4. You do not take corrective actions
5. You take late or unsuccessful corrective actions
6. You do not stop treatment when proportioning tolerances are exceeded
7. You use malfunctioning or failed proportioning devices

If you stop treatment for noncompliance, notify the Engineer of any corrective actions taken and conduct a successful 20-minute test run before resuming treatment.

39-1.01D(8)(d) Liquid Antistrip Treatment

For continuous mixing or batch-plant mixing, sample asphalt binder before adding liquid antistrip. For continuous mixing, sample the combined asphalt binder and liquid antistrip after the static mixer.

39-1.01D(8)(e) Production Start-up Evaluation

You and the Engineer evaluate HMA production and placement at production start-up.

Within the first 750 tons produced on the 1st day of HMA production, in the Engineer's presence, and from the same production run, take samples of:

1. Aggregate
2. Asphalt binder
3. RAP
4. HMA

Sample aggregate from the combined cold-feed belt or hot bin. Take RAP samples from the RAP system.

For aggregate, RAP, and HMA, split the samples into at least 4 parts and label their containers. Submit 3 parts to the Engineer and keep 1 part.

You and the Engineer must test the samples and report test results, except for AASHTO T 324 (Modified) and AASHTO T 283 test results, within 5 business days of sampling. For AASHTO T 324 (Modified) and AASHTO T 283 test results, report test results within 15 days of sampling. If you proceed before receipt of the test results, the Engineer may consider the HMA placed to be represented by these test results.

Take one 4- or 6-inch diameter density core for each 250 tons or portion thereof of HMA placed. For each density core, the Engineer reports the bulk specific gravity determined under AASHTO T 275, Method A, in addition to the percent of theoretical maximum density.

39-1.01D(8)(f) Hot Mix Asphalt Density

During HMA placement determine HMA density using a nuclear gauge. On the 1st day of production, develop a correlation factor between cores and nuclear gauge under California Test 375.

Test for in-place density using cores and a nuclear gauge. Test at random locations you select and include the test results in your QC production tests reports.

39-1.01D(8)(g) Tapered Notched Wedge

Perform QC testing on the completed tapered notched wedge joint as follows:

1. Perform field compaction tests at the rate of 1 test for each 750-foot section along the joint. Select random locations for testing within each 750-foot section.
2. Perform field compaction tests at the centerline of the joint, 6 inches from the upper vertical notch, after the adjacent lane is placed and before opening the pavement to traffic.
3. Determine theoretical maximum density.
4. Determine percent compaction of the longitudinal joint as the ratio of the daily average of the field compaction values and the maximum density test results.

Determine percent compaction values each day the tapered notched wedge joint is completed. If the percent compaction of 1 day's production is less than 91 percent, that day's notched wedge joint is rejected. Discontinue placement of the tapered notched wedge and notify the Engineer of changes you will make to your construction process in order to comply with the specifications.

39-1.01D(8)(h) Density Cores

To determine density, take 4- or 6-inch diameter density cores at least once every 5 business days. Take 1 density core for every 250 tons of HMA from random locations the Engineer designates. Take density cores in the Engineer's presence and backfill and compact holes with authorized material. Before submitting a density core, mark it with the density core's location and place it in a protective container.

If a density core is damaged, replace it with a density core taken within 1 foot longitudinally from the original density core. Relocate any density core located within 1 foot of a rumble strip to 1 foot transversely away from the rumble strip.

For a tapered notched wedge joint, take 4- or 6-inch diameter density cores 6 inches from the upper vertical notch of the completed longitudinal joint for every 3,000 feet at locations designated by the Engineer. Take cores after the adjacent lane is placed and before opening the pavement to traffic. Cores must be taken in the presence of the Engineer and backfill and compact holes with authorized material. Before submitting a density core, mark it with the core's location and place it in a protective container.

39-1.01D(8)(i) Reserved

39-1.01D(8)(j) Pavement Smoothness

39-1.01D(8)(j)(i) General

Test pavement smoothness using an inertial profiler except use a 12-foot straightedge for the HMA pavement at the following locations:

1. Traffic lanes less than 1,000 feet in length including ramps, turn lanes, and acceleration and deceleration lanes
2. HMA pavement within 3 feet from and parallel to the construction joint formed between curbs, gutters, or existing pavement
3. Areas within 15 feet of manholes
4. Shoulders
5. Weigh-in-motion areas
6. Miscellaneous areas such as medians, gore areas, turnouts, and maintenance pullouts

Where inertial profiler testing is required, pavement smoothness for each lane must be determined by the International Roughness Index for the left and right wheel paths in an individual lane and then averaging the results. The average of the International Roughness Index values from the left and right wheel paths for the same lane is the Mean Roughness Index of the lane. The wheel paths are a pair of lines 3 feet from and parallel to the edge of a lane. Left and right wheel paths are based on the direction of travel.

Where inertial profiler testing is required, identify areas of localized roughness. Areas of localized roughness must be identified using the FHWA's engineering software ProVAL smoothness assurance analysis by calculating continuous International Roughness Index values for each wheel path with a 25-foot interval using a 250 mm filter.

Collect profiling data under AASHTO R 56 and analyze data using 250 mm and International Roughness Index filters.

39-1.01D(8)(j)(ii) Inertial Profiler Calibration and Verification Tests

Operate the inertial profiler according to the manufacturer's instructions and AASHTO R 57 at 1-inch recording intervals.

Notify the Engineer 2 business days before performing inertial profiler calibration and verification testing.

Conduct the following inertial profiler calibration and verification tests in the Engineer's presence each day before performing inertial profiling:

1. Block test. Verify the height sensor accuracy under California Test 387.
2. Bounce test. Verify the combined height sensor and accelerometer accuracy under California Test 387.
3. Distance measurement instrument test. Calibrate the accuracy of the testing procedure under California Test 387.
4. Manufacturer's recommended tests.

Conduct cross-correlation inertial profiler verification test in the Engineer's presence before performing initial profiling. Verify cross-correlation inertial profiler verification test at least annually. Conduct 5 repeat runs of the inertial profiler on an authorized test section. The test section must be on an existing asphalt concrete pavement surface 0.1 mile long. Calculate a cross-correlation to determine the repeatability of your device under California Test 387 using ProVAL profiler certification analysis with a 3 feet maximum offset. The cross-correlation must be a minimum of 0.92.

For each 0.1 mile section, your International Roughness Index values must be within 10 percent of the Department's International Roughness Index values. The Engineer may order you to recalibrate your inertial profiler equipment and reprofile. If your results are inaccurate due to operator error, the Engineer may disqualify your inertial profiler operator.

39-1.01D(8)(j)(iii) Smoothness Testing

Notify the Engineer of start location by station and start time at least 2 business days before profiling.

Remove foreign objects on the pavement surface before profiling.

Mark the beginning and ending station on the pavement shoulder before profiling. Stationing must be the same when profiling more than one surface.

While collecting the profile data to determine the International Roughness Index values, record the following locations in the raw profile data:

1. Begin and end of all bridge approach slabs
2. Begin and end of all bridges
3. Begin and end of all culverts visible on the roadway surface

Determine the Mean Roughness Index for 0.1-mile fixed sections using the ProVAL ride quality analysis with a 250 mm filter. Profile the left and right wheel paths of each lane. Calculate the Mean Roughness Index of each lane. A partial section less than 0.1 mile that is the result of an interruption to continuous pavement surface must comply with the Mean Roughness Index specifications for a full section. Adjust the Mean Roughness Index for a partial section to reflect a full section based on the proportion of a section paved.

Determine the areas of localized roughness using a continuous International Roughness Index for each wheel path with a 25-foot interval using a 250 mm filter.

Pavement smoothness must comply with the specifications in section 39-1.01D(9)(c).

39-1.01D(9) Department Acceptance

39-1.01D(9)(a) General

The Department tests treated aggregate for acceptance before lime treatment except for gradation.

The Engineer takes HMA samples for AASHTO T 283 and AASHTO T 324 (Modified) from one of the following:

1. Automatic sampling device at the plant
2. Automatic sampling device at the truck
3. Windrow

The Engineer takes HMA samples for all other tests from one of the following:

1. Automatic sampling device at the plant
2. Automatic sampling device at the truck
3. Windrow
4. Mat behind the paver

The Engineer's sampling and testing is independent of your QC sampling and testing.

If you request, the Engineer splits samples and provides you with a part.

No single test result may represent more than 750 tons or one day's production, whichever is less, excluding AASHTO T 283 and AASHTO T 324 (Modified).

Except for smoothness, if 2 consecutive Department acceptance test results or any 3 Department acceptance test results for 1 day's production do not comply with the specifications:

1. Stop HMA production
2. Take corrective action
3. Demonstrate compliance with the specifications before resuming production and placement

The Engineer accepts HMA based on:

1. Authorized JMF
2. Authorized QC plan
3. Asphalt binder compliance
4. Asphalt emulsion compliance
5. Visual inspection
6. Pavement smoothness

39-1.01D(9)(b) In-Place Density

The Engineer tests the density core you take from each 250 tons of HMA. The Engineer determines the percent of theoretical maximum density for each density core by determining the density core's density and dividing by the theoretical maximum density.

The Department determines the percent of maximum theoretical density from density cores if any of the following applies:

1. 1/2-inch, 3/8-inch, or no. 4 aggregate gradation is used and the specified total paved thickness is greater than 0.15 foot and any layer is less than 0.15 foot.
2. 3/4-inch aggregate gradation is used and the specified total paved thickness is greater than 0.20 foot and any layer is less than 0.20 foot.

Density cores must be taken from the final layer, cored to the specified total paved thickness.

If the percent of theoretical maximum density does not comply with the specifications, the Engineer may accept the HMA and take a payment deduction.

For acceptance of a completed tapered notched wedge joint, the Engineer determines density from cores based on:

1. Field compaction by measuring the bulk specific gravity of the cores under AASHTO T 275, Method A
2. Percent compaction as the ratio of the average of the bulk specific gravity of the core for each day's production to the maximum density test value

39-1.01D(9)(c) Pavement Smoothness

For areas that require pavement smoothness determined using an inertial profiler, the pavement surface must:

1. Have no areas of localized roughness with an International Roughness Index greater than 160 in/mi
2. Comply with the Mean Roughness Index requirements shown in the following table for a 0.1 mile section:

HMA[a] Pavement Smoothness Acceptance Criteria

HMA thickness	Mean Roughness Index requirement
> 0.20 foot	60 in/mi or less
≤ 0.20 foot	75 in/mi or less

[a] Except OGFC

The final surface of HMA must comply with the Mean Roughness Index requirements before placing OGFC. Correct pavement to the Mean Roughness Index specifications. Localized roughness greater than 160 in/mi must be corrected regardless of the International Roughness Index values of a 0.1-mile section.

For areas that require pavement smoothness determined using a 12-foot straightedge, the HMA pavement surface must not vary from the lower edge of the straightedge by more than:

1. 0.01 foot when the straightedge is laid parallel with the centerline
2. 0.02 foot when the straightedge is laid perpendicular to the centerline and extends from edge to edge of a traffic lane
3. 0.02 foot when the straightedge is laid within 24 feet of a pavement conform

Pavement smoothness may be accepted based on your testing in the absence of the Department's testing.

39-1.01D(9)(d) Dispute Resolution

You and the Engineer must work together to avoid potential conflicts and to resolve disputes regarding test result discrepancies. Notify the Engineer within 5 business days of receiving a test result if you dispute the test result.

If you or the Engineer dispute each other's test results, submit QC test results and copies of paperwork including worksheets used to determine the disputed test results. An independent third party performs referee testing. Before the third party participates in a dispute resolution, it must be qualified under AASHTO Materials Reference Laboratory program, and the Department's Independent Assurance Program. The independent third party must have no prior direct involvement on this Contract. By mutual agreement, the independent third party is chosen from:

1. Department laboratory in a district or region not in the district or region the project is located
2. Transportation Laboratory
3. Laboratory not currently employed by you or your HMA producer

If split QC or acceptance samples are not available, the independent third party uses any available material representing the disputed HMA for evaluation.

If the independent third party determines the Department's test results are valid, the Engineer deducts the independent third party's testing costs from payments. If the independent third party determines your test results are valid, the Department pays the independent third party's testing costs.

39-1.02 MATERIALS

39-1.02A General

Reserved

39-1.02B Mix Design

39-1.02B(1) General

The HMA mix design must comply with AASHTO R 35 except:

1. Notes 3, 6, and 10 do not apply
2. AASHTO M 323 does not apply on combinations of aggregate gradation and asphalt binder contents to determine the OBC and HMA mixture qualities

The Contractor Hot Mix Asphalt Design Data form must show documentation on aggregate quality.

39-1.02B(2) Hot Mix Asphalt Treatments

If the test results for AASHTO T 283 or AASHTO T 324 (Modified) for untreated plant-produced HMA are less than the minimum requirements for HMA mix design, determine the plasticity index of the aggregate blend under California Test 204.

If the plasticity index is greater than 10, do not use that aggregate blend.

If the plasticity index is from 4 to 10, treat the aggregate with dry lime with marination or lime slurry with marination.

If the plasticity index is less than 4, treat the aggregate with dry lime or lime slurry with marination, or treat the HMA with liquid antistrip.

39-1.02B(3) Warm Mix Asphalt Technology

For HMA with warm mix asphalt additive technology, produce HMA mix samples for your mix design using your methodology for inclusion of warm mix asphalt admixture in laboratory-produced HMA. For warm mix asphalt water injection foam technology, the use of foamed asphalt for mix design is not required.

39-1.02C Asphalt Binder

Asphalt binder must comply with section 92.

For replace asphalt concrete surfacing or hot mix asphalt (leveling) the grade of asphalt binder for the HMA must be PG 64-10 or PG 64-16.

39-1.02D Aggregate

39-1.02D(1) General

Aggregate must be clean and free from deleterious substances.

The aggregate for replace asphalt concrete surfacing and hot mix asphalt (leveling) must comply with the gradation specifications for Type A HMA in section 39-2.02.

39-1.02D(2) Aggregate Gradations

Aggregate gradation must be determined before the addition of asphalt binder and must include supplemental fines. Test for aggregate gradation under AASHTO T 27. Note 4 of AASHTO T 27 and AASHTO T 11 do not apply. Use a mechanical sieve shaker. Aggregate shaking time must not exceed 10 minutes for each coarse and fine aggregate portion.

Choose a target value within the target value limits shown in the tables titled "Aggregate Gradations."

Gradations are based on nominal maximum aggregate size.

39-1.02D(3) Aggregate Lime Treatments

39-1.02D(3)(a) General

If aggregate lime treatment is required, virgin aggregate must comply with the aggregate quality specifications.

Lime for treating aggregate must comply with section 24-2.02B.

Water for lime treatment of aggregate with lime slurry must comply with section 24-2.02C.

Notify the Engineer at least 24 hours before the start of aggregate treatment.

Do not treat RAP.

The lime ratio is the pounds of dry lime per 100 lb of dry virgin aggregate expressed as a percentage. Water content of slurry or untreated aggregate must not affect the lime ratio.

Coarse and fine aggregate fractions must have the lime ratio ranges shown in the following table:

Aggregate fractions	Lime ratio percent
Coarse	0.4–1.0
Fine	1.5–2.0
Combined	0.8–1.5

The lime ratio for fine and coarse aggregate must be within ±0.2 percent of the lime ratio in the accepted JMF. The lime ratio must be within ±0.2 percent of the authorized lime ratio when you combine the individual aggregate sizes in the JMF proportions. The lime ratio must be determined before the addition of RAP.

If marination is required, marinate treated aggregate in stockpiles from 24 hours to 60 days before using in HMA. Do not use aggregate marinated longer than 60 days.

Treated aggregate must not have lime balls or clods.

39-1.02D(3)(b) Dry Lime

If marination is required:

1. Treat and marinate coarse and fine aggregates separately
2. Treat the aggregate and stockpile for marination only once
3. Treat the aggregate separate from HMA production

Proportion dry lime by weight with an automatic continuous proportioning system.

If you use a batch-type proportioning system for HMA production, control proportioning in compliance with the specifications for continuous mixing plants. Use a separate dry lime aggregate treatment system for HMA batch mixing including:

1. Pugmill mixer
2. Controller
3. Weigh belt for the lime
4. Weigh belt for the aggregate

If using a continuous mixing plant for HMA production without lime marinated aggregates, use a controller that measures the blended aggregate weight after any additional water is added to the mixture. The controller must determine the quantity of lime added to the aggregate from the aggregate weigh belt input in connection with the manually input total aggregate moisture, the manually input target lime content, and the lime proportioning system output. Use a continuous aggregate weigh belt and pugmill mixer for lime treatment in addition to the weigh belt for the aggregate proportioning to asphalt binder in the HMA plant. If you use a water meter for moisture control for lime treatment, the meter must comply with Department's Material Plant Quality Program manual.

At the time of mixing dry lime with aggregate, the aggregate moisture content must ensure complete lime coating. The aggregate moisture content must not cause aggregate to be lost between the point of weighing the combined aggregate continuous stream and the dryer. Add water to the aggregate for mixing and coating before dry lime addition. Immediately before mixing lime with aggregate, water must not visibly separate from the aggregate.

Mix aggregate, water, and dry lime with a continuous pugmill mixer with twin shafts. Immediately before mixing lime with aggregate, water must not visibly separate from the aggregate. Store dry lime in a uniform and free-flowing condition. Introduce dry lime to the pugmill in a continuous process. The introduction must occur after the aggregate cold feed and before the point of proportioning across a weigh belt and the aggregate dryer. Prevent loss of dry lime.

The pugmill must be equipped with paddles arranged to provide sufficient mixing action and mixture movement. The pugmill must produce a homogeneous mixture of uniformly coated aggregates at mixer discharge.

If the aggregate treatment process is stopped longer than 1 hour, clean the equipment of partially treated aggregate and lime.

Aggregate must be completely treated before introduction into the mixing drum.

39-1.02D(3)(c) Lime Slurry

For lime slurry aggregate treatment, treat aggregate separate from HMA production. Stockpile and marinate the aggregate.

Proportion lime and water with a continuous or batch mixing system.

Add lime to the aggregate as slurry consisting of mixed dry lime and water at a ratio of 1 part lime to from 2 to 3 parts water by weight. The slurry must completely coat the aggregate.

Immediately before mixing lime slurry with the aggregate, water must not visibly separate from the aggregate.

Proportion lime slurry and aggregate by weight in a continuous process.

39-1.02E Liquid Antistrip Treatment

Liquid antistrip must be from 0.25 to 1.0 percent by weight of asphalt binder. Do not use liquid antistrip as a substitute for asphalt binder.

Liquid antistrip total amine value must be 325 minimum when tested under ASTM D2074.

Use only 1 liquid antistrip type or brand at a time. Do not mix liquid antistrip types or brands.

Store and mix liquid antistrip under the manufacturer's instructions.

39-1.02F–39-1.02G Reserved

39-1.02H Hot Mix Asphalt Production

39-1.02H(1) General

Do not start HMA production before verification and authorization of JMF.

HMA plants must be Department-qualified. Before production, the HMA plant must have a current qualification under the Department's Materials Plant Quality Program.

For lime treated aggregate, the HMA plant must be equipped with a bag-house dust system. Material collected in the dust system must be returned to the mix.

Weighing and metering devices used for the production of HMA modified with additives must comply with the requirements of the Department's Material Plant Quality Program. If a loss-in-weight meter is used for dry HMA additive, the meter must have an automatic and integral material delivery control system for the refill cycle.

Calibrate the loss-in-weight meter by:

1. Including at least 1 complete system refill cycle during each calibration test run
2. Operating the device in a normal run mode for 10 minutes immediately before starting the calibration process
3. Isolating the scale system within the loss-in-weight feeder from surrounding vibration
4. Checking the scale system within the loss-in-weight feeder for accuracy before and after the calibration process and daily during mix production
5. Using a 15-minute or 250-pound-minimum test run size for a dry ingredient delivery rate of less than 1 ton per hour.
6. Complying with the limits of Table B, "Conveyor Scale Testing Extremes," in the Department's Material Plant Quality Program

Proportion aggregate by hot or cold-feed control. During production, you may adjust hot or cold-feed proportion controls for virgin aggregate and RAP.

Aggregate temperature must not be more than 375 degrees F when mixed with the asphalt binder. Temperature requirements do not apply to RAP.

Asphalt binder temperature must be from 275 to 375 degrees F when mixed with aggregate.

Mix HMA ingredients into a homogeneous mixture of coated aggregates.

HMA with or without RAP must not be more than 325 degrees F.

For HMA produced using warm mix asphalt technology, HMA must be at a temperature between 240 and 325 degrees F.

If method compaction is used, HMA must be produced at a temperature between 305 and 325 degrees F.

If you stop production for longer than 30 days, a production start-up evaluation is required.

39-1.02H(2) Liquid Antistrip

If 3 consecutive sets of recorded production data show actual delivered liquid antistrip weight is more than ±1 percent of the authorized mix design liquid antistrip weight, stop production and take corrective action.

If a set of recorded production data shows actual delivered liquid antistrip weight is more than ±2 percent of the authorized mix design liquid antistrip weight, stop production. If the liquid antistrip weight exceeds 1.2 percent of the asphalt binder weight, do not use the HMA represented by that data.

The continuous mixing plant controller proportioning the HMA must produce a production data log. The log consists of a series of data sets captured at 10-minute intervals throughout daily production. The data must be a production activity register and not a summation. The material represented by the data is the quantity produced 5 minutes before and 5 minutes after the capture time. For the duration of the Contract, collected data must be stored by the plant controller or a computer's memory at the plant.

The Engineer orders proportioning activities stopped for any of the following:

1. You do not submit data
2. You submit incomplete, untimely, or incorrectly formatted data
3. You do not take corrective actions
4. You take late or unsuccessful corrective actions
5. You do not stop production when proportioning tolerances are exceeded
6. You use malfunctioning or failed proportioning devices

If you stop production, notify the Engineer of any corrective actions taken before resuming.

39-1.02H(3) Warm Mix Asphalt Technology

Proportion all ingredients by weight. The HMA plant process controller must be the sole source of ingredient proportioning control and be fully interfaced with all scales and meters used in the production process. The addition of the HMA additive must be controlled by the plant process controller.

Liquid ingredient additive, including a normally dry ingredient made liquid, must be proportioned with a mass flow meter at continuous mixing plants. Use a mass flow meter or a container scale to proportion liquid additives at batch mixing plants.

Continuous mixing plants using HMA additives must comply with the following:

1. Dry ingredient additives for continuous production must be proportioned with a conveyor scale or a loss-in-weight meter.
2. HMA plant process controller and ingredient measuring systems must be capable of varying all ingredient feed rates proportionate with the dry aggregate delivery at all production rates and rate changes.
3. Liquid HMA additive must enter the production stream with the binder. Dry HMA additive must enter the production stream at or before the mixing area.

4. If dry HMA additives are used at continuous mixing HMA plants, baghouse dust systems must return all captured material to the mix.
5. HMA additive must be proportioned to within ±0.3 percent of the target additive rate.

Batch mixing plants using HMA additives must comply with the following:

1. Metered HMA additive must be placed in an intermediate holding vessel before being added to the stream of asphalt binder as it enters the pugmill.
2. If a container scale is used, weigh additive before combining with asphalt binder. Keep the container scale separate from other ingredient proportioning. The container scale capacity must be no more than twice the volume of the maximum additive batch size. The container scale's graduations must be smaller than the proportioning tolerance or 0.001 times the container scale capacity.
3. Dry HMA additive proportioning devices must be separate from metering devices for the aggregates and asphalt binder. Proportion dry HMA additive directly into the pugmill or place in an intermediate holding vessel to be added to the pugmill at the appropriate time in the batch cycle. Dry ingredients for batch production must be proportioned with a hopper scale.
4. Zero tolerance for the HMA additive batch scale is ±0.5 percent of the target additive weight. The indicated HMA additive batch scale weight may vary from the preselected weight setting by up to ±1.0 percent of the target additive weight.

39-1.02I Geosynthetic Pavement Interlayer

Geosynthetic pavement interlayer must comply with the specifications for pavement fabric, paving mat, paving grid, paving geocomposite grid, or geocomposite strip membrane as shown.

The asphalt binder for geosynthetic pavement interlayer must be PG 64-10, PG 64-16, or PG 70-10.

39-1.02J Tack Coat

Tack coat must comply with the specifications for asphaltic emulsion or asphalt binder. Choose the type and grade.

39-1.02K Miscellaneous Areas and Dikes

For miscellaneous areas and dikes:

1. Choose either the 3/8-inch or 1/2-inch aggregate gradation for Type A HMA.
2. Minimum asphalt binder content must be 6.8 percent for 3/8-inch aggregate and 6.0 percent for 1/2-inch aggregate. If you request and the Engineer authorizes, you may reduce the minimum asphalt binder content.
3. Choose asphalt binder Grade PG 64-10, PG 64-16 or PG 70-10.

For HMA used in miscellaneous areas and dikes, sections 39-1.01C, 39-1.01D, 39-1.02B, 39-1.02D(3), and 39-1.02E–J do not apply.

39-1.03 CONSTRUCTION

39-1.03A General

Do not place HMA on wet pavement or frozen surface.

You may deposit HMA in a windrow and load it in the paver if:

1. Paver is equipped with a hopper that automatically feeds the screed
2. Loading equipment can pick up the windrowed material and deposit it in the paver hopper without damaging base material
3. Activities for deposit, pickup, loading, and paving are continuous
4. HMA temperature in the windrow does not fall below 260 degrees F

HMA placed in a windrow on the roadway surface must not extend more than 150 feet in front of the loading equipment or material transfer vehicle.

You may place HMA in 1 or more layers on areas less than 5 feet wide and outside the traveled way, including shoulders. You may use mechanical equipment other than a paver for these areas. The equipment must produce uniform smoothness and texture.

HMA handled, spread, or windrowed must not stain the finished surface of any improvement, including pavement.

Do not use petroleum products such as kerosene or diesel fuel to release HMA from trucks, spreaders, or compactors.

HMA must be free of:

1. Segregation
2. Coarse or fine aggregate pockets
3. Hardened lumps

Where density or data core samples are taken, backfill and compact holes with authorized material.

Complete finish rolling activities before the pavement surface temperature is:

1. Below 150 degrees F for HMA with unmodified binder
2. Below 140 degrees F for HMA with modified binder
3. Below 130 degrees F for HMA with warm mix asphalt technology

39-1.03B Spreading and Compacting Equipment

39-1.03B(1) General

Paving equipment for spreading must be:

1. Self-propelled
2. Mechanical
3. Equipped with a screed or strike-off assembly that can distribute HMA the full width of a traffic lane
4. Equipped with a full-width compacting device
5. Equipped with automatic screed controls and sensing devices that control the thickness, longitudinal grade, and transverse screed slope

Install and maintain grade and slope references.

The screed must be heated and produce a uniform HMA surface texture without tearing, shoving, or gouging.

The paver must not leave marks such as ridges and indentations unless you can eliminate them by rolling.

Rollers must be equipped with a system that prevents HMA from sticking to the wheels. You may use a parting agent that does not damage the HMA or impede the bonding of layers.

In areas inaccessible to spreading and compacting equipment:

1. Spread the HMA by any means to obtain the specified lines, grades, and cross sections
2. Use a pneumatic tamper, plate compactor, or equivalent to achieve thorough compaction

39-1.03B(2) Material Transfer Vehicle

If a material transfer vehicle is specified, the material transfer vehicle must have sufficient capacity to prevent stopping the paver and must be capable of:

1. Either receiving HMA directly from trucks or using a windrow pickup head to load it from a windrow deposited on the roadway surface
2. Remixing the HMA with augers before transferring into the paver's receiving hopper or feed system
3. Transferring HMA directly into the paver's receiving hopper or feed system

39-1.03B(3) Method Compaction Equipment

For method compaction, each paver spreading HMA must be followed by 3 rollers:

1. One vibratory roller specifically designed to compact HMA. The roller must be capable of at least 2,500 vibrations per minute and must be equipped with amplitude and frequency controls. The roller's gross static weight must be at least 7.5 tons.

2. One oscillating type pneumatic-tired roller at least 4 feet wide. Pneumatic tires must be of equal size, diameter, type, and ply. The tires must be inflated to 60 psi minimum and maintained so that the air pressure does not vary more than 5 psi.
3. One steel-tired, 2-axle tandem roller. The roller's gross static weight must be at least 7.5 tons.

Each roller must have a separate operator. Rollers must be self-propelled and reversible.

39-1.03B(4)–39-1.03B(6) Reserved

39-1.03C Surface Preparation

39-1.03C(1) General

Before placing HMA, remove loose paving particles, dirt, and other extraneous material by any means including flushing and sweeping.

39-1.03C(2) Subgrade

Prepare subgrade to receive HMA under the sections for the material involved. Subgrade must be free of loose and extraneous material.

39-1.03C(3) Reserved

39-1.03C(4) Prepaving Inertial Profiler

Section 39-1.03C(4) applies to existing asphalt concrete surfaces receiving an HMA overlay if a bid item for prepaving inertial profiler is shown in the Bid Item List.

Before starting paving activities, perform prepaving inertial profiler measurements. Prepaving inertial profiler includes taking profiles of the existing pavement, analyzing the data with ProVAL to determine existing pavement International Roughness Index, Mean Roughness Index, and areas of localized roughness.

If the Contract includes cold planing, perform prepaving inertial profiler measurements before cold planning.

If the Contract includes replace asphalt concrete surfacing, perform prepaving inertial profiler measurements after replacing the asphalt concrete surfacing.

39-1.03C(5) Prepaving Grinding

Section 39-1.03C(5) applies to all existing asphalt concrete surfaces that will not be cold planned or milled and that will receive an HMA overlay less than or equal to 0.20 foot exclusive of OGFC if a bid item for prepaving grinding day is shown in the Bid Item List.

After performing prepaving inertial profiling, correct areas of localized roughness greater than 180 in/mi.

Prepaving grinding day includes correcting areas of localized roughness, taking profiles of the corrected areas, and submitting profile data as specified in section 39-1.01C(13)(d).

Notify the Engineer of those areas of localized roughness that cannot be corrected by prepaving grinding. The Engineer responds to your notification within 5 business days.

For those areas of localized roughness that cannot be corrected by grinding, the Engineer may order you to either (1) not correct the areas of localized roughness or (2) correct areas of localized roughness by a different method and take profiles of the corrected areas with an inertial profiler. Corrective work performed by a different method, including taking profiles of the corrected areas and associated traffic control, is change order work.

If ordered not to correct areas of localized roughness, the smoothness specifications do not apply to the final pavement surface placed in those areas.

Correct prepaving areas of localized roughness that you predict will cause the final surface of HMA pavement to be noncompliant with the smoothness specifications. After correcting prepaving areas of localized roughness, take profiles of the corrected area and submit profile data as specified in section 39-1.01C(13)(d).

Dispose of grinding residue.

Pave within 7 days of correcting areas.

The final pavement surface must comply with section 39-1.01D(9)(c).

If the Engineer determines more time is required for prepaving grinding than the Contract allows for and if prepaving grinding is a controlling activity, the Engineer makes a time adjustment.

39-1.03C(6) Tack Coat

Apply tack coat:

1. To existing pavement including planed surfaces
2. Between HMA layers
3. To vertical surfaces of:
   3.1. Curbs
   3.2. Gutters
   3.3. Construction joints

Before placing HMA, apply tack coat in 1 application at the minimum residual rate shown in the following table for the condition of the underlying surface:

Tack Coat Application Rates for HMA

HMA over:	Minimum Residual Rates (gal/sq yd)		
	CSS1/CSS1h, SS1/SS1h and QS1h/CQS1h Asphaltic Emulsion	CRS1/CRS2, RS1/RS2 and QS1/CQS1 Asphaltic Emulsion	Asphalt Binder and PMRS2/PMCRS2 and PMRS2h/PMCRS2h Asphaltic Emulsion
New HMA (between layers)	0.02	0.03	0.02
PCC and existing AC surfacing	0.03	0.04	0.03
Planed pavement	0.05	0.06	0.04

Notify the Engineer if you dilute asphaltic emulsion with water. The weight ratio of added water to asphaltic emulsion must not exceed 1 to 1.

Measure added water either by weight or volume under section 9-1.02 or you may use water meters from water districts, cities, or counties. If you measure water by volume, apply a conversion factor to determine the correct weight.

With each dilution, submit:

1. Weight ratio of water to bituminous material in the original asphaltic emulsion
2. Weight of asphaltic emulsion before diluting
3. Weight of added water
4. Final dilution weight ratio of water to asphaltic emulsion

Apply to vertical surfaces with a residual tack coat rate that will thoroughly coat the vertical face without running off.

If you request and the Engineer authorizes, you may:

1. Change tack coat rates
2. Omit tack coat between layers of new HMA during the same work shift if:
   2.1. No dust, dirt, or extraneous material is present
   2.2. Surface is at least 140 degrees F

Immediately in advance of placing HMA, apply additional tack coat to damaged areas or where loose or extraneous material is removed.

Close areas receiving tack coat to traffic. Do not track tack coat onto pavement surfaces beyond the job site.

Asphalt binder tack coat temperature must be from 285 to 350 degrees F when applied.

39-1.03C(7) Geosynthetic Pavement Interlayer

If specified, place geosynthetic pavement interlayer over a coat of asphalt binder. Place geosynthetic pavement interlayer in compliance with the manufacturer's instructions.

Before placing the geosynthetic pavement interlayer and asphalt binder:

1. Repair cracks 1/4 inch and wider, spalls, and holes in the pavement. Repairing cracks is change order work.
2. Clean the pavement of loose and extraneous material.

Immediately before placing the interlayer, apply 0.25 ± 0.03 gallon of asphalt binder per square yard of interlayer or until the fabric is saturated. Apply asphalt binder the width of the geosynthetic pavement interlayer plus 3 inches on each side. At an interlayer overlap, apply asphalt binder on the lower interlayer the same overlap distance as the upper interlayer.

Align and place the interlayer with no overlapping wrinkles, except a wrinkle that overlaps may remain if it is less than 1/2 inch thick. If the overlapping wrinkle is more than 1/2 inch thick, cut the wrinkle out and overlap the interlayer no more than 2 inches.

The minimum HMA thickness over the interlayer must be 0.12 foot thick including conform tapers. Do not place the interlayer on a wet or frozen surface.

Overlap the interlayer borders between 2 to 4 inches. In the direction of paving, overlap the following roll with the preceding roll at any break.

You may use rolling equipment to correct distortions or wrinkles in the interlayer.

If asphalt binder tracked onto the interlayer or brought to the surface by construction equipment causes interlayer displacement, cover it with a small quantity of HMA.

Before placing HMA on the interlayer, do not expose the interlayer to:

1. Traffic except for crossings under traffic control and only after you place a small HMA quantity
2. Sharp turns from construction equipment
3. Damaging elements

Pave HMA on the interlayer during the same work shift.

39-1.03D Longitudinal Joints

39-1.03D(1) General

Longitudinal joints in the top layer must match lane lines. Alternate the longitudinal joint offsets in the lower layers at least 0.5 foot from each side of the lane line. You may request other longitudinal joint placement patterns.

A vertical longitudinal joint of more than 0.15 foot is not allowed at any time between adjacent lanes open to traffic.

For HMA thickness of 0.15 foot or less, the distance between the ends of the adjacent surfaced lanes at the end of each day's work must not be greater than can be completed in the following day of normal paving.

For HMA thickness greater than 0.15 foot, you must place HMA on adjacent traveled way lanes or shoulder so that at the end of each work shift the distance between the ends of HMA layers on adjacent lanes is from 5 to 10 feet. Place additional HMA along the transverse edge at each lane's end and along the exposed longitudinal edges between adjacent lanes. Hand rake and compact the additional HMA to form temporary conforms. You may place kraft paper or other authorized release agent under the conform tapers to facilitate the taper removal when paving activities resume.

If placing HMA against the edge of existing pavement, sawcut or grind the pavement straight and vertical along the joint and remove extraneous material.

39-1.03D(2) Tapered Notched Wedge

For divided highways with an HMA lift thickness greater than 0.15 foot, you may construct a 1-foot wide tapered notched wedge joint as a longitudinal joint between adjacent lanes open to traffic. A vertical notch of 0.75 inch maximum must be placed at the top and bottom of the tapered wedge.

The tapered notched wedge must retain its shape while exposed to traffic. Pave the adjacent lane within 1 day.

Construct the tapered portion of the tapered notched wedge with an authorized strike-off device. The strike-off device must provide a uniform slope and must not restrict the main screed of the paver.

You may use a device attached to the screed to construct longitudinal joints that will form a tapered notched wedge in a single pass. The tapered notched wedge must be compacted to a minimum of 91 percent compaction.

39-1.03E Edge Treatments

Construct edge treatment on the HMA pavement as shown.

Where a safety edge is required, use the same type of HMA used for the adjacent lane or shoulder.

The edge of roadway where the safety edge treatment is to be placed must have a solid base, free of debris such as loose material, grass, weeds, or mud. Grade areas to receive the safety edge as required.

The safety edge treatment must be placed monolithic with the adjacent lane or shoulder and shaped and compacted with a device attached to the paver.

The device must be capable of shaping and compacting HMA to the required cross section as shown. Compaction must be by constraining the HMA to reduce the cross sectional area by 10 to 15 percent. The device must produce a uniform surface texture without tearing, shoving, or gouging and must not leave marks such as ridges and indentations. The device must be capable of transition to cross roads, driveways, and obstructions.

For safety edge treatment, the angle of the slope must not deviate by more than ±5 degrees from the angle shown. Measure the angle from the plane of the adjacent finished pavement surface.

If paving is done in multiple lifts, the safety edge treatment must be placed with each lift.

Short sections of hand work are allowed to construct transitions for safety edge treatment.

39-1.03F Widening Existing Pavement

If widening existing pavement, construct new pavement structure to match the elevation of the existing pavement's edge before placing HMA over the existing pavement.

39-1.03G Shoulders, Medians, and Other Road Connections

Until the adjoining through lane's top layer has been paved, do not pave the top layer of:

1. Shoulders
2. Tapers
3. Transitions
4. Road connections
5. Driveways
6. Curve widenings
7. Chain control lanes
8. Turnouts
9. Turn pockets

If the number of lanes changes, pave each through lane's top layer before paving a tapering lane's top layer. Simultaneous to paving a through lane's top layer, you may pave an adjoining area's top layer, including shoulders. Do not operate spreading equipment on any area's top layer until completing final compaction.

If shoulders or median borders are shown, pave shoulders and median borders adjacent to the lane before opening a lane to traffic.

If shoulder conform tapers are shown, place conform tapers concurrently with the adjacent lane's paving.

If a driveway or a road connection is shown, place additional HMA along the pavement's edge to conform to road connections and driveways. Hand rake, if necessary, and compact the additional HMA to form a smooth conform taper.

39-1.03H Leveling

Section 39-1.03H applies if a bid item for hot mix asphalt (leveling), is shown on the Bid Item List.

Fill and level irregularities and ruts with HMA before spreading HMA over the base, existing surfaces, or bridge decks. You may use mechanical equipment other than a paver for these areas. The equipment must produce uniform smoothness and texture. HMA used to change an existing surface's cross slope or profile is not paid for as hot mix asphalt (leveling).

39-1.03I Miscellaneous Areas and Dikes

Prepare the area to receive HMA for miscellaneous areas and dikes, including excavation and backfill as needed.

Spread miscellaneous areas in 1 layer and compact to the specified lines and grades.

In median areas adjacent to slotted median drains, each layer of HMA must not exceed 0.20 foot maximum compacted thickness.

The finished surface must be:

1. Textured uniformly
2. Compacted firmly
3. Without depressions, humps, and irregularities

39-1.03J Replace Asphalt Concrete Surfacing

Where replace asphalt concrete surfacing is shown, remove existing asphalt concrete surfacing and replace with HMA. The Engineer determines the exact limits of asphalt concrete surfacing to be replaced.

Replace asphalt concrete in a lane before the lane is specified to be opened to traffic.

Before removing asphalt concrete, outline the replacement area and cut neat lines with a saw or grind to full depth of the existing asphalt concrete. Do not damage asphalt concrete and base remaining in place.

If the base is excavated beyond the specified plane, replace it with HMA. The Department does not pay for this HMA.

Do not use a material transfer vehicle if replace asphalt concrete surfacing is specified.

39-1.03K–39-1.03N Reserved

39-1.03O Compaction

39-1.03O(1) General

Rolling must leave the completed surface compacted and smooth without tearing, cracking, or shoving.

If a vibratory roller is used as a finish roller, turn the vibrator off.

Do not open new HMA pavement to traffic until the surface temperature is below 130 degrees F.

If the surface to be paved is both in sunlight and shade, pavement surface temperatures are taken in the shade.

39-1.03O(2) Method Compaction

Use method compaction for any of the following conditions:

1. Specified paved thickness is less than 0.15 foot
2. Specified paved thickness is less than 0.20 foot and a 3/4-inch aggregate gradation is specified and used
3. Specified paved thickness is less than 0.25 foot and a 1-inch aggregate gradation is specified and used

4. Replace asphalt concrete surfacing
5. Leveling courses
6. Areas the Engineer determines conventional compaction and compaction measurement methods are impeded

HMA compaction coverage is the number of passes needed to cover the paving width. A pass is 1 roller's movement parallel to the paving in either direction. Overlapping passes are part of the coverage being made and are not a subsequent coverage. Do not start a coverage until completing the prior coverage.

Method compaction must consist of performing:

1. Breakdown compaction of each layer with 3 coverages using a vibratory roller. The speed of the vibratory roller in miles per hour must not exceed the vibrations per minute divided by 1,000. If the HMA layer thickness is less than 0.08 foot, turn the vibrator off.
2. Intermediate compaction of each layer of HMA with 3 coverages using a pneumatic-tired roller at a speed not to exceed 5 mph.
3. Finish compaction of HMA with 1 coverage using a steel-tired roller.

Start rolling at the lower edge and progress toward the highest part.

The Engineer may order fewer coverages if the layer thickness of HMA is less than 0.15 foot.

39-1.03O(3)–39-1.03O(5) Reserved

39-1.03P Smoothness Corrections

If the final surface of the pavement does not comply with the smoothness specifications, grind the pavement to within specified tolerances, remove and replace it, or place an overlay of HMA. Do not start corrective work until your method is authorized.

Do not use equipment with carbide cutting teeth to grind the pavement unless authorized.

Smoothness correction of the final pavement surface must leave at least 75 percent of the specified HMA thickness. If ordered, core the pavement at the locations determined by the Engineer. Coring, including traffic control, is change order work. Remove and replace deficient pavement areas where the overlay thickness is less than 75 percent of the thickness specified as determined by the Engineer.

Corrected HMA pavement areas must be uniform rectangles with edges:

1. Parallel to the nearest HMA pavement edge or lane line
2. Perpendicular to the pavement centerline

On ground areas not to be overlaid with OGFC, apply fog seal coat under section 37-2.

Where corrections are made within areas requiring testing with inertial profiler, reprofile the entire lane length with the inertial profiler device.

Where corrections are made within areas requiring testing with a 12-foot straightedge, retest the corrected area with the straightedge.

39-1.03Q Data Cores

Section 39-1.03Q applies if a bid item for data core is shown on the Bid Item List.

Take data cores of the completed HMA pavement, underlying base, and subbase material. Notify the Engineer 3 business days before coring.

Protect data cores and surrounding pavement from damage.

Take 4-inch or 6-inch diameter data cores:

1. At the beginning, end, and every 1/2 mile within the paving limits of each route on the project
2. After all paving is complete
3. From the center of the specified lane

On a 2-lane roadway, take data cores from either lane. On a 4-lane roadway, take data cores from each direction in the outermost lane. On a roadway with more than 4 lanes, take data cores from the median lane and the outermost lane in each direction.

Each core must include the stabilized materials encountered. You may choose not to recover unstabilized material but you must identify the material. Unstabilized material includes:

1. Granular material
2. Crumbled or cracked stabilized material
3. Sandy or clayey soil

After data core summary and photograph submittal, dispose of cores.

39-1.04 PAYMENT

Geosynthetic pavement interlayer is measured by the square yard for the actual pavement area covered.

If tack coat, asphalt binder, and asphaltic emulsion are paid as separate bid items, their bid items are measured under section 92 or section 94.

The Department does not adjust the unit price for an increase or decrease in the tack coat quantity.

HMA of the type shown in the Bid Item List is measured based on the combined mixture weight. If recorded batch weights are printed automatically, the bid item for HMA is measured by using the printed batch weights, provided:

1. Total aggregate and supplemental fine aggregate weight per batch is printed. If supplemental fine aggregate is weighed cumulatively with the aggregate, the total aggregate batch weight must include the supplemental fine aggregate weight.
2. Total asphalt binder weight per batch is printed.
3. Each truckload's zero tolerance weight is printed before weighing the first batch and after weighing the last batch.
4. Time, date, mix number, load number and truck identification is correlated with a load slip.
5. Copy of the recorded batch weights is certified by a licensed weigh master and submitted.

Place hot mix asphalt dike of the type shown in the Bid Item List is measured along the completed length. Payment for the HMA used to construct the dike is not included in the payment for place hot mix asphalt dike.

Place hot mix asphalt (miscellaneous areas) is measured as the in-place compacted area. Payment for the HMA used for miscellaneous areas is not included in the payment for place hot mix asphalt (miscellaneous areas).

If replace asphalt concrete surfacing is shown, the bid item for replace asphalt concrete is measured based on the specified dimensions and any adjustments ordered.

The Department does not adjust the unit price for an increase or decrease in the prepaving grinding day quantity.

The Department reduces payment for noncompliance of HMA density based on the factors shown in the following table:

Reduced Payment Factors for Percent of Maximum Theoretical Density

HMA percent of maximum theoretical density	Reduced payment factor	HMA percent of maximum theoretical density	Reduced payment factor
91.0	0.0000	97.0	0.0000
90.9	0.0125	97.1	0.0125
90.8	0.0250	97.2	0.0250
90.7	0.0375	97.3	0.0375
90.6	0.0500	97.4	0.0500
90.5	0.0625	97.5	0.0625
90.4	0.0750	97.6	0.0750
90.3	0.0875	97.7	0.0875
90.2	0.1000	97.8	0.1000
90.1	0.1125	97.9	0.1125
90.0	0.1250	98.0	0.1250
89.9	0.1375	98.1	0.1375
89.8	0.1500	98.2	0.1500
89.7	0.1625	98.3	0.1625
89.6	0.1750	98.4	0.1750
89.5	0.1875	98.5	0.1875
89.4	0.2000	98.6	0.2000
89.3	0.2125	98.7	0.2125
89.2	0.2250	98.8	0.2250
89.1	0.2375	98.9	0.2375
89.0	0.2500	99.0	0.2500
< 89.0	Remove and replace	> 99.0	Remove and replace

39-2 TYPE A HOT MIX ASPHALT

39-2.01 GENERAL

39-2.01A Summary

Section 39-2 includes specifications for producing and placing Type A hot mix asphalt.

You may produce Type A HMA using an authorized warm mix asphalt technology.

39-2.01B Definitions

Reserved

39-2.01C Submittals

39-2.01C(1) General

Reserved

39-2.01C(2) Job Mix Formula

The JMF must be based on an HMA mix design determined as described in the Superpave Mix Design SP-2 Manual by the Asphalt Institute.

39-2.01C(3) Reclaimed Asphalt Pavement

Submit QC test results for RAP gradation with the combined aggregate gradation within 2 business days of taking RAP samples during HMA production.

39-2.01C(4)–39-2.01C(6) Reserved

39-2.01D Quality Control and Assurance

39-2.01D(1) General

Reserved

39-2.01D(2) Quality Control

39-2.01D(2)(a) General

Reserved

39-2.01D(2)(b) Aggregate

Test the quality characteristics of aggregate under the test methods and frequencies shown in the following table:

Aggregate Testing Frequencies

Quality characteristic	Test method	Minimum testing frequency
Gradation[a]	AASHTO T 27	1 per 750 tons and any remaining part
Sand equivalent[b, c]	AASHTO T 176	
Moisture content[d]	AASHTO T 329	
Crushed particles	AASHTO T 335	1 per 10,000 tons or 2 per project whichever is greater
Los Angeles rattler	AASHTO T 96	
Flat and elongated particles	ASTM D4791	
Fine aggregate angularity	AASHTO T 304 Method A	

[a]If RAP is used, test the combined aggregate gradation under California Test 384.
[b]Reported value must be the average of 3 tests from a single sample.
[c]Use of a sand reading indicator is required as shown in AASHTO T 176, Figure 1. Sections 4.7, 4.8, 7.1.2, 8.4.2 and 8.4.3 do not apply.
[d]Test at continuous mixing plants only. If RAP is used, test the RAP moisture content at continuous mixing plant and batch mixing plant.

For lime treated aggregate, test aggregate before treatment and test for gradation and moisture content during HMA production.

39-2.01D(2)(c) Reclaimed Asphalt Pavement

Sample and test processed RAP at a minimum frequency of 1 sample per 1000 tons with a minimum of 6 samples per fractionated stockpile. If a fractionated RAP stockpile is augmented, sample and test processed RAP quality characteristics at a minimum frequency of 1 sample per 500 tons of augmented RAP.

The combined RAP sample when tested under AASHTO T 164 must be within ±2.0 percent of the average asphalt binder content reported on page 4 of your Contractor Hot Mix Asphalt Design Data form. If new fractionated RAP stockpiles are required, the average binder content of the new fractionated RAP stockpile must be within ±2.0 percent of the average binder reported on page 4 of your Contractor Hot Mix Asphalt Design Data form.

The combined RAP sample when tested under AASHTO T 209 must be within ±0.06 of the average maximum specific gravity reported on page 4 of your Contractor Hot Mix Asphalt Design Data form.

During HMA production, sample RAP twice daily and perform QC testing for:

1. Aggregate gradation at least once a day under California Test 384
2. Moisture content at least twice a day

39-2.01D(2)(d) Hot Mix Asphalt Production

Test the quality characteristics of HMA under the test methods and frequencies shown in the following table:

Hot Mix Asphalt Testing Frequencies

Quality characteristic	Test method	Minimum testing frequency
Asphalt binder content	AASHTO T 308 Method A	1 per 750 tons and any remaining part
HMA moisture content	AASHTO T 329	1 per 2,500 tons but not less than 1 per paving day
Air voids content	AASHTO T 269	1 per 4,000 tons or 2 every 5 paving days, whichever is greater
Voids in mineral aggregate	SP-2 Asphalt Mixture Volumetrics	1 per 10,000 tons or 2 per project whichever is greater
Dust proportion	SP-2 Asphalt Mixture Volumetrics	
Density of core	California Test 375	2 per paving day
Nuclear gauge density	California Test 375	3 per 250 tons or 3 per paving day, whichever is greater
Hamburg wheel track	AASHTO T 324 (Modified)	1 per 10,000 tons or 1 per project, whichever is greater
Moisture susceptibility	AASHTO T 283	

39-2.01D(3)–39-2.01D(4) Reserved

39-2.01D(5) Department Acceptance

The Department accepts Type A HMA based on compliance with:

1. Aggregate quality requirements shown in the following table:

Aggregate Quality

Quality characteristic	Test method	Requirement
Aggregate gradation[a]	AASHTO T 27	JMF ± Tolerance
Percent of crushed particles Coarse aggregate (min, %) One-fractured face Two-fractured faces Fine aggregate (min, %) (Passing No. 4 sieve and retained on No. 8 sieve.) One fractured face	AASHTO T 335	 95 90 70
Los Angeles Rattler (max, %) Loss at 100 Rev. Loss at 500 Rev.	AASHTO T 96	 12 40
Sand equivalent (min.)[b, c]	AASHTO T 176	47
Flat and elongated particles (max, % by weight at 5:1)	ASTM D4791	10
Fine aggregate angularity (min, %)[d]	AASHTO T 304 Method A	45

[a]The Engineer determines combined aggregate gradations containing RAP under California Test 384.
[b]Reported value must be the average of 3 tests from a single sample.
[c]Use of a sand reading indicator is required as shown in AASHTO T 176, Figure 1. Sections 4.7, 4.8, 7.1.2, 8.4.2 and 8.4.3 do not apply.
[d] The Engineer waives this specification if HMA contains 10 percent or less of nonmanufactured sand by weight of total aggregate. Manufactured sand is fine aggregate produced by crushing rock or gravel.

2. If RAP is used, RAP quality requirements shown in the following table:

Reclaimed Asphalt Pavement Quality

Quality characteristic	Test method	Requirement
Binder content (% within the average value reported)	AASHTO T 164	±2.0
Specific gravity (within the average value reported)	AASHTO T 209	±0.06

3. In-place HMA quality requirements shown in the following table:

Type A HMA Acceptance In Place

Quality characteristic	Test method	Requirement
Asphalt binder content (%)	AASHTO T 308 Method A	JMF -0.3, +0.5
HMA moisture content (max, %)	AASHTO T 329	1
Air voids content at N_{design} (%)[a, b]	AASHTO T 269	4 ± 1.5
Voids in mineral aggregate on plant-produced HMA (min, %)[a]	SP-2 Asphalt Mixture Volumetrics[c]	
Gradation:		
No. 4		15.5–18.5
3/8-inch		14.5–17.5
1/2-inch		13.5–16.5
3/4-inch		12.5–15.5
1-inch		
with NMAS[g] = 1-inch		12.5–15.5
with NMAS[g] = 3/4-inch		13.5–16.5
Dust proportion	SP-2 Asphalt Mixture Volumetrics	0.6–1.3
Density of core (% of max theoretical density)[e, f]	California Test 375	91–97
Hamburg wheel track (min number of passes at 0.5-inch rut depth)	AASHTO T 324 (Modified)	
Binder grade:		
PG 58		10,000
PG 64		15,000
PG 70		20,000
PG 76 or higher		25,000
Hamburg wheel track (min number of passes at inflection point)	AASHTO T 324 (Modified)	
Binder grade:		
PG 58		10,000
PG 64		10,000
PG 70		12,500
PG 76 or higher		15,000
Moisture susceptibility (min, psi, dry strength)	AASHTO T 283	100
Moisture susceptibility (min, psi, wet strength)	AASHTO T 283	70

[a]Prepare 3 briquettes. Report the average of 3 tests.
[b]The Engineer determines the bulk specific gravity of each lab-compacted briquette under AASHTO T 275, Method A, and theoretical maximum specific gravity under AASHTO T 209, Method A.
[c]Determine bulk specific gravity under AASHTO T 275, Method A.
[d]The Engineer determines the laboratory-prepared HMA value for mix design verification only.
[e]The Engineer determines percent of theoretical maximum density under California Test 375 except the Engineer uses:

1. AASHTO T 275 to determine in-place density of each density core
2. AASHTO T 209, Method A to determine theoretical maximum density instead of calculating test maximum density

[f]The Engineer determines theoretical maximum density under AASHTO T 209, Method A, at the frequency specified in California Test 375, Part 5. D.
[g]NMAS means nominal maximum aggregate size.

39-2.02 MATERIALS

39-2.02A General

Reserved

39-2.02B Mix Design

The mix design must comply with the requirements shown in the following table:

Type A HMA Mix Design Requirements

Quality characteristic	Test method	Requirement
Air voids content (%)	AASHTO T 269[a]	$N_{initial}$ > 8.0 N_{design} = 4.0 (N_{design} = 5.0 for 1-inch aggregate) N_{max} > 2.0
Gyration compaction (no. of gyrations)	AASHTO T 312	$N_{initial}$ = 8 N_{design} = 85.0 N_{max} = 130
Voids in mineral aggregate (min, %)[b] Gradation: No. 4 3/8-inch 1/2-inch 3/4-inch 1-inch with NMAS[e] = 1-inch with NMAS[e] = 3/4-inch	SP-2 Asphalt Mixture Volumetrics	 16.5–19.5 15.5–18.5 14.5–17.5 13.5–16.5 13.5–16.5 14.5–17.5
Dust proportion	SP-2 Asphalt Mixture Volumetrics	0.6–1.3
Hamburg wheel track (min number of passes at 0.5-inch rut depth) Binder grade: PG 58 PG 64 PG 70 PG 76 or higher	AASHTO T 324 (Modified)[c]	 10,000 15,000 20,000 25,000
Hamburg wheel track (min number of passes at the inflection point) Binder grade: PG 58 PG 64 PG 70 PG 76 or higher	AASHTO T 324 (Modified)[c]	 10,000 10,000 12,500 15,000
Moisture susceptibility, dry strength (min, psi)	AASHTO T 283[c]	100
Moisture susceptibility, wet strength (min, psi)	AASHTO T 283[c, d]	70

[a]Calculate the air voids content of each specimen using AASHTO T 275, Method A, to determine bulk specific gravity. Use AASHTO T 209, Method A, to determine theoretical maximum specific gravity. Use a digital manometer and pycnometer when performing AASHTO T 209.
[b]Measure bulk specific gravity using AASHTO T 275, Method A.
[c]Test plant produced HMA.
[d]Freeze thaw required.
[e]NMAS means nominal maximum aggregate size.

For HMA mixtures using RAP, the maximum binder replacement is 25.0 percent for surface course and 40.0 percent for lower courses.

For HMA with a binder replacement percent less than or equal to 25 percent of your specified OBC, you may request that the performance graded asphalt binder grade with upper and lower temperature classifications be reduced by 6 degrees C from the specified grade.

For HMA with a binder replacement greater than 25 percent of your specified OBC and less than or equal to 40 percent of OBC, you must use a performance graded asphalt binder grade with upper and lower temperature classifications reduced by 6 degrees C from the specified grade.

39-2.02C Asphalt Binder

Reserved

39-2.02D Aggregates

39-2.02D(1) General

Before the addition of asphalt binder and lime treatment, the aggregate must comply with the requirements shown in the following table:

Aggregate Quality

Quality characteristic	Test method	Requirement
Percent of crushed particles Coarse aggregate (min, %) One-fractured face Two-fractured faces Fine aggregate (min, %) (Passing No. 4 sieve and retained on No. 8 sieve.) One fractured face	AASHTO T 335	 95 90 70
Los Angeles Rattler (max, %) Loss at 100 Rev. Loss at 500 Rev.	AASHTO T 96	 12 40
Sand equivalent (min)[a, b]	AASHTO T 176	47
Flat and elongated particles (max, % by weight at 5:1)	ASTM D4791	10
Fine aggregate angularity (min, %)[c]	AASHTO T 304 Method A	45

[a]Reported value must be the average of 3 tests from a single sample.
[b]Use of a Sand Reader Indicator is required as shown in AASHTO T 176, Figure 1. Sections 4.7, 4.8, 7.1.2, 8.4.2 and 8.4.3 do not apply.
[c] The Engineer waives this specification if HMA contains 10 percent or less of nonmanufactured sand by weight of total aggregate, except if your JMF fails verification. Manufactured sand is fine aggregate produced by crushing rock or gravel.

39-2.02D(2) Aggregate Gradations

The aggregate gradations for Type A HMA must comply with the requirements shown in the following table:

Aggregate Gradation Requirements

Type A HMA pavement thickness	Gradation
0.10 foot	3/8 inch
Greater than 0.10 to less than 0.20 foot	1/2 inch
0.20 foot and greater	3/4 inch
0.30 foot or greater	1 inch

Aggregate gradation must be within the target value limits for the specified sieve size shown in the following tables:

Aggregate Gradations
(Percentage Passing)

1-inch

Sieve size	Target value limit	Allowable tolerance
1"	100	--
3/4"	88–93	TV ± 5
1/2"	72–85	TV ± 6
3/8"	55–70	TV ± 6
No. 4	35–52	TV ± 7
No. 8	22–40	TV ± 5
No. 30	8–24	TV ± 4
No. 50	5–18	TV ± 4
No. 200	3–7	TV ± 2

3/4-inch

Sieve size	Target value limit	Allowable tolerance
1"	100	--
3/4"	90–98	TV ± 5
1/2"	70–90	TV ± 6
No. 4	42–58	TV ± 5
No. 8	29–43	TV ± 5
No. 30	10–23	TV ± 4
No. 200	2–7	TV ± 2

1/2-inch

Sieve sizes	Target value limit	Allowable tolerance
3/4"	100	--
1/2"	95–98	TV ± 5
3/8"	72–95	TV ± 5
No. 4	52–69	TV ± 5
No. 8	35–55	TV ± 5
No. 30	15–30	TV ± 4
No. 200	2–8	TV ± 2

3/8-inch

Sieve sizes	Target value limits	Allowable tolerance
1/2"	100	--
3/8"	95–98	TV ± 5
No. 4	55–75	TV ± 5
No. 8	30–50	TV ± 5
No. 30	15–35	TV ± 5
No. 200	2–9	TV ± 2

No. 4

Sieve sizes	Target value limits	Allowable tolerance
3/8"	100	--
No. 4	95–98	TV ± 5
No. 8	70–80	TV ± 6
No. 30	34–45	TV ± 5
No. 200	2–12	TV ± 4

39-2.02E Reclaimed Asphalt Pavement

You may substitute RAP for part of the virgin aggregate in a quantity up to a maximum of 25 percent of the aggregate blend.

Provide enough space for meeting all RAP handling requirements at your facility. Provide a clean, graded base, well drained area for stockpiles.

If RAP is from multiple sources, blend the RAP thoroughly and completely before fractionating.

For RAP substitution of 15 percent or less, fractionation is not required.

For RAP substitution greater than 15 percent, fractionate RAP stockpiles into 2 sizes, a coarse fraction RAP retained on 3/8-inch sieve, and a fine fraction RAP passing 3/8-inch sieve.

The RAP fractionation must comply with the requirements shown in the following table:

RAP Stockpile Fractionation Gradation Requirements

Quality characteristic	Test method	Requirement
Coarse (% passing the 1-inch sieve)	California Test 202[a]	100
Fine (% passing the 3/8-inch sieve)	California Test 202[a]	98–100

[a]Maximum mechanical shaking time is 10 minutes

You may use the coarse fractionated stockpile, the fine fractionated stockpile, or a combination of the coarse and fine fractionated stockpiles.

Isolate the processed RAP stockpiles from other materials. Store processed RAP in conical or longitudinal stockpiles. Processed RAP must not be agglomerated or be allowed to congeal in large stockpiles.

39-2.02F Hot Mix Asphalt Production

If RAP is used, the asphalt binder set point target value for HMA with RAP must be:

$$\text{Asphalt Binder Set Point Target Value} = \frac{\dfrac{BC_{OBC}}{\left(1 - \dfrac{BC_{OBC}}{100}\right)} - R_{RAP}\left[\dfrac{BC_{RAP}}{\left(1 - \dfrac{BC_{RAP}}{100}\right)}\right]}{100 + \dfrac{BC_{OBC}}{\left(1 - \dfrac{BC_{OBC}}{100}\right)}} \times 100$$

Where:
BC_{OBC} = optimum asphalt binder content, percent based on total weight of mix
R_{RAP} = RAP ratio by weight of aggregate
BC_{RAP} = asphalt binder content of RAP, percent based on total weight of RAP mix

39-2.03 CONSTRUCTION

Spread Type A HMA at the atmospheric and surface temperatures shown in the following table:

Minimum Atmospheric and Surface Temperatures for Type A HMA

Compacted layer thickness, feet	Atmospheric,°F		Surface,°F	
	Unmodified asphalt binder	Modified asphalt binder	Unmodified asphalt binder	Modified asphalt binder
< 0.15	55	50	60	55
0.15 – 0.25	45	45	50	50

For Type A HMA placed under method compaction, if the asphalt binder is:

1. Unmodified, complete:

1.1. 1st coverage of breakdown compaction before the surface temperature drops below 250 degrees F
1.2. Breakdown and intermediate compaction before the surface temperature drops below 190 degrees F
1.3. Finish compaction before the surface temperature drops below 150 degrees F
2. Modified, complete:
2.1. 1st coverage of breakdown compaction before the surface temperature drops below 240 degrees F
2.2. Breakdown and intermediate compaction before the surface temperature drops below 180 degrees F
2.3. Finish compaction before the surface temperature drops below 140 degrees F

If you request and the Engineer authorizes, you may cool Type A HMA with water when rolling activities are complete. Apply water under section 17.

39-2.04 PAYMENT

Not Used

39-3 RUBBERIZED HOT MIX ASPHALT–GAP GRADED

39-3.01 GENERAL

39-3.01A Summary

Section 39-3 includes specifications for producing and placing rubberized hot mix asphalt–gap graded.

You may produce RHMA-G using a warm mix asphalt technology.

39-3.01B Definitions

Reserved

39-3.01C Submittals

39-3.01C(1) General

Reserved

39-3.01C(2) Job Mix Formula

With your proposed JMF include MSDS for:

1. Base asphalt binder
2. CRM and asphalt modifier
3. Blended asphalt rubber binder components

The JMF must be based on an HMA mix design determined as described in the Superpave Mix Design SP-2 Manual by the Asphalt Institute.

39-3.01C(3) Asphalt Rubber Binder

Submit a proposal for asphalt rubber binder design and profile. In the design, include the asphalt binder, asphalt modifier, and CRM and their proportions.

If you change asphalt rubber binder supplier or any component material used in asphalt rubber binder or its percentage, submit a new JMF.

For the asphalt rubber binder used, submit:

1. Log of production daily.
2. Certificate of compliance with test results for CRM and asphalt modifier with each truckload delivered to the HMA plant. The certificate of compliance for asphalt modifier must represent no more than 5,000 lb.
3. Submit certified weight slips for the CRM and asphalt modifier furnished.

Submit a certificate of compliance for the asphalt rubber binder. With the certificate of compliance, submit test results for CRM and asphalt modifier with each truckload delivered to the HMA plant. A certificate of compliance for asphalt modifier must not represent more than 5,000 lb.

39-3.01D Quality Control and Assurance

39-3.01D(1) General

Reserved

39-3.01D(2) Job Mix Formula Verification

If you request, the Engineer verifies RHMA-G quality requirements within 7 days of receiving all verification samples and after the JMF document submittal has been accepted.

39-3.01D(3) Quality Control

39-3.01D(3)(a) General

Reserved

39-3.01D(3)(b) Asphalt Rubber Binder

39-3.01D(3)(b)(i) General

The asphalt rubber binder blending plant must be authorized under the Department's Material Plant Quality Program.

Take asphalt rubber binder samples from the feed line connecting the asphalt rubber binder tank to the HMA plant. Use an AASHTO-certified laboratory for testing.

39-3.01D(3)(b)(ii) Asphalt Modifier

Test asphalt modifier under the test methods and frequencies shown in the following table:

Asphalt Modifier for Asphalt Rubber Binder

Quality characteristic	Test method	Frequency
Viscosity	ASTM D445	1 per shipment
Flash point	ASTM D92	
Molecular Analysis		
Asphaltenes	ASTM D2007	1 per shipment
Aromatics	ASTM D2007	

39-3.01D(3)(b)(iii) Crumb Rubber Modifier

Sample and test scrap tire CRM and high natural CRM separately. Test CRM under the test methods and frequencies shown in the following table:

Crumb Rubber Modifier for Asphalt Rubber Binder

Quality characteristic	Test method	Frequency
Scrap tire CRM gradation	California Test 385	1 per 10,000 lb
High natural CRM gradation	California Test 385	1 per 3,400 lb
Wire in CRM	California Test 385	1 per 10,000 lb
Fabric in CRM	California Test 385	
CRM particle length	--	
CRM specific gravity	California Test 208	
Natural rubber content in high natural CRM	ASTM D297	1 per 3,400 lb

Sample and test scrap tire CRM and high natural CRM separately.

39-3.01D(3)(b)(iv) Asphalt Rubber Binder

Test asphalt rubber binder under the test methods and frequencies shown in the following table:

Quality characteristic	Test method	Frequency
Cone penetration	ASTM D217	1 per lot
Resilience	ASTM D5329	
Softening point	ASTM D36	
Viscosity	ASTM D7741	1 per hour

39-3.01D(3)(c) Aggregate

Test the quality characteristics of aggregate under the test methods and frequencies shown in the following table:

Aggregate Testing Frequencies

Quality characteristic	Test method	Minimum testing frequency
Gradation	AASHTO T 27	1 per 750 tons and any remaining part
Sand equivalent[a, b]	AASHTO T 176	
Moisture content[c]	AASHTO T 329	
Crushed particles	AASHTO T 335	1 per 10,000 tons or 2 per project, whichever is greater
Los Angeles rattler	AASHTO T 96	
Flat and elongated particles	ASTM D4791	
Fine aggregate angularity	AASHTO T 304 Method A	

[a]Reported value must be the average of 3 tests from a single sample.
bUse of a sand reading indicator is required as shown in AASHTO T 176, Figure 1. Sections 4.7, 4.8, 7.1.2, 8.4.2 and 8.4.3 do not apply.
[c]Test at continuous mixing plants only

For lime treated aggregate, test aggregate before treatment and test for gradation and moisture content during RHMA-G production.

39-3.01D(3)(d) Hot Mix Asphalt Production

Test the quality characteristics of RHMA-G under the test methods and frequencies shown in the following table:

RHMA-G Mix Asphalt Testing Frequencies

Quality characteristic	Test method	Minimum testing frequency
Asphalt binder content	AASHTO T 308 Method A	1 per 750 tons and any remaining part
HMA moisture content	AASHTO T 329	1 per 2,500 tons but not less than 1 per paving day
Air voids content	AASHTO T 269	1 per 4,000 tons or 2 every 5 paving days, whichever is greater
Voids in mineral aggregate	SP-2 Asphalt Mixture Volumetrics	1 per 10,000 tons or 2 per project whichever is greater
Dust proportion	SP-2 Asphalt Mixture Volumetrics	
Density of core	California Test 375	2 per paving day
Nuclear gauge density	California Test 375	3 per 250 tons or 3 per paving day, whichever is greater
Hamburg wheel track	AASHTO T 324 (Modified)	1 per 10,000 tons or 1 per project, whichever is greater
Moisture susceptibility	AASHTO T 283	

39-3.01D(4) Reserved

39-3.01D(5) Department Acceptance

39-3.01D(5)(a) General

The Department accepts RHMA-G based on compliance with:

1. Aggregate quality requirements shown in the following table:

Aggregate Quality

Quality characteristic	Test method	Requirement
Aggregate gradation	AASHTO T 27	JMF ± Tolerance
Percent of crushed particles Coarse aggregate (min, %) One-fractured face Two-fractured faces Fine aggregate (min, %) (Passing No. 4 sieve and retained on No. 8 sieve.) One fractured face	AASHTO T 335	 -- 90 70
Los Angeles Rattler (max, %) Loss at 100 Rev. Loss at 500 Rev.	AASHTO T 96	 12 40
Sand equivalent (min)[a, b]	AASHTO T 176	47
Flat and elongated particles (max, % by weight at 5:1)	ASTM D4791	Report only
Fine aggregate angularity (min, %)[c]	AASHTO T 304 Method A	45

[a]Reported value must be the average of 3 tests from a single sample.
[b]Use of a sand reading Indicator is required as shown in AASHTO T 176, Figure 1. Sections 4.7, 4.8, 7.1.2, 8.4.2 and 8.4.3 do not apply.
[c] The Engineer waives this specification if RHMA-G contains 10 percent or less of nonmanufactured sand by weight of total aggregate. Manufactured sand is fine aggregate produced by crushing rock or gravel.

2. In-place RHMA-G quality requirements shown in the following table:

RHMA-G In-Place Acceptance

Quality characteristic	Test method	Requirement
Asphalt binder content (%)	AASHTO T 308 Method A	JMF -0.4, +0.5
HMA moisture content (max, %)	AASHTO T 329	1
Air voids content @ N_{design} (%)[a, b]	AASHTO T 269	4.0 ± 1.5
Voids in mineral aggregate on laboratory-produced HMA[d] (min, %) Gradation: 1/2-inch and 3/4-inch	SP-2 Asphalt Mixture Volumetrics[c]	 18.0–23.0
Voids in mineral aggregate on plant-produced HMA (min, %)[a] Gradation: 1/2-inch and 3/4-inch	SP-2 Asphalt Mixture Volumetrics[c]	 18.0–23.0
Dust proportion[a]	SP-2 Asphalt Mixture Volumetrics	Report only
Density of core (% of max theoretical density)[e, f]	California Test 375	91–97
Hamburg wheel track (min number of passes at 0.5-inch rut depth) Binder grade: PG 58 PG 64 PG 70	AASHTO T 324 (Modified)	 15,000 20,000 25,000
Hamburg wheel track (min number of passes at inflection point) Binder grade: PG 58 PG 64 PG 70	AASHTO T 324 (Modified)	 10,000 12,500 15,000
Moisture susceptibility (min, psi, dry strength)	AASHTO T 283	100
Moisture susceptibility (min, psi, wet strength)	AASHTO T 283	70

[a]Prepare 3 briquettes. Report the average of 3 tests.
[b]The Engineer determines the bulk specific gravity of each lab-compacted briquette under AASHTO T 275, Method A, and theoretical maximum specific gravity under AASHTO T 209, Method A.
[c]Determine bulk specific gravity under AASHTO T 275, Method A.
[d]The Engineer determines the laboratory-prepared RHMA-G value for mix design verification only.
[e]The Engineer determines percent of theoretical maximum density under California Test 375 except the Engineer uses:

1. AASHTO T 275, Method A, to determine in-place density of each density core instead of using the nuclear gauge
2. AASHTO T 209, Method A to determine theoretical maximum density instead of calculating test maximum density.

[f]The Engineer determines theoretical maximum density under AASHTO T 209, Method A, at the frequency specified in California Test 375, Part 5. D.

39-3.01D(5)(b) Asphalt Rubber Binder

39-3.01D(5)(b)(i) General

The Department does not use asphalt rubber binder design profile for production acceptance.

39-3.01D(5)(b)(ii) Asphalt Modifier

The Department accepts asphalt modifier based on compliance with the requirements shown in the following table:

Asphalt Modifier for Asphalt Rubber Binder

Quality characteristic	Test method	Requirement
Viscosity at 100 °C ($m^2/s \times 10^{-6}$)	ASTM D445	X ± 3[a]
Flash point (min, °C)	ASTM D92	207
Molecular Analysis		
Asphaltenes (max, % by mass (max)	ASTM D2007	0.1
Aromatics (min % by mass)	ASTM D2007	55

[a]The symbol "X" is the asphalt modifier viscosity.

39-3.01D(5)(b)(iii) Crumb Rubber Modifier

The Department accepts scrap tire CRM and high natural CRM based on compliance with the requirements shown in the following table:

Crumb Rubber Modifier for Asphalt Rubber Binder

Quality characteristic	Test method	Requirement
Scrap tire CRM gradation (% passing No. 8 sieve)	California Test 385	100
High natural CRM gradation (% passing No. 10 sieve)	California Test 385	100
Wire in CRM (max, %)	California Test 385	0.01
Fabric in CRM (max, %)	California Test 385	0.05
CRM particle length (max, in)	--	3/16
CRM specific gravity	California Test 208	1.1–1.2

Scrap tire CRM and high natural CRM are sampled and tested separately.

39-3.01D(5)(b)(iv) Asphalt Rubber Binder

The Department accepts asphalt rubber binder based on compliance with the requirements shown in the following table:

Quality characteristic	Test method	Requirement
Cone penetration at 25 °C (0.10 mm)	ASTM D217	25–70
Resilience at 25 °C (min, % rebound)	ASTM D5329	18
Softening point (°C)	ASTM D36	52–74
Viscosity at 190 °C (centipoises)	ASTM D7741	1,500–4,000

39-3.01D(5)(c)–39-3.01D(5)(f) Reserved

39-3.02 MATERIALS

39-3.02A General

Reserved

39-3.02B Mix Design

For RHMA-G, the mix design must comply with the requirements shown in the following table:

RHMA-G Mix Design Requirements

Quality characteristic	Test method	Requirement
Air voids content (%)	AASHTO T 269[a]	N_{design} = 4.0
Gyration compaction (no. of gyrations	AASHTO T 312	N_{design} = 50–150[b]
Voids in mineral aggregate (min, %)	SP-2 Asphalt Mixture Volumetrics[c]	18.0–23.0
Dust proportion	SP-2 Asphalt Mixture Volumetrics	Report only
Hamburg wheel track (min number of passes at 0.5-inch rut depth) Binder grade: PG 58 PG 64 PG 70	AASHTO T 324 (Modified)[d]	 15,000 20,000 25,000
Hamburg wheel track (min number of passes at the inflection point) Binder grade: PG 58 PG 64 PG 70	AASHTO T 324 (Modified)[d]	 10,000 10,000 12,500
Moisture susceptibility, dry strength (min, psi)	AASHTO T 283[d]	100
Moisture susceptibility, wet strength (min, psi)	AASHTO T 283[d, e]	70

[a]Calculate the air voids content of each specimen using AASHTO T 275, Method A, to determine bulk specific gravity and AASHTO T 209, Method A, to determine theoretical maximum specific gravity. Under AASHTO T 209 use a digital manometer and pycnometer when performing AASHTO T 209.
[b]Superpave gyratory compactor ram pressure may be increased to a maximum of 825kPa, and specimens may be held at a constant height for a maximum of 90 minutes.
[c]Measure bulk specific gravity using AASHTO T 275, Method A.
[d]Test plant produced HMA.
[e]Freeze thaw required.

Determine the amount of asphalt rubber binder to be mixed with the aggregate for RHMA-G as follows:

1. Base the calculations on the average of 3 briquettes produced at each asphalt rubber binder content.
2. Plot asphalt rubber binder content versus average air voids content for each set of 3 specimens and connect adjacent points with a best-fit curve.
3. Calculate voids in mineral aggregate for each specimen, average each set, and plot the average versus asphalt rubber binder content.
4. Calculate the dust proportion and plot versus asphalt rubber binder content.
5. From the curve plotted, select the theoretical asphalt rubber binder content at 4 percent air voids.
6. At the selected asphalt rubber binder content, calculate dust proportion.
7. Record the asphalt rubber binder content in the Contractor Hot Mix Asphalt Design Data Form as the OBC.

The OBC must not fall below 7.5 percent by total weight of the mix.

Laboratory mixing and compaction must comply with AASHTO R 35, except the mixing temperature of the aggregate must be between 300 and 325 degrees F. The mixing temperature of the asphalt rubber binder must be between 375 and 425 degrees F. The compaction temperature of the combined mixture must be between 290 and 320 degrees F.

39-3.02C Asphalt Rubber Binder

39-3.02C(1) General

Asphalt rubber binder must be a combination of:

1. Asphalt binder
2. Asphalt modifier
3. CRM

The combined asphalt binder and asphalt modifier must be 80.0 ± 2.0 percent by weight of the asphalt rubber binder.

39-3.02C(2) Asphalt Modifier

Asphalt modifier must be a resinous, high flash point, and aromatic hydrocarbon, and must comply with the requirements shown in the following table:

Asphalt Modifier for Asphalt Rubber Binder

Quality characteristic	Test method	Requirement
Viscosity at 100 °C ($m^2/s \times 10^{-6}$)	ASTM D445	X ± 3[a]
Flash point (min, °C)	ASTM D92	207
Molecular Analysis		
Asphaltenes (max, % by mass)	ASTM D2007	0.1
Aromatics (min, % by mass)	ASTM D2007	55

[a]The symbol "X" is the proposed asphalt modifier viscosity. "X" must be between 19 and 36. A change in "X" requires a new asphalt rubber binder design.

Asphalt modifier must be from 2.0 to 6.0 percent by weight of the asphalt binder in the asphalt rubber binder.

39-3.02C(3) Crumb Rubber Modifier

CRM must be a ground or granulated combination of scrap tire CRM and high natural CRM. CRM must be 75.0 ± 2.0 percent scrap tire CRM and 25.0 ± 2.0 percent high natural CRM by total weight of CRM. Scrap tire CRM must be from any combination of automobile tires, truck tires, or tire buffings.

The CRM must comply with the requirements shown in the following table:

Crumb Rubber Modifier for Asphalt Rubber Binder

Quality characteristic	Test method	Requirement
Scrap tire CRM gradation (% passing No. 8 sieve)	California Test 385	100
High natural CRM gradation (% passing No. 10 sieve)	California Test 385	100
Wire in CRM (max, %)	California Test 385	0.01
Fabric in CRM (max, %)	California Test 385	0.05
CRM particle length (max, in)[a]	--	3/16
CRM specific gravity	California Test 208	1.1–1.2
Natural rubber content in high natural CRM (%)	ASTM D297	40.0–48.0

[a]Test at mix design and for certificate of compliance.

CRM must be ground or granulated at ambient temperature. If steel and fiber are cryogenically separated, separation must occur before grinding or granulating. Cryogenically produced CRM particles must be ground or granulated and not pass through the grinder or granulator.

CRM must be dry, free-flowing particles that do not stick together. CRM must not cause foaming when combined with the asphalt binder and asphalt modifier. You may add calcium carbonate or talc up to 3 percent by weight of CRM.

39-3.02C(4) Design and Profile

Design the asphalt rubber binder from testing you perform for each quality characteristic and for the reaction temperatures expected during production. The profile must include the same component sources for the asphalt rubber binder used. The 24-hour (1,440-minute) interaction period determines the design profile. At a minimum, mix asphalt rubber binder components, take samples, and perform and record the tests shown in the following table:

Asphalt Rubber Binder Reaction Design Profile

Quality characteristic	Test Method	Minutes of reaction [a]							Limits
		45	60	90	120	240	360	1440	
Cone penetration at 25 °C (0.10 mm)	ASTM D217	X [b]				X		X	25–70
Resilience at 25 °C (min, % rebound)	ASTM D5329	X				X		X	18
Field softening point (°C)	ASTM D36	X				X		X	52–74
Viscosity (centipoises)	ASTM D7741	X	X	X	X	X	X	X	1,500–4,000

[a]Six hours (360 minutes) after CRM addition, reduce the oven temperature to 275 °F for 16 hours. After the 16-hour (960 minutes) cool down after CRM addition, reheat the binder to the reaction temperature expected during production for sampling and testing at 24 hours (1,440 minutes).
[b]"X" denotes required testing

39-3.02C(5) Asphalt Rubber Binder Production

39-3.02C(5)(a) General

Deliver scrap tire CRM and high natural CRM in separate bags.

39-3.02C(5)(b) Mixing

Proportion and mix asphalt binder, asphalt modifier, and CRM simultaneously or premix the asphalt binder and asphalt modifier before adding CRM. If you premix asphalt binder and asphalt modifier, mix them for at least 20 minutes. When you add CRM, the asphalt binder and asphalt modifier must be from 375 to 440 degrees F.

After interacting for at least 45 minutes, the quality characteristics of asphalt rubber binder must comply with the requirements shown in the following table:

Quality characteristic	Test method	Requirement
Cone penetration at 25 °C (0.10 mm)	ASTM D217	25–70
Resilience at 25 °C (min, % rebound)	ASTM D5329	18
Softening point (°C)	ASTM D36	52–74
Viscosity at 190 °C (centipoises)	ASTM D7741	1,500–4,000

Do not use asphalt rubber binder during the first 45 minutes of the reaction period. During this period, the asphalt rubber binder mixture must be between 375 degrees F and the lower of 425 or 25 degrees F below the asphalt binder's flash point indicated in the MSDS.

If any asphalt rubber binder is not used within 4 hours after the reaction period, discontinue heating. If the asphalt rubber binder drops below 375 degrees F, reheat before use. If you add more scrap tire CRM to the reheated asphalt rubber binder, the binder must undergo a 45-minute reaction period. The added scrap tire CRM must not exceed 10 percent of the total asphalt rubber binder weight. Reheated and reacted asphalt rubber binder must comply with the viscosity specifications. Do not reheat asphalt rubber binder more than twice.

39-3.02D Aggregates

39-3.02D(1) General

For RHMA-G, before the addition of asphalt binder and lime treatment, the aggregate must comply with the requirements shown in the following table:

Aggregate Quality

Quality characteristic	Test method	Requirement
Percent of crushed particles Coarse aggregate (min, %) One-fractured face Two-fractured faces Fine aggregate (min, %) (Passing No. 4 sieve and retained on No. 8 sieve.) One fractured face	AASHTO T 335	 -- 90 70
Los Angeles Rattler (max, %) Loss at 100 Rev. Loss at 500 Rev.	AASHTO T 96	 12 40
Sand equivalent (min)[a, b]	AASHTO T 176	47
Flat and elongated particles (max, % by weight at 5:1)	ASTM D4791	Report only
Fine aggregate angularity (min, %)[c]	AASHTO T 304 Method A	45

[a]Reported value must be the average of 3 tests from a single sample.
[b]Use of a sand reading indicator is required as shown in AASHTO T 176, Figure 1. Sections 4.7, 4.8, 7.1.2, 8.4.2 and 8.4.3 do not apply.
[c] The Engineer waives this specification if HMA contains 10 percent or less of nonmanufactured sand by weight of total aggregate, except if your JMF fails verification. Manufactured sand is fine aggregate produced by crushing rock or gravel.

39-3.02D(2) Aggregate Gradations

The aggregate gradations for RHMA-G must comply with the requirements shown in the following table:

Aggregate Gradation Requirements

Type A HMA pavement thickness	Gradation
0.10 to less than 0.20 foot	1/2 inch
0.20 foot and greater	3/4 inch

For RHMA-G, the aggregate gradations must be within the target value limits for the specified sieve size shown in the following tables:

Aggregate Gradation
(Percentage Passing)
Rubberized Hot Mix Asphalt - Gap Graded (RHMA-G)

3/4-inch RHMA-G

Sieve Sizes	Target Value Limits	Allowable Tolerance
1"	100	--
3/4"	95–98	TV ± 5
1/2"	83–87	TV ± 6
3/8"	65–70	TV ± 5
No. 4	28–42	TV ± 6
No. 8	14–22	TV ± 5
No. 200	0–6	TV ± 2

1/2-inch RHMA-G

Sieve Sizes	Target Value Limits	Allowable Tolerance
3/4"	100	--
1/2"	90–98	TV ± 6
3/8"	83–87	TV ± 5
No. 4	28–42	TV ± 6
No. 8	14–22	TV ± 5
No. 200	0–6	TV ± 2

39-3.02E Rubberized Hot Mix Asphalt Production

Asphalt rubber binder must be from 375 to 425 degrees F when mixed with aggregate.

If the dry and wet moisture susceptibility test result for treated plant-produced RHMA-G is less than the RHMA-G mix design requirement for dry and wet moisture susceptibility strength, the minimum dry and wet strength requirement is waived, but you must use one of the following treatments:

1. Aggregate lime treatment using the slurry method
2. Aggregate lime treatment using the dry lime method
3. Liquid antistrip treatment of HMA

39-3.03 CONSTRUCTION

Use a material transfer vehicle when placing RHMA-G.

Do not use a pneumatic tired roller to compact RHMA-G.

For RHMA-G placed under method compaction:

1. Only spread and compact if the atmospheric temperature is at least 55 degrees F and the surface temperature is at least 60 degrees F.
2. Complete the 1st coverage of breakdown compaction before the surface temperature drops below 285 degrees F.
3. Complete breakdown and intermediate compaction before the surface temperature drops below 250 degrees F.
4. Complete finish compaction before the surface temperature drops below 200 degrees F.
5. If the atmospheric temperature is below 70 degrees F, cover loads in trucks with tarps. The tarps must completely cover the exposed load until you transfer the mixture to the paver's hopper or to the pavement surface. Tarps are not required if the time from discharge to truck until transfer to the paver's hopper or the pavement surface is less than 30 minutes.

Spread sand at a rate between 1 and 2 lb/sq yd on new RHMA-G pavement when finish rolling is complete. Sand must be free of clay or organic matter. Sand must comply with section 90-1.02C(3). Keep traffic off the pavement until spreading sand is complete.

39-3.04 PAYMENT

Not Used

39-4 OPEN GRADED FRICTION COURSES

39-4.01 GENERAL

39-4.01A Summary

Section 39-4 includes specifications for producing and placing open graded friction courses. Open graded friction courses include HMA-O, RHMA-O, and RHMA-O-HB.

You may produce OGFC using a warm mix asphalt technology.

39-4.01B Definitions

Reserved

39-4.01C Submittals

Submit a complete JMF, except do not specify an asphalt binder content.

39-4.01D Quality Control and Assurance

39-4.01D(1) General

Reserved

39-4.01D(2) Quality Control

39-4.01D(2)(a) General

Reserved

39-4.01D(2)(b) Asphalt Rubber Binder

For RHMA-O and RHMA-O-HB, the asphalt rubber binder must comply with the specifications in 39-3.01D(2)(b).

39-4.01D(2)(c) Aggregate

Test the quality characteristics of aggregate under the test methods and frequencies shown in the following table:

Aggregate Testing Frequencies

Quality characteristic	Test method	Minimum testing frequency
Gradation	AASHTO T 27	1 per 750 tons and any remaining part
Moisture content[a]	AASHTO T 329	1 per 1500 tons and any remaining part
Crushed particles	AASHTO T 335	1 per 10,000 tons or 2 per project, whichever is greater
Los Angeles rattler	AASHTO T 96	
Flat and elongated particles	ASTM D4791	

[a]Test at continuous mixing plants only

For lime treated aggregate, test aggregate before treatment and test for gradation and moisture content during OGFC production.

39-4.01D(2)(d) Hot Mix Asphalt Production

Test the quality characteristics of OGFC under the test methods and frequencies shown in the following table:

OGFC Testing Frequencies

Quality characteristic	Test method	Minimum testing frequency
Asphalt binder content	AASHTO T 308 Method A	1 per 750 tons and any remaining part
HMA moisture content	AASHTO T 329	1 per 2,500 tons but not less than 1 per paving day

39-4.01D(3) Department Acceptance

39-4.01D(3)(a) General

The Department accepts OGFC based on compliance with:

1. Aggregate quality requirements shown in the following table:

Aggregate Quality

Quality characteristic	Test method	Requirement
Aggregate gradation	AASHTO T 27	JMF ± Tolerance
Percent of crushed particles Coarse aggregate (min, %) One-fractured face Two-fractured faces Fine aggregate (min, %) (Passing No. 4 sieve and retained on No. 8 sieve.) One fractured face	AASHTO T 335	 90 90 90
Los Angeles Rattler (max, %) Loss at 100 Rev. Loss at 500 Rev.	AASHTO T 96	 12 40
Flat and elongated particles (max, % by weight @ 5:1)	ASTM D4791	Report only

2. In-place OGFC quality requirements shown in the following table:

OGFC Acceptance In Place

Quality characteristic	Test method	Requirement
Asphalt binder content (%)	AASHTO T 308 Method A	JMF -0.4, +0.5
HMA moisture content (max, %)	AASHTO T 329	1

39-4.01D(3)(b) Asphalt Rubber Binder

The Department accepts asphalt rubber binder in RHMA-O and RHMA-O-HB under 39-3.01D(5)(b).

39-4.01D(3)(c) Pavement Smoothness

Pavement smoothness of OGFC must comply with the Mean Roughness Index requirements shown in the following table for a 0.1 mile section:

OGFC Pavement Smoothness Acceptance Criteria

OGFC placement on	Mean Roughness Index requirement
New construction or HMA overlay	60 in/mi or less
Existing pavement	75 in/mi or less
Milled surface	75 in/mi or less

39-4.01D(3)(d)–39-4.01D(3)(f) Reserved

39-4.02 MATERIALS

39-4.02A General

When mixed with asphalt binder, aggregate must not be more than 325 degrees F except aggregate for OGFC with unmodified asphalt binder must be not more than 275 degrees F.

39-4.02B Mix Design

The Department determines the asphalt binder content under California Test 368 within 20 days of your complete JMF submittal and provides you a Caltrans Hot Mix Asphalt Verification form.

For OGFC, the 1st paragraph of section 39-1.02B(1) does not apply.

39-4.02C Asphalt Binder

Asphalt rubber binder in RHMA-O and RHMA-O-HB must comply with section 39-3.02B.

39-4.02D Aggregate

39-4.02D(1) General

Aggregate must comply with the requirements shown in the following table:

Aggregate Quality

Quality characteristic	Test method	Requirement
Percent of crushed particles Coarse aggregate (min, %) One-fractured face Two-fractured faces Fine aggregate (min, %) (Passing No. 4 sieve and retained on No. 8 sieve.) One fractured face	AASHTO T 335	 -- 90 90
Los Angeles Rattler (max, %) Loss at 100 Rev. Loss at 500 Rev.	AASHTO T 96	 12 40
Flat and elongated particles (max, % by weight at 5:1)	ASTM D4791	Report only

39-4.02D(2) Aggregate Gradations

The aggregate gradations for HMA-O must comply with the requirements shown in the following table:

Aggregate Gradation Requirements

HMA-O pavement thickness	Gradation
Greater than 0.10 to less than 0.15 foot	1/2 inch
0.15 foot and greater	1 inch

The aggregate gradations for RHMA-O and RHMA-O-HB must comply with the requirements shown in the following table:

Aggregate Gradation Requirements

RHMA-O and RHMA-O-HB pavement thickness	Gradation
Greater than 0.10 foot	1/2 inch

For RHMA-O and RHMA-O-HB, the 1-inch aggregate gradation is not allowed.

For OGFC, the aggregate gradations must be within the target value limits for the specified sieve size shown in the following tables:

Aggregate Gradations
(Percentage Passing)
Open Graded Friction Course (OGFC)

1-inch OGFC

Sieve size	Target value limit	Allowable tolerance
1 1/2"	100	--
1"	99–100	TV ± 5
3/4"	85–96	TV ± 5
1/2"	55–71	TV ± 6
No. 4	10–25	TV ± 7
No. 8	6–16	TV ± 5
No. 200	0–6	TV ± 2

1/2-inch OGFC

Sieve size	Target value limit	Allowable tolerance
3/4"	100	--
1/2"	95–100	TV ± 6
3/8"	78–89	TV ± 6
No. 4	28–37	TV ± 7
No. 8	7–18	TV ± 5
No. 30	0–10	TV ± 4
No. 200	0–3	TV ± 2

If lime treatment is required, you may reduce the lime ratio for the combined aggregate from 1.0 to 0.5 percent for OGFC.

39-4.03 CONSTRUCTION

Use a material transfer vehicle when placing OGFC.

If the atmospheric temperature is below 70 degrees F, cover loads in trucks with tarps. The tarps must completely cover the exposed load until you transfer the mixture to the paver's hopper or to the pavement surface. Tarps are not required if the time from discharge to truck until transfer to the paver's hopper or the pavement surface is less than 30 minutes.

Apply a tack coat before placing OGFC. The tack coat application rate must comply with the requirements of the following table:

Tack Coat Application Rates for OGFC

OGFC over:	Minimum Residual Rates (gal/sq yd)		
	CSS1/CSS1h, SS1/SS1h and QS1h/CQS1h Asphaltic Emulsion	CRS1/CRS2, RS1/RS2 and QS1/CQS1 Asphaltic Emulsion	Asphalt Binder and PMRS2/PMCRS2 and PMRS2h/PMCRS2h Asphaltic Emulsion
New HMA	0.03	0.04	0.03
PCC and existing AC surfacing	0.05	0.06	0.04
Planed pavement	0.06	0.07	0.05

Compact OGFC with steel-tired, 2-axle tandem rollers. If placing over 300 tons of OGFC per hour, use at least 3 rollers for each paver. If placing less than 300 tons of OGFC per hour, use at least 2 rollers for each paver. Each roller must weigh between 126 to 172 lb per linear inch of drum width. Turn the vibrator off.

Compact OGFC with 2 coverages. The Engineer may order fewer coverages if the layer thickness of OGFC is less than 0.20 foot.

For HMA-O with unmodified asphalt binder:

1. Spread and compact only if the atmospheric temperature is at least 55 degrees F and the surface temperature is at least 60 degrees F.
2. Complete the 1st coverage using 2 rollers before the surface temperature drops below 240 degrees F.
3. Complete all compaction before the surface temperature drops below 200 degrees F.

For HMA-O with modified asphalt binder except asphalt rubber binder:

1. Spread and compact only if the atmospheric temperature is at least 50 degrees F and the surface temperature is at least 50 degrees F.
2. Complete the 1st coverage using 2 rollers before the surface temperature drops below 240 degrees F.
3. Complete all compaction before the surface temperature drops below 180 degrees F.

For RHMA-O and RHMA-O-HB:

1. Spread and compact only if the atmospheric temperature is at least 55 degrees F and surface temperature is at least 60 degrees F.
2 Complete the 1st coverage using 2 rollers before the surface temperature drops below 280 degrees F.
3. Complete compaction before the surface temperature drops below 250 degrees F.

Spread sand at a rate between 1 and 2 lb/sq yd on new RHMA-O and RHMA-O-HB pavement when finish rolling is complete. Sand must be free of clay or organic matter. Sand must comply with section 90-1.02C(3). Keep traffic off the pavement until spreading sand is complete.

If you choose to correct OGFC for smoothness, the Engineer determines if the corrective method causes raveling. OGFC that is raveling must be removed and replaced.

39-4.04 PAYMENT

Not Used

39-5 BONDED WEARING COURSES

39-5.01 GENERAL

39-5.01A General

39-5.01A(1) Summary

Section 39-5 includes specifications for producing and placing bonded wearing courses.

BWC includes placing a polymer modified asphaltic emulsion and the specified HMA in a single pass with an integrated paving machine.

BWC using RHMA-G, RHMA-O, or HMA-O must comply with the specifications for RHMA-G, RHMA-O, or HMA-O.

39-5.01A(2) Definitions

Reserved

39-5.01A(3) Submittals

With your JMF submittal, include:

1. Asphaltic emulsion membrane target residual rate
2. Weight ratio of water to bituminous material in the original asphaltic emulsion

Within 3 business days following the 1st job site delivery, submit test results for asphaltic emulsion properties performed on a sample taken from the asphaltic emulsion delivered.

Within 1 business day of each job site delivery of asphaltic emulsion, submit to METS a 2-quart sample and a certificate of compliance. Ship each sample so that it is received at METS within 48 hours of sampling.

Each day BWC is placed, submit the residual and application rate for the asphaltic emulsion membrane.

During production, submit certified volume or weight slips for the materials supplied.

39-5.01A(4) Quality Control and Assurance

39-5.01A(4)(a) General

For each job site delivery of asphaltic emulsion, take a 2-quart sample in the presence of the Engineer. Take samples from the delivery truck at mid-load from a sampling tap or thief. If the sample is taken from the tap, draw and discard 4 quarts before sampling.

If you unload asphalt binder or asphaltic emulsion into a bulk storage tank, do not use material from the tank until you submit test results for a sample taken from the bulk storage tank. Testing must be performed by an AASHTO-accredited laboratory.

39-5.01A(4)(b) Quality Control

Sample BWC in two 1-gallon metal containers.

The asphaltic emulsion membrane must be tested under ASTM D2995 at least once per paving day at the job site.

39-5.01A(4)(c) Department Acceptance

The Department accepts asphaltic emulsion membrane based on compliance with the requirements shown in the following tables:

Asphaltic Emulsion Membrane

Quality characteristic	Test method	Requirement
Saybolt Furol Viscosity at 25 °C (SFS) [a]	AASHTO T 59	20–100
Sieve test on original emulsion at time of delivery (max, %)	AASHTO T 59	0.05
24-hour storage stability (max, %)	AASHTO T 59	1
Residue by evaporation (min, %)	California Test 331	63
Tests on residue from evaporation test:		
Torsional recovery, measure entire arc of recovery at 25 °C (min, %)	California Test 332	40
Penetration at 25 °C (0.01 mm) PG76-22 M PG64-28 M	AASHTO T 49	 50–70 150–200

[a] SFS means Saybolt Furol seconds

The Department accepts the BWC based on the submitted asphaltic emulsion membrane target residual rate ±0.02 gal/sq yd when tested under ASTM D2995.

39-5.01B Materials

39-5.01B(1) General

Reserved

39-5.01B(2) Asphaltic Emulsion Membrane

The asphaltic emulsion membrane must comply with the requirements shown in the following table:

Asphaltic Emulsion Membrane

Quality characteristic	Test method	Requirement
Saybolt Furol Viscosity at 25 °C (SFS) [a]	AASHTO T 59	20–100
Sieve test on original emulsion at time of delivery (max, %)	AASHTO T 59	0.05
24-hour storage stability (max, %)	AASHTO T 59	1
Residue by evaporation (min, %)	California Test 331	63
Tests on residue from evaporation test:		
Torsional recovery, measure entire arc of recovery at 25 °C (min, %)	California Test 332	40
Penetration at 25 °C (0.01 mm) PG76-22 M PG64-28 M	AASHTO T 49	 50–70 150–200

[a] SFS means Saybolt Furol seconds

39-5.01B(3) Reserved

39-5.01C Construction

39-5.01C(1) General

Use method compaction for BWC.

Do not dilute the asphaltic emulsion.

Do not place BWC if rain is forecast for the project area within 24 hours by the National Weather Service.

39-5.01C(2) Spreading and Compacting Equipment

Use a material transfer vehicle when placing BWC.

Use an integrated distributor paver capable of spraying the asphaltic emulsion membrane, spreading the HMA, and leveling the mat surface in 1 pass.

Apply asphaltic emulsion membrane at a uniform rate for the full paving width. The asphaltic emulsion membrane must not be touched by any part of the paver including wheels or tracks.

If the spray bar is adjusted for changing pavement widths, the paver must prevent excess spraying of asphaltic emulsion beyond 2 inches of the HMA edge.

39-5.01C(3) Applying Asphaltic Emulsion

Before spreading HMA, apply asphaltic emulsion membrane on dry or damp pavement with no free water. Apply asphaltic emulsion when the atmospheric and pavement temperatures are above:

1. 50 degrees F if PG 76-22 M is specified
2. 45 degrees F if PG 64-28 M is specified

Apply emulsion at a temperature from 120 to 180 degrees F and in a single application at the residual rate specified for the condition of the underlying surface. Asphaltic emulsion membrane must have a target residual rate for the surfaces to receive the emulsion as shown in the following table:

Asphaltic Emulsion Membrane Target Residual Rate

Surface to receive asphaltic emulsion membrane	Target residual rates (gal/sq yd)
PCC pavement	0.09–0.11
Dense, compacted, new HMA pavement	0.11–0.14
Open textured, dry, aged or oxidized existing AC pavement	0.13–0.17

If requested and authorized, you may change the asphaltic emulsion membrane application rates.

39-5.01C(4) Placing and Compacting Hot Mix Asphalt

Construct a transverse joint if the HMA remains in the paver for more than 30 minutes.

Do not reintroduce HMA spread over asphaltic emulsion membrane into the paving process.

Do not overlap or hot lap HMA. Pave through lanes after paving adjacent:

1. Shoulders
2. Tapers
3. Transitions
4. Road connections
5. Driveways
6. Curve widenings
7. Chain control lanes
8. Turnouts
9. Turn pockets
10. Ramps

For BWC placed on areas adjacent to through lanes that extend into the through lanes, cut the BWC to a neat, straight vertical line at the lane line.

If you spill asphaltic emulsion into the paver hopper, stop paving and remove the contaminated material.

39-5.01D Payment

Not Used

39-5.02 BONDED WEARING COURSES-GAP GRADED

39-5.02A General

39-5.02A(1) Summary

Section 39-5.02 includes specifications for producing bonded wearing course-gap graded.

39-5.02A(2) Definitions

Reserved

39-5.02A(3) Submittals

Include film thickness and calculations and AASHTO T 305 results with your JMF submittal.

39-5.02A(4) Quality Control and Assurance

39-5.02A(4)(a) General

Reserved

39-5.02A(4)(b) Quality Control

39-5.02A(4)(b)(i) General

Reserved

39-5.02A(4)(b)(ii) Aggregate

Test the quality characteristics of aggregate under the test methods and frequencies shown in the following table:

Aggregate Testing Frequencies

Quality characteristic	Test method	Minimum testing frequency
Gradation	AASHTO T 27	1 per 750 tons and any remaining part
Sand equivalent[a, b]	AASHTO T 176	
Moisture content[c]	AASHTO T 329	1 per 1500 tons and any remaining part
Crushed particles	AASHTO T 335	1 per 10,000 tons or 2 per project, whichever is greater
Los Angeles rattler	AASHTO T 96	
Flat and elongated particles	ASTM D4791	
Fine aggregate angularity	AASHTO T 304 Method A	

[a]Reported value must be the average of 3 tests from a single sample.
[b]Use of a sand reading indicator is required as shown in AASHTO T 176, Figure 1. Sections 4.7, 4.8, 7.1.2, 8.4.2, and 8.4.3 do not apply.
[c]Test at continuous mixing plants only.

For lime treated aggregate, test aggregate before treatment and test for gradation and moisture content during BWC-G production.

39-5.02A(4)(b)(iii) Hot Mix Asphalt Production

Sample BWC in two 1-gallon metal containers.

Test the quality characteristics of BWC-G under the test methods and frequencies shown in the following table:

BWC-G Testing Frequencies

Quality characteristic	Test method	Minimum testing frequency
Asphalt binder content	AASHTO T 308 Method A	1 per 750 tons and any remaining part
HMA moisture content	AASHTO T 329	1 per 2,500 tons but not less than 1 per paving day

39-5.02A(4)(b)(iv)–39-5.02A(4)(b)(vii) Reserved

39-5.02A(4)(c) Department Acceptance

The Department accepts BWC-G based on compliance with:

1. Asphalt binder content at JMF -0.4, +0.5 percent when tested under AASHTO T 308, Method A.
2. Aggregate quality requirements shown in the following table:

Aggregate Quality

Quality characteristic	Test method	Requirement
Aggregate gradation	AASHTO T 27	JMF ± Tolerance
Percent of crushed particles Coarse aggregate (min, %) One-fractured face Two-fractured faces Fine aggregate (min, %) (Passing No. 4 sieve and retained on No. 8 sieve.) One fractured face	AASHTO T 335	 -- 90 85
Los Angeles Rattler (max, %) Loss at 100 Rev. Loss at 500 Rev.	AASHTO T 96	 12 35
Sand equivalent (min)	AASHTO T 176	47
Flat and elongated particles (max, % by weight at 5:1)	ASTM D4791	25
Fine aggregate angularity (min, %)	AASHTO T 304 Method A	45

[a]Reported value must be the average of 3 tests from a single sample.
[b]Use of a sand reading indicator is required as shown in AASHTO T 176, Figure 1. Sections 4.7, 4.8, 7.1.2, 8.4.2 and 8.4.3 do not apply.

The Department accepts asphaltic emulsion membrane based on compliance with the requirements shown in the following tables:

Asphaltic Emulsion Membrane

Quality characteristic	Test method	Requirement
Saybolt Furol Viscosity at 25 °C (SFS) [a]	AASHTO T 59	20–100
Sieve test on original emulsion at time of delivery (max, %)	AASHTO T 59	0.05
24-hour storage stability (max, %)	AASHTO T 59	1
Residue by evaporation (min, %)	California Test 331	63
Tests on residue from evaporation test:		
Torsional recovery, measure entire arc of recovery at 25 °C (min, %)	California Test 332	40
Penetration at 25 °C (0.01 mm) PG76-22 M PG64-28 M	AASHTO T 49	 50–70 150–200

[a] SFS means Saybolt Furol seconds

39-5.02B Materials

39-5.02B(1) General

Reserved

39-5.02B(2) Mix Design

For BWC-G, the 1st paragraph of section 39-1.02B(1) does not apply.

Determine the proposed OBC from a mix design that complies with the requirements shown in the following table:

Hot Mix Asphalt Mix Design Requirements

Quality characteristic	Test method	Requirement
Film thickness (min, µm)	Asphalt Institute MS-2 Table 6.1 [a]	12
Drain down (max, %)	AASHTO T 305 [b]	0.1

[a] Film thickness is calculated based on the effective asphalt content and determined as follows:

$$FT = \left(\frac{P_{be}}{SA \times G_b \times 1000} \right) 10^6$$

Where:

FT = Film thickness in µm
P_{be} = Effective asphalt content by total weight of mix using SP-2 Asphalt Mixture
SA = Estimated surface area of the aggregate blend in m^2/kg from Table 6.1 in the Asphalt Institute Manual Series No. 2 (MS-2).
G_b = Specific gravity of asphalt binder

[b] Combine aggregate and asphalt at the asphalt binder supplier's instructed mixing temperature. Coated aggregates that fall through the wire basket during loading must be returned to the basket before conditioning at 350 °F for 1 hour.

The OBC must be greater than 4.9 percent by total weight of mix.

39-5.02B(3) Asphalt Binder

Reserved

39-5.02B(4) Aggregate

The aggregate must comply with the requirements shown in the following table:

Aggregate Quality

Quality characteristic	Test method	Requirement
Percent of crushed particles Coarse aggregate (min, %) One-fractured face Two-fractured faces Fine aggregate (min, %) (Passing No. 4 sieve and retained on No. 8 sieve.) One fractured face	AASHTO T 335	 -- 90 85
Los Angeles Rattler (max, %) Loss at 100 Rev. Loss at 500 Rev.	AASHTO T 96	 12 35
Sand equivalent (min)	AASHTO T 176	47
Flat and elongated particles (max, % by weight @ 5:1)	ASTM D4791	25
Fine aggregate angularity (min, %)	AASHTO T 304 Method A	45

[a]Reported value must be the average of 3 tests from a single sample.
[b]Use of a sand reading indicator is required as shown in AASHTO T 176, Figure 1. Sections 4.7, 4.8, 7.1.2, 8.4.2 and 8.4.3 do not apply.

The aggregate gradations for BWC-G must comply with the requirements shown in the following table:

Aggregate Gradation Requirements

BWC-G pavement thickness	Gradation
less than 0.08 foot	No. 4 or 3/8 inch
0.08 foot and greater	1/2 inch

The proposed aggregate gradation must be within the TV limits for the specified sieve sizes shown in the following tables:

Aggregate Gradation
(Percentage Passing)
Bonded Wearing Course—Gap Graded

1/2-inch BWC-G

Sieve sizes	Target value limits	Allowable tolerance
3/4"	100	--
1/2"	80–100	TV ± 6
3/8"	55–80	TV ± 6
No. 4	25–40	TV ± 7
No. 8	19–32	TV ± 5
No. 16	16–22	TV ± 5
No. 30	10–18	TV ± 4
No. 50	8–13	TV ± 4
No. 100	6–10	TV ± 2
No. 200	4.0–7.0	TV ± 2

3/8-inch BWC-G

Sieve sizes	Target value limits	Allowable tolerance
1/2"	100	--
3/8"	80–100	TV ± 6
No. 4	25–40	TV ± 7
No. 8	19–32	TV ± 5
No. 16	16–22	TV ± 5
No. 30	10–18	TV ± 4
No. 50	8–13	TV ± 4
No. 100	7–11	TV ± 2
No. 200	6.0–10.0	TV ± 2

No. 4 BWC-G

Sieve sizes	Target value limits	Allowable tolerance
1/2"	100	--
3/8"	95–100	TV ± 2
No. 4	42–55	TV ± 7
No. 8	19–32	TV ± 5
No. 16	16–22	TV ± 5
No. 30	10–18	TV ± 4
No. 50	8–13	TV ± 4
No. 100	7–11	TV ± 2
No. 200	6.0–10.0	TV ± 2

39-5.02C Construction

Not Used

39-5.02D Payment

Not Used

39-6 HOT MIX ASPHALT ON BRIDGE DECKS

39-6.01 GENERAL

Section 39-6 includes specifications for producing and placing hot mix asphalt on bridge decks.

HMA used for bridge decks must comply with the specifications for Type A HMA in section 39-2.

39-6.02 MATERIALS

Do not use the 1-inch or 3/4-inch aggregate gradation for HMA on bridge decks.

The grade of asphalt binder for HMA must be PG 64-10 or PG 64-16.

39-6.03 CONSTRUCTION

Spread and compact HMA on bridge decks using method compaction.

If a concrete expansion dam is to be placed at a bridge deck expansion joint, tape oil-resistant construction paper to the deck over the area to be covered by the dam before placing the tack coat and HMA across the joint.

Apply tack coat at the minimum residual rate specified in section 39-1.03C(5). For HMA placed on a deck seal, use the minimum residual rate specified for PCC.

For HMA placed on a deck seal:

1. Place the HMA within 7 days after installing the deck seal.
2. If a paper mask is placed on the deck under section 54-5.03, place the HMA continuously across the paper mask.
3. Place HMA in at least 2 approximately equal layers.
4. For placement of the 1st HMA layer:
   - 4.1. Comply with the HMA application temperature recommended by the deck seal manufacturer.
   - 4.2. Deliver and place HMA using equipment with pneumatic tires or rubber-faced wheels. Do not operate other vehicles or equipment on the bare deck seal.
   - 4.3. Deposit HMA on the deck seal in such a way that the deck seal is not damaged. Do not use a windrow.
   - 4.4. Place HMA in a downhill direction on bridge decks with grades over 2 percent.
   - 4.5. Self-propelled spreading equipment is not required.

39-6.04 PAYMENT

Not Used

39-7 MINOR HOT MIX ASPHALT

39-7.01 GENERAL

39-7.01A Summary

Section 39-7 includes specifications for producing and placing minor hot mix asphalt.

Minor HMA must comply with section 39-2 except as specified in this section 39-7.

39-7.01B Definitions

Reserved

39-7.01C Submittals

The QC plan, test results, and inertial profiler specifications in sections 39-1.01C(3), 39-1.01C(4), 39-1.01C(13)(c)–(d) do not apply.

39-7.01D Quality Control and Assurance

39-7.01D(1) General

For minor HMA, the JMF renewal, inertial profiler certifications and testing, and prepaving meeting specifications in sections 39-1.01D(4), 39-1.01D(6)(c), and 39-1.01D(7) do not apply.

Test pavement smoothness with a 12 foot straightedge.

39-7.01D(2) Quality Control

For minor HMA, section 39-2.01D(2) applies except testing for compliance with the following quality characteristics is not required:

1. Flat and elongated particles
2. Fine aggregate angularity
3. Hamburg wheel track
4. Moisture susceptibility

39-7.01D(3) Department Acceptance

The Department accepts minor HMA under section 39-2.01D(5) except compliance with the following quality characteristics is not required:

1. Flat and elongated particles
2. Fine aggregate angularity
3. Hamburg wheel track
4. Moisture susceptibility

39-7.02 MATERIALS

39-7.02A General

Reserved

39-7.02B Mix Design

The mix design for minor HMA must comply with section 39-2.02B except the Hamburg wheel track and moisture susceptibility requirements do not apply.

39-7.02C Asphalt Binder

The grade of asphalt binder for minor HMA must be PG-64-10 or PG-64-16.

39-7.02D Liquid Antistrip Treatment

Treat minor HMA with liquid antistrip.

39-7.03 CONSTRUCTION

Not Used

39-7.04 PAYMENT

Not Used

39-8–39-10 RESERVED

^^

40 CONCRETE PAVEMENT

07-19-13

Replace the headings and paragraphs in section 40 with:

07-19-13

40-1 GENERAL

40-1.01 GENERAL

40-1.01A Summary

Section 40-1 includes general specifications for constructing concrete pavement.

40-1.01B Definitions

concrete raveling: Progressive disintegration of the pavement surface resulting from dislodged aggregate.

full depth crack: Crack that runs from one edge of the slab to the opposite or adjacent side of the slab, except a crack parallel to and within 0.5 foot of either side of a planned contraction joint

working crack: Crack that extends through the full depth of the slab and is parallel to and within 0.5 foot of either side of a planned contraction joint.

action limit: Value at which corrective actions must be made while production may continue.

suspension limit: Value at which production must be suspended while corrections are made.

40-1.01C Submittals

40-1.01C(1) General

At least 15 days before delivery to the job site, submit manufacturer's recommendations and instructions for storage and installation of:

1. Threaded tie bar splice couplers
2. Joint filler

As an informational submittal, submit calibration documentation and operational guidelines for frequency measuring devices (tachometer) for concrete consolidation vibrators.

Submit updated quality control charts each paving day.

40-1.01C(2) Certificates of Compliance

Submit a certificate of compliance for:

1. Tie bars
2. Threaded tie bar splice couplers
3. Dowel bars
4. Tie bar baskets
5. Dowel bar baskets
6. Joint filler
7. Epoxy powder coating

40-1.01C(3) Quality Control Plan

Submit a concrete pavement QC plan. Allow 30 days for review.

40-1.01C(4) Mix Design

At least 15 days before testing for mix proportions, submit a copy of the AASHTO accreditation for your laboratory determining the mix proportions. At least 15 days before starting field qualification, submit the proposed concrete mix proportions, the corresponding mix identifications, and laboratory test reports including the modulus of rupture for each trial mixture at 10, 21, 28, and 42 days.

40-1.01C(5) Concrete Field Qualification

Submit field qualification data and test reports including:

1. Mixing date
2. Mixing equipment and procedures used
3. Batch volume in cubic yards. The minimum batch size is 5 cu yd.
4. Type and source of ingredients used
5. Penetration of the concrete
6. Air content of the plastic concrete
7. Age and strength at time of concrete beam testing

Field qualification test reports must be certified with a signature by an official in responsible charge of the laboratory performing the tests.

40-1.01C(6) Cores

Submit for authorization the name of the laboratory you propose to use for testing the cores for air content.

Submit each core in an individual plastic bag marked with a location description.

40-1.01C(7) Profile Data and Straightedge Measurements

At least 5 business days before start of initial profiling or changing profiler or operator, submit:

1. Inertial profiler (IP) certification issued by the Department. The certification must not be more than 12 months old.
2. Operator certification for the IP issued by the Department. The operator must be certified for each different model of IP device operated. The certification must not be more than 12 months old.
3. List of manufacturer's recommended test procedures for IP calibration and verification.

Within 2 business days after cross correlation testing, submit ProVAL profiler certification analysis report for cross correlation test results performed on test section. ProVAL is FHWA's software. Submit the certification analysis report to the Engineer and to the electronic mailbox address:

smoothness@dot.ca.gov

Within 2 business days after each day of inertial profiling, submit profile data to the Engineer and to the electronic mailbox address:

smoothness@dot.ca.gov

Within 2 business days of performing straightedge testing, submit a report of areas requiring smoothness correction.

40-1.01C(8)–40-1.01C(12) Reserved

40-1.01D Quality Control and Assurance

40-1.01D(1) General

If the pavement quantity is at least 2000 cu yd, provide a QC manager.

Core pavement as described for, thickness, bar placement, and air content.

For the Department's modulus of rupture testing, assist the Engineer in fabricating test beams by providing materials and labor.

Allow at least 25 days for the Department to schedule testing for coefficient of friction. Notify the Engineer when the pavement is scheduled to be opened to traffic. Notify the Engineer when the pavement is ready for testing which is the latter of:

1. Seven days after paving
2. When the pavement has attained a modulus of rupture of at least 550 psi

The Department tests for coefficient of friction within 7 days of receiving notification that the pavement is ready for testing.

40-1.01D(2) Prepaving Conference

Schedule a prepaving conference at a mutually agreed upon time and place to meet with the Engineer. Make the arrangements for the conference facility. Discuss QC plan and methods of performing each item of the work.

Prepaving conference attendees must sign an attendance sheet provided by the Engineer. The prepaving conference must be attended by your:

1. Project superintendent
2. QC manager
3. Paving construction foreman
4. Workers and your subcontractor's workers, including:
   - 4.1. Foremen including subcontractor's Foremen
   - 4.2. Concrete plant manager
   - 4.3. Concrete plant operator

Do not start paving activities including test strips until the listed personnel have attended a prepaving conference.

40-1.01D(3) Just-In-Time-Training

Reserved

40-1.01D(4) Quality Control Plan

Establish, implement, and maintain a QC plan for pavement. The QC plan must describe the organization and procedures used to:

1. Control the production process
2. Determine if a change to the production process is needed
3. Implement a change

The QC plan must include action and suspension limits and details of corrective action to be taken if any process is out of those limits. Suspension limits must not exceed specified acceptance criteria.

The QC plan must address the elements affecting concrete pavement quality including:

1. Mix proportions
2. Aggregate gradation
3. Materials quality
4. Stockpile management
5. Line and grade control
6. Proportioning
7. Mixing and transportation
8. Placing and consolidation
9. Contraction and construction joints
10. Bar reinforcement placement and alignment
11. Dowel bar placement, alignment, and anchorage
12. Tie bar placement
13. Modulus of rupture
14. Finishing and curing
15. Protecting pavement
16. Surface smoothness

40-1.01D(5) Mix Design

Use a laboratory that complies with ASTM C 1077 to determine the mix proportions for concrete pavement. The laboratory must have a current AASHTO accreditation for:

1. AASHTO T 97 or ASTM C 78
2. ASTM C 192/C 192M

Make trial mixtures no more than 24 months before field qualification.

Using your trial mixtures, determine the minimum cementitious materials content. Use your value for minimum cementitious material content for *MC* in equation 1 and equation 2 of section 90-1.02B(3).

To determine the minimum cementitious materials content or maximum water to cementitious materials ratio, use modulus of rupture values of at least 570 psi for 28 days age and at least 650 psi for 42 days age.

If changing an aggregate supply source or the mix proportions, produce a trial batch and field-qualify the new concrete. The Engineer does not adjust contract time for performing sampling, testing, and qualifying new mix proportions or changing an aggregate supply source.

40-1.01D(6) Quality Control Testing

40-1.01D(6)(a) General

Testing laboratories and testing equipment must comply with the Department's Independent Assurance Program.

40-1.01D(6)(b) Concrete Mix

Before placing pavement, your mix design must be field qualified. Use an ACI certified "Concrete Laboratory Technician, Grade I" to perform field qualification tests and calculations. Test for modulus of rupture under California Test 523 at 10, 21, and 28 days of age.

When placing pavement, your quality control must include testing properties at the frequencies shown in the following table:

QC Testing Frequency

Property	Test method	Minimum frequency
Cleanness value	California Test 227	2 per day
Sand equivalent	California Test 217	2 per day
Aggregate gradation	California Test 202	2 per day
Air content (air entrainment specified)	California Test 504	1 per hour
Air content (air entrainment not specified)	California Test 504	1 per 4 hours
Density	California Test 518	1 per 4 hours
Penetration	California Test 533	1 per 4 hours
Aggregate moisture meter calibration[a]	California Test 223 or California Test 226	1 per day

[a] Check calibration of the plant moisture meter by comparing moisture meter readings with California Test 223 or California Test 226 test results.

Maintain control charts to identify potential problems and assignable causes. Post a copy of each control chart at a location determined by the Engineer.

Individual measurement control charts must use the target values in the mix proportions as indicators of central tendency.

Develop linear control charts for:

1. Cleanness value
2. Sand equivalent
3. Fine and coarse aggregate gradation
4. Air content
5. Penetration

Control charts must include:

1. Contract number
2. Mix proportions
3. Test number
4. Each test parameter
5. Action and suspension limits
6. Specification limits
7. Quality control test results

For fine and coarse aggregate gradation control charts, record the running average of the previous 4 consecutive gradation tests for each sieve and superimpose the specification limits.

For air content control charts, the action limit is ±1.0 percent of the specified value. If no value is specified, the action limit is ±1.0 percent of the value used for your approved mix design.

As a minimum, a process is out of control if any of the following occurs:

1. For fine and coarse aggregate gradation, 2 consecutive running averages of 4 tests are outside the specification limits
2. For individual penetration or air content measurements:
   2.1. One point falls outside the suspension limit line
   2.2. Two points in a row fall outside the action limit line

Stop production and take corrective action for out of control processes or the Engineer rejects subsequent material.

Before each day's concrete pavement placement and at intervals not to exceed 4 hours of production, use a tachometer to test and record vibration frequency for concrete consolidation vibrators.

40-1.01D(6)(c) Pavement Smoothness

40-1.01D(6)(c)(i) General

Notify the Engineer 2 business days before performing smoothness testing including IP calibration and verification testing. The notification must include start time and locations by station.

Before testing the pavement smoothness, remove foreign objects from the surface, and mark the beginning and ending station on the pavement shoulder.

Test pavement smoothness using an IP except use a 12-foot straightedge at the following locations:

1. Traffic lanes less than 1,000 feet in length including ramps, turn lanes, and acceleration and deceleration lanes
2. Areas within 15 feet of manholes
3. Shoulders
4. Weigh-in-motion areas
5. Miscellaneous areas such as medians, gore areas, turnouts, and maintenance pullouts

40-1.01D(6)(c)(ii) Straightedge Testing

Identify locations of areas requiring correction by:

1. Location Number
2. District-County-Route
3. Beginning station or post mile to the nearest 0.01 mile
4. For correction areas within a lane:
   - 4.1. Lane direction as NB, SB, EB, or WB
   - 4.2. Lane number from left to right in direction of travel
   - 4.3. Wheel path as "L" for left, "R" for right, or "B" for both
5. For correction areas not within a lane:
   - 5.1. Identify pavement area (e.g., shoulder, weight station, turnout)
   - 5.2. Direction and distance from centerline as "L" for left or "R" for right
6. Estimated size of correction area

40-1.01D(6)(c)(iii) Inertial Profile Testing

IP equipment must display a current certification decal with expiration date.

Conduct cross correlation IP verification test in the Engineer's presence before performing initial profiling. Verify cross correlation IP verification test at least annually. Conduct 5 repeat runs of the IP on an authorized test section. The test section must be on an existing concrete pavement surface 0.1 mile long. Calculate a cross correlation to determine the repeatability of your device under Section 8.3.1.2 of AASHTO R 56 using ProVAL profiler certification analysis with a 3 feet maximum offset. The cross correlation must be a minimum of 0.92.

Conduct the following IP calibration and verification tests in the Engineer's presence each day before performing inertial profiling:

1. Block test. Verify the height sensor accuracy under AASHTO R 57, section 5.3.2.3.
2. Bounce test. Verify the combined height sensor and accelerometer accuracy under AASHTO R 57, section 5.3.2.3.2.
3. DMI test. Calibrate the accuracy of the testing procedure under AASHTO R 56, section 8.4.
4. Manufacturer's recommended tests.

Collect IP data using the specified ProVAL analysis with 250 mm and IRI filters. Comply with the requirements for data collection under AASHTO R 56.

For IP testing, wheel paths are 3 feet from and parallel to the edge of a lane. Left and right are relative to the direction of travel. The IRI is the pavement smoothness along a wheel path of a given lane. The MRI is the average of the IRI values for the left and right wheel path from the same lane.

Operate the IP according to the manufacturer's recommendations and AASHTO R 57 at 1-inch recording intervals and a minimum 4 inch line laser sensor.

Collect IP data under AASHTO R 56.IP data must include:

1. Raw profile data for each lane.
2. ProVAL ride quality analysis report for the international roughness index (IRI) of left and right wheel paths of each lane. Submit in pdf file format.
3. ProVAL ride quality analysis report for the mean roughness index (MRI) of each lane. Submit in pdf file format.
4. ProVAL smoothness assurance analysis report for IRIs of left wheel path. Submit in pdf file format.
5. ProVAL smoothness assurance analysis report for IRIs of right wheel path. Submit in pdf file format.
6. GPS data file for each lane in GPS exchange. Submit in GPS eXchange file format.
7. Manufacturer's recommended IP calibration and verification tests results.
8. AASHTO IP calibration and verification test results including bounce, block, and distance measurement instrument (DMI).

Submit the IP raw profile data in unfiltered electronic pavement profile file (PPF) format. Name the PPF file using the following naming convention:

YYYYMMDD_TTCCCRRR_D_L_W_S_X_PT.PPF

where:
YYYY = year
MM = Month, leading zero
DD = Day of month, leading zero
TT = District, leading zero
CCC = County, 2 or 3 letter abbreviation as shown in section 1-1.08
RRR = Route number, no leading zeros
D = Traffic direction as NB, SB, WB, or EB
L = Lane number from left to right in direction of travel
W = Wheel path as "L" for left, "R" for right, or "B" for both
S = Beginning station to the nearest foot (e.g., 10+20) or beginning post mile to the nearest hundredth (e.g., 25.06) no leading zero
X = Profile operation as "EXIST" for existing pavement, "PAVE" for after paving, or "CORR" for after final surface pavement correction
PT = Pavement type (e.g., "concrete", etc.)

Determine IRIs using the ProVAL ride quality analysis with a 250 mm and IRI filters. While collecting the profile data to determine IRI, record the following locations in the raw profile data:

1. Begin and end of all bridge approach slabs
2. Begin and end of all bridges
3. Begin and end of all culverts visible on the roadway surface

For each 0.1 mile section, your IRI values must be within 10 percent of the Department's IRI values. The Engineer may order you to recalibrate your IP equipment and reprofile. If your results are inaccurate due to operator error, the Engineer may disqualify your IP operator.

Determine the MRI for 0.1-mile fixed sections. A partial section less than 0.1 mile that is the result of an interruption to continuous pavement surface must comply with the MRI specifications for a full section. Adjust the MRI for a partial section to reflect a full section based on the proportion of a section paved.

Determine the areas of localized roughness. Use the ProVAL smoothness assurance with a continuous IRI for each wheel path, 25-foot interval, and 250 mm and IRI filters.

40-1.01D(6)(c)(iv) Reserved

40-1.01D(6)(d)–40-1.01D(6)(h) Reserved

40-1.01D(7) Pavement Acceptance

40-1.01D(7)(a) Acceptance Testing

40-1.01D(7)(a)(i) General

The Department's acceptance testing includes testing the pavement properties at the minimum frequencies shown in the following table:

Acceptance Testing

Property	Test Method		Frequency[a]
	CRCP	JPCP	
Modulus of rupture (28 day)	California Test 523		1,000 cu yd
Air content[b]	California Test 504		1 day's paving
Dowel bar placement	--	Measurement[a]	700 sq yd
Tie bar placement	--	Measurement[a]	4,000 sq yd
Thickness	California Test 531		1,200 sq yd
Coefficient of friction	California Test 342		1 day's paving

[a]A single test represents no more than the frequency specified.
[b]Tested only when air entrainment is specified.

Pavement smoothness may be accepted based on your testing in the absence of the Department's testing.

40-1.01D(7)(a)(ii) Air Content

If air-entraining admixtures are specified, the Engineer uses a t-test to compare your QC test results with the Department's test results. The t-value for test data is determined using the following equation:

$$t = \frac{\left|\overline{X}_c - \overline{X}_v\right|}{S_p\sqrt{\frac{1}{n_c} + \frac{1}{n_v}}} \quad \text{and} \quad S_p^2 = \frac{S_c^2(n_c - 1) + S_v^2(n_v - 1)}{n_c + n_v - 2}$$

where:

n_c = Number of your quality control tests (minimum of 6 required)
n_v = Number of Department's tests (minimum of 2 required)
$\overline{X}_c$ = Mean of your quality control tests
$\overline{X}_v$ = Mean of the Department's tests
S_p = Pooled standard deviation
(When $n_v = 1$, $S_p = S_c$)
S_c = Standard deviation of your quality control tests
S_v = Standard deviation of the Department's tests (when $n_v > 1$)

The Engineer compares your QC test results with the Department's test results at a level of significance of $\alpha = 0.01$. The Engineer compares the t-value to tcrit, using degrees of freedom showing in the following table:

degrees of freedom (nc+nv-2)	tcrit (for α = 0.01)
1	63.657
2	9.925
3	5.841
4	4.604
5	4.032
6	3.707
7	3.499
8	3.355
9	3.250
10	3.169

If the t-value calculated is less than or equal to tcrit, your quality control test results are verified. If the t-value calculated is greater than tcrit, quality control test results are not verified.

If your quality control test results are not verified, core at least 3 specimens from concrete pavement under section 40-1.03P. The Engineer selects the core locations. The authorized laboratory must test these specimens for air content under ASTM C 457. The Engineer compares these test results with your quality control test results using the t-test method. If your quality control test results are verified based on this comparison, the Engineer uses the quality control test results for acceptance of concrete pavement for air content. If your quality control test results are not verified based on this comparison, the Engineer uses the air content of core specimens determined by the authorized laboratory under ASTM C 457 for acceptance.

40-1.01D(7)(a)(iii) Dowel and Tie Bar Placement

For JPCP, drill cores under section 40-1.03P for the Department's acceptance testing.

The Engineer identifies which joint and dowel or tie bar are to be tested. Core each day's paving within 2 business days. Each dowel or tie bar test consists of 2 cores, 1 on each bar end to expose both ends and allow measurement.

If the tests indicate dowel or tie bars are not placed within the specified tolerances or if there is unconsolidated concrete around the dowel or tie bars, core additional specimens identified by Engineer to determine the limits of unacceptable work.

40-1.01D(7)(a)(iv) Thickness

Drill cores under section 40-1.03P for the Department's acceptance testing in the primary area, which is the area placed in 1 day for each thickness. Core at locations determined by the Engineer and in the Engineer's presence.

Do not core until any grinding has been completed.

The core specimen diameter must be 4 inches. To identify the limits of concrete pavement deficient in thickness by more than 0.05 foot, you may divide primary areas into secondary areas. The Engineer measures cores under California Test 531 to the nearest 0.01 foot. Core at least 1 foot from existing, contiguous, and parallel concrete pavement not constructed as part of this Contract.

You may request the Engineer make additional thickness measurements and use them to determine the average thickness variation. The Engineer determines the locations with random sampling methods.

If each thickness measurement in a primary area is less than 0.05 foot deficient, the Engineer calculates the average thickness deficiency in that primary area. The Engineer uses 0.02 foot for a thickness difference more than 0.02 foot over the specified thickness.

For each thickness measurement in a primary area deficient by more than 0.05 foot, the Engineer determines a secondary area where the thickness deficiency is more than 0.05 foot. The Engineer determines this secondary area by measuring the thickness of each concrete pavement slab adjacent to

the measurement found to be more than 0.05 foot deficient. The Engineer continues to measure the thickness until an area that is bound by slabs with thickness deficient by 0.05 foot or less is determined.

Slabs without bar reinforcement are defined by the areas bound by longitudinal and transverse joints and concrete pavement edges. Slabs with bar reinforcement are defined by the areas bound by longitudinal joints and concrete pavement edges and 15-foot lengths. Secondary area thickness measurements in a slab determine that entire slab's thickness.

The Engineer measures the remaining primary area thickness after removing the secondary areas from consideration for determining the average thickness deficiency.

40-1.01D(7)(a)(v)–40-1.01D(7)(a)(ix) Reserved

40-1.01D(7)(b) Acceptance Criteria

40-1.01D(7)(b)(i) General

Reserved

40-1.01D(7)(b)(ii) Modulus of Rupture

For field qualification, the modulus of rupture at no later than 28 days must be at least:

1. 550 psi for each single beam
2. 570 psi for the average of 5 beams

For production, the modulus of rupture for the average of the individual test results of 2 beams aged for 28 days must be at least 570 psi.

40-1.01D(7)(b)(iii) Air Content

The air content must be within ±1.5 percent of the specified value. If no value is specified, the air content must be within ±1.5 percent of, the value used for your approved mix design.

40-1.01D(7)(b)(iv) Bar Reinforcement

In addition to requirements of Section 52, bar reinforcement must be more than 1/2 inch below the saw cut depth at concrete pavement joints.

40-1.01D(7)(b)(v) Dowel Bar and Tie Bar Placement

Tie bar placement must comply with the tolerances shown in the following table:

Tie Bar Tolerance

Dimension	Tolerance
Horizontal and vertical skew	5 1/4 inch, max
Longitudinal translation	± 2 inch
Horizontal offset (embedment)	± 2 inch
Vertical depth	1. At least 1/2 inch below the bottom of the saw cut 2. When measured at any point along the bar, not less than 2 inches clear of the pavement's surface and bottom

NOTE: Tolerances are measured relative to the completed joint.

Dowel bar placement must comply with the tolerances shown in the following table:

Dowel Bar Tolerances

Dimension	Tolerance
Horizontal offset	±1 inch
Longitudinal translation	±2 inch
Horizontal skew	5/8 inch, max
Vertical skew	5/8 inch, max
Vertical depth	The minimum distance measured from concrete pavement surface to any point along the top of dowel bar must be: DB + 1/2 inch where: DB = one third of pavement thickness in inches, or the saw cut depth, whichever is greater The maximum distance below the depth shown must be 5/8 inch.

NOTE: Tolerances are measured relative to the completed joint.

The Engineer determines the limits for removal and replacement.

40-1.01D(7)(b)(vi) Pavement Thickness

Concrete pavement thickness must not be deficient by more than 0.05 foot.

The minimum thickness is not reduced for specifications that may affect concrete pavement thickness such as allowable tolerances for subgrade construction.

The Engineer determines the areas of noncompliant pavement, the thickness deficiencies, and the limits where removal is required.

Pavement with an average thickness deficiency less than 0.01 foot is acceptable. If the thickness deficiency is 0.01 foot or more and less than 0.05 foot, you may request authorization to leave the pavement in place and accept a pay adjustment. If the deficiency is more than 0.05 foot the pavement must be removed and replaced.

40-1.01D(7)(b)(vii) Pavement Smoothness

Where testing with an IP is required, the pavement surface must have:

1. No areas of localized roughness with an IRI greater than 120 in/mi
2. MRI of 60 in/mi or less within a 0.1 mile section

Where testing with a straightedge is required, the pavement surface must not vary from the lower edge of the straightedge by more than:

1. 0.01 foot when the straightedge is laid parallel with the centerline
2. 0.02 foot when the straightedge is laid perpendicular to the centerline and extends from edge to edge of a traffic lane
3. 0.02 foot when the straightedge is laid within 24 feet of a pavement conform

40-1.01D(7)(b)(viii) Coefficient of Friction

Initial and final texturing must produce a coefficient of friction of at least 0.30. Do not open the pavement to traffic unless the coefficient of friction is at least 0.30.

40-1.01D(7)(b)(ix)–40-1.01D(7)(b)(xii) Reserved

40-1.02 MATERIALS

40-1.02A General

Water for coring must comply with section 90.

Tack coat must comply with section 39.

40-1.02B Concrete

40-1.02B(1) General

PCC for pavement must comply with section 90-1 except as otherwise specified.

40-1.02B(2) Cementitious Material

Concrete must contain from 505 pounds to 675 pounds cementitious material per cubic yard. The specifications for reducing cementitious material content in section 90-1.02E(2) do not apply .

40-1.02B(3) Aggregate

Aggregate must comply with section 90-1.02C except the specifications for reduction in operating range and contract compliance for cleanness value and sand equivalent specified in section 90-1.02C(2) and section 90-1.02C(3) do not apply.

For coarse aggregate in high desert and high mountain climate regions, the loss must not exceed 25 percent when tested under California Test 211 with 500 revolutions.

For combined aggregate gradings, the difference between the percent passing the 3/8-inch sieve and the percent passing the no. 8 sieve must not be less than 16 percent of the total aggregate.

40-1.02B(4) Air Entrainment

The second paragraph of section 90-1.02I(2)(a) does not apply.

For a project shown in the low and south mountain climate regions, add air-entraining admixture to the concrete at the rate required to produce an air content of 4 percent in the freshly mixed concrete.

For a project shown in the high desert and high mountain climate regions, add air-entraining admixture to the concrete at the rate required to produce an air content of 6 percent in the freshly mixed concrete.

40-1.02B(5)–40-1.02B(8) Reserved

40-1.02C Reinforcement, Bars, and Baskets

40-1.02C(1) Bar Reinforcement

Bar reinforcement must be deformed bars.

If the project is not shown to be in high desert or any mountain climate region, bar reinforcement must comply with section 52.

If the project is shown to be in high desert or any mountain climate regions, bar reinforcement must be one of the following:

1. Epoxy-coated bar reinforcement under section 52-2.03B except bars must comply with either ASTM A 706/A 706M; ASTM A 996/A 996M; or ASTM A 615/A 615M, Grade 40 or 60. Bars must be handled under ASTM D 3963/D 3963M and section 52-2.02C.
2. Low carbon, chromium steel bar complying with ASTM A 1035/A 1035M

40-1.02C(2) Dowel Bars

Dowel bars must be plain bars. Fabricate, sample, and handle epoxy-coated dowel bars under ASTM D 3963/D 3963M and section 52-2.03C except each sample must be 18 inches long.

If the project is not shown to be in high desert or any mountain climate region, dowel bars must be one of the following:

1. Epoxy-coated bars. Bars must comply with ASTM A 615/A 615M, Grade 40 or 60. Epoxy coating must comply with either section 52-2.02B or 52-2.03B.

2. Stainless-steel bars. Bars must be descaled solid stainless-steel bars under ASTM A 955/A 955M, UNS Designation S31603 or S31803.
3. Low carbon, chromium-steel bars under ASTM A 1035/A 1035M.

If the project is shown to be in high desert or any mountain climate region, dowel bars must be one of the following:

1. Epoxy-coated bars. Bars must comply with ASTM A 615/A 615M, Grade 40 or 60. Epoxy coating must comply with section 52-2.03B.
2. Stainless-steel bars. Bars must be descaled solid stainless-steel bars under ASTM A 955/A 955M, UNS Designation S31603 or S31803.

40-1.02C(3) Tie Bars

Tie bars must be deformed bars.

If the project is not shown to be in high desert or any mountain climate region, tie bars must be one of the following:

1. Epoxy-coated bar reinforcement. Bars must comply with either section 52-2.02B or 52-2.03B except bars must comply with either ASTM A 706/A 706M; ASTM A 996/A 996M; or ASTM A 615/A 615M, Grade 40 or 60.
2. Stainless-steel bars. Bars must be descaled solid stainless-steel bars under ASTM A 955/A 955M, UNS Designation S31603 or S31803.
3. Low carbon, chromium-steel bars under ASTM A 1035/A 1035M.

If the project is shown to be in high desert or any mountain climate region, tie bars must be one of the following:

1. Epoxy-coated bar reinforcement. Bars must comply with section 52-2.03B except bars must comply with either ASTM A 706/A 706M; ASTM A 996/A 996M; or ASTM A 615/A 615M, Grade 40 or 60.
2. Stainless-steel bars. Bars must be descaled solid stainless-steel bars under ASTM A 955/A 955M, UNS Designation S31603 or S31803.

Fabricate, sample, and handle epoxy-coated tie bars under ASTM D 3963/D 3963M, section 52-2.02, or section 52-2.03.

Do not bend tie bars.

40-1.02C(4) Dowel and Tie Bar Baskets

For dowel and tie bar baskets, wire must comply with ASTM A 82/A 82M and be welded under ASTM A 185/A 185M, Section 7.4. The minimum wire-size no. is W10. Use either U-frame or A-frame shaped assemblies.

If the project is not shown to be in high desert or any mountain climate region, baskets may be epoxy-coated, and the epoxy coating must comply with either section 52-2.02B or 52-2.03B.

If the project is shown to be in high desert or any mountain climate region, wire for dowel bar and tie bar baskets must be one of the following:

1. Epoxy-coated wire complying with section 52-2.03B
2. Stainless-steel wire. Wire must be descaled solid stainless-steel. Wire must comply with (1) the chemical requirements in ASTM A 276/A 276M, UNS Designation S31603 or S31803 and (2) the tension requirements in ASTM A 1022/ A 1022M.

Handle epoxy-coated tie bar and dowel bar baskets under ASTM D 3963/D 3963M and either section 52-2.02 or 52-2.03.

Fasteners must be driven fasteners under ASTM F 1667. Fasteners on lean concrete base or HMA must have a minimum shank diameter of 3/16 inch and a minimum shank length of 2-1/2 inches. For asphalt treated permeable base or cement treated permeable base, the shank diameter must be at least 3/16 inch and the shank length must be at least 5 inches.

Fasteners, clips, and washers must have a minimum 0.2-mil thick zinc coating applied by either electroplating or galvanizing.

40-1.02D Dowel Bar Lubricant

Dowel bar lubricant must be petroleum paraffin based or a curing compound. Paraffin-based lubricant must be Dayton Superior DSC BB-Coat or Valvoline Tectyl 506 or an approved equal and must be factory-applied. Curing compound must be curing compound no. 3.

40-1.02E Joint Filler

Joint filler for isolation joint must be preformed expansion joint filler for concrete (bituminous type) under ASTM D 994.

40-1.02F Curing Compound

Curing compound must be curing compound no. 1 or 2.

40-1.02G Nonshrink Hydraulic Cement Grout

Nonshrink hydraulic cement grout must comply with ASTM C 1107/C 1107M. Clean, uniform, rounded aggregate filler may be used to extend the grout. Aggregate filler must not exceed 60 percent of the grout mass or the maximum recommended by the manufacturer, whichever is less. Aggregate filler moisture content must not exceed 0.5 percent when tested under California Test 223 or California Test 226. Aggregate filler tested under California Test 202 must comply with the grading shown in the following table:

Aggregate Filler Grading

Sieve size	Percentage passing
1/2-inch	100
3/8-inch	85–100
No. 4	10–30
No. 8	0–10
No. 16	0–5

40-1.02H Temporary Roadway Pavement Structure

Temporary roadway pavement structure must comply with section 41-1.02E.

40-1.02I–40-1.02N Reserved

40-1.03 CONSTRUCTION

40-1.03A General

Aggregate and bulk cementitious material must be proportioned by weight by means of automatic proportioning devices of approved types.

For widenings and lane reconstruction, construct only the portion of pavement where the work will be completed during the same lane closure. If you fail to complete the construction during the same lane closure, construct a temporary pavement structure under section 41-1.

40-1.03B Water Supply

Before placing concrete pavement, develop enough water supply.

40-1.03C Test Strips

Construct a test strip for each type of pavement with a quantity of more than 2,000 cu yd. Obtain authorization of the test strip before constructing pavement. Test strips must be:

1. 700 to 1,000 feet long
2. Same width as the planned paving, and
3. Constructed using the same equipment proposed for paving

The Engineer selects from 6 to 12 core locations for dowel bars and up to 6 locations for tie bars per test strip. If you use mechanical dowel bar inserters, the test strip must demonstrate they do not leave voids, segregations, or surface irregularities such as depressions, dips, or high areas.

Test strips must comply with the acceptance criteria for:

1. Smoothness, except IP is not required
2. Dowel bars and tie bars placement
3. Pavement thickness
4. Final finishing, except the coefficient of friction is not considered

Allow 3 business days for evaluation. If the test strip is noncompliant, stop paving and submit a plan for changed materials, methods, or equipment. Allow 3 business days for authorization of the plan. Construct another test strip per the authorized plan.

Remove and dispose of noncompliant test strips.

If the test strip is compliant except for smoothness and final finishing, you may grind the surface. After grinding retest the test strip smoothness under section 40-1.01D(6)(c).

If the test strip is compliant for smoothness and thickness, construction of an additional test strip is not required and the test strip may remain in place.

Construct additional test strips if you:

1. Propose different paving equipment including:
   1.1. Paver
   1.2. Dowel bar inserter
   1.3. Tie bar inserter
   1.4. Tining
   1.5. Curing equipment
2. Change concrete mix proportions

You may request authorization to eliminate the test strip if you use paving equipment and personnel from a Department project (1) for the same type of pavement and (2) completed within the past 12 months. Submit supporting documents and previous project information with your request.

40-1.03D Joints

40-1.03D(1) General

Do not bend tie bars or reinforcement in existing concrete pavement joints.

For contraction joints and isolation joints, saw cut a groove with a power-driven saw. After cutting, immediately wash slurry from the joint with water at less than 100 psi pressure.

Keep joints free from foreign material including soil, gravel, concrete, and asphalt. To keep foreign material out of the joint, you may use filler material. Filler material must not react adversely with the concrete or cause concrete pavement damage. After sawing and washing, install filler material that keeps moisture in the adjacent concrete during the 72 hours after paving. If you install filler material, the specifications for spraying the sawed joint with additional curing compound in section 40-1.03K does not apply. If using absorptive filler material, moisten the filler immediately before or after installation.

40-1.03D(2) Construction Joints

Construction joints must be vertical.

Before placing fresh concrete against hardened concrete, existing concrete pavement, or structures, apply curing compound no. 1 or 2 to the vertical surface of the hardened concrete, existing concrete pavement, or structures and allow it to dry.

At joints between concrete pavement and HMA, apply tack coat between the concrete pavement and HMA.

Use a metal or wooden bulkhead to form transverse construction joints. If dowel bars are described, the bulkhead must allow dowel bar installation.

40-1.03D(3) Contraction Joints

Saw contraction joints before cracking occurs and after the concrete is hard enough to saw without spalling, raveling, or tearing.

Saw cut using a power saw with a diamond blade. After cutting, immediately wash slurry from the joint with water at less than 100 psi pressure.

Except for longitudinal joints parallel to a curving centerline, transverse and longitudinal contraction joints must not deviate by more than 0.1 foot from either side of a 12-foot straight line

Cut transverse contraction joints within 0.5 foot of the spacing described. Adjust spacing if needed such that slabs are at least 10 feet long.

For widenings, do not match transverse contraction joints with existing joint spacing or skew unless otherwise described.

Cut transverse contraction joints straight across the full concrete pavement width, between isolation joints and edges of pavement. In areas of converging and diverging pavements, space transverse contraction joints such that the joint is continuous across the maximum pavement width. Longitudinal contraction joints must be parallel with the concrete pavement centerline, except when lanes converge or diverge.

40-1.03D(4) Isolation Joints

Before placing concrete at isolation joints, prepare the existing concrete face and secure joint filler. Prepare by saw cutting and making a clean flat vertical surface. Make the saw cut the same depth as the depth of the new pavement.

40-1.03E Bar Reinforcement

Place bar reinforcement under section 52.

40-1.03F Dowel Bar Placement

If using curing compound as lubricant, apply the curing compound to dowels in 2 separate applications. Lubricate each dowel bar entirely before placement. The last application must be applied not more than 8 hours before placing the dowel bars. Apply each curing compound application at a rate of 1 gallon per 150 square feet.

Install dowel bars using one of the following methods:

1. Drill and bond bars. Comply with section 41-10.
2. Mechanical insertion. Eliminate evidence of the insertion by reworking the concrete over the dowel bars.
3. Dowel bar baskets. Anchor baskets with fasteners. Use at least 1 fastener per foot for basket sections. Baskets must be anchored at least 200 feet in advance of the concrete placement activity unless your waiver request is authorized. If requesting a waiver, describe the construction limitations or restricted access preventing the advanced anchoring. After the baskets are anchored and before the concrete is placed, cut and remove temporary spacer wires and demonstrate the dowel bars do not move from their specified depth and alignment during concrete placement.

If dowel bars are noncompliant, stop paving activities, demonstrate your correction, and obtain verbal approval from the Engineer.

40-1.03G Tie Bar Placement

Install tie bars at longitudinal joints using one of the following methods:

1. Drill and bond bars. Comply with section 41-10.
2. Insert bars. Mechanically insert tie bars into plastic slip-formed concrete before finishing. Inserted tie bars must have full contact between the bar and the concrete. Eliminate evidence of the insertion by reworking the concrete over the tie bars.
3. Threaded couplers. Threaded tie bar splice couplers must be fabricated from deformed bar reinforcement and free of external welding or machining.
4. Tie bar baskets. Anchor baskets at least 200 feet in advance of pavement placement activity. If you request a waiver, describe the construction limitations or restricted access preventing the advanced

anchoring. After the baskets are anchored and before paving, demonstrate the tie bars do not move from their specified depth and alignment during paving. Use fasteners to anchor tie bar baskets.

If tie bars are noncompliant, stop paving activities, demonstrate your correction, and obtain verbal approval from the Engineer.

40-1.03H Placing Concrete

40-1.03H(1) General

Immediately prior to placing concrete, the surface to receive concrete must be:

1. In compliance with specified requirements, including compaction and elevation tolerances
2. Free of loose and extraneous material
3. Uniformly moist, but free of standing or flowing water

Place concrete pavement with stationary side forms or slip-form paving equipment.

Place consecutive concrete loads within 30 minutes of each other. Construct a transverse construction joint when concrete placement is interrupted by more than 30 minutes. The transverse construction joint must coincide with the next contraction joint location, or you must remove fresh concrete pavement to the preceding transverse joint location.

Place concrete pavement in full slab widths separated by construction joints or monolithically in multiples of full lane widths with a longitudinal contraction joint at each traffic lane line.

Do not retemper concrete.

If the concrete pavement surface width is constructed as specified, you may construct concrete pavement sides on a batter not flatter than 6:1 (vertical:horizontal).

40-1.03H(2) Paving Adjacent to Existing Concrete Pavement

Where pavement is placed adjacent to existing concrete pavement:

1. Grinding adjacent pavement must be completed before placing the pavement
2. Use paving equipment with padded crawler tracks or rubber-tired wheels with enough offset to prevent damage
3. Match pavement grade with the elevation of existing concrete pavement after grinding.

40-1.03H(3) Concrete Pavement Transition Panel

For concrete pavement placed in a transition panel, texture the surface with a drag strip of burlap, broom, or spring steel tine device that produces scoring in the finished surface. Scoring must be either parallel or transverse to the centerline. Texture at the time that produces the coarsest texture.

40-1.03H(4) Stationary Side Form Construction

Stationary side forms must be straight and without defects including warps, bends, and indentations. Side forms must be metal except at end closures and transverse construction joints where other materials may be used.

You may build up side forms by attaching a section to the top or bottom. If attached to the top of metal forms, the attached section must be metal.

The side form's base width must be at least 80 percent of the specified concrete pavement thickness.

Side forms including interlocking connections with adjoining forms must be rigid enough to prevent springing from subgrading and paving equipment and concrete pressure.

Construct subgrade to final grade before placing side forms. Side forms must bear fully on the foundation throughout their length and base width. Place side forms to the specified grade and alignment of the finished concrete pavement's edge. Support side forms during concrete placing, compacting, and finishing.

After subgrade work is complete and immediately before placing concrete, true side forms and set to line and grade for a distance that avoids delays due to form adjustment.

Clean and oil side forms before each use.

Side forms must remain in place for at least 1 day after placing concrete and until the concrete pavement edge no longer requires protection from the forms.

Spread, screed, shape, and consolidate concrete with 1 or more machines. The machines must uniformly distribute and consolidate the concrete. The machines must operate to place the concrete pavement to the specified cross section with minimal hand work.

Consolidate the concrete without segregation. If vibrators are used:

1. The vibration rate must be at least 3,500 cycles per minute for surface vibrators and 5,000 cycles per minute for internal vibrators
2. Amplitude of vibration must cause perceptible concrete surface movement at least 1 foot from the vibrating element
3. Use a calibrated tachometer for measuring frequency of vibration
4. Vibrators must not rest on side forms or new concrete pavement
5. Power to vibrators must automatically cease when forward or backward motion of the paving machine is stopped
6. Uniformly consolidate the concrete across the paving width including adjacent to forms by using high-frequency internal vibrators within 15 minutes of depositing concrete on the subgrade
7. Do not shift the mass of concrete with vibrators.

40-1.03H(5) Slip-Form Construction

If slip-form construction is used, spread, screed, shape, and consolidate concrete to the specified cross section with slip-form machines and minimal hand work. Slip-form paving machines must be equipped with traveling side forms and must not segregate the concrete.

Do not deviate from the specified concrete pavement alignment by more than 0.1 foot.

Slip-form paving machines must use high frequency internal vibrators to consolidate concrete. You may mount vibrators with their axes parallel or normal to the concrete pavement alignment. If mounted with axes parallel to the concrete pavement alignment, space vibrators no more than 2.5 feet measured center to center. If mounted with axes normal to the concrete pavement alignment, space the vibrators with a maximum 0.5-foot lateral clearance between individual vibrators.

Each vibrator must have a vibration rate from 5,000 to 8,000 cycles per minute. The amplitude of vibration must cause perceptible concrete surface movement at least 1 foot from the vibrating element. Use a calibrated tachometer to measure frequency of vibration.

40-1.03I Edge Treatment

Construct edge treatments as shown. Regrade when required for the preparation of safety edge areas.

Sections 40-1.03J(2) and 40-1.03J(3) do not apply to safety edges.

For safety edges placed after the concrete pavement is complete, concrete may comply with the requirements for minor concrete.

For safety edges placed after the concrete pavement is complete, install connecting bar reinforcement under section 52.

Saw cutting or grinding may be used to construct safety edges.

For safety edges, the angle of the slope must not deviate by more than ± 5 degrees from the angle shown. Measure the angle from the plane of the adjacent finished pavement surface.

40-1.03J Finishing

40-1.03J(1) General

Reserved

40-1.03J(2) Preliminary Finishing

40-1.03J(2)(a) General

Preliminary finishing must produce a smooth and true-to-grade finish. After preliminary finishing, mark each day's paving with a stamp. The stamp must be authorized before paving starts. The stamp must be approximately 1 by 2 feet in size. The stamp must form a uniform mark from 1/8 to 1/4 inch deep. Locate the mark 20 ± 5 feet from the transverse construction joint formed at each day's start of paving and 1 ± 0.25 foot from the pavement's outside edge. The stamp mark must show the month, day, and year of placement and the station of the transverse construction joint. Orient the stamp mark so it can be read from the pavement's outside edge.

Do not apply water to the pavement surface before float finishing.

40-1.03J(2)(b) Stationary Side Form Finishing

If stationary side form construction is used, give the pavement a preliminary finish by the machine float method or the hand method.

If using the machine float method:

1. Use self-propelled machine floats.
2. Determine the number of machine floats required to perform the work at a rate equal to the pavement delivery rate. If the time from paving to machine float finishing exceeds 30 minutes, stop pavement delivery. When machine floats are in proper position, you may resume pavement delivery and paving.
3. Run machine floats on side forms or adjacent pavement lanes. If running on adjacent pavement, protect the adjacent pavement surface under section 40-1.03L. Floats must be hardwood, steel, or steel-shod wood. Floats must be equipped with devices that adjust the underside to a true flat surface.

If using the hand method, finish pavement smooth and true to grade with manually operated floats or powered finishing machines.

40-1.03J(2)(c) Slip-Form Finishing

If slip-form construction is used, the slip-form paver must give the pavement a preliminary finish. You may supplement the slip-form paver with machine floats.

Before the pavement hardens, correct pavement edge slump in excess of 0.02 foot exclusive of edge rounding.

40-1.03J(3) Final Finishing

After completing preliminary finishing, round the edges of the initial paving widths to a 0.04-foot radius. Round transverse and longitudinal construction joints to a 0.02-foot radius.

Before curing, texture the pavement. Perform initial texturing with a burlap drag or broom device that produces striations parallel to the centerline. Perform final texturing with a steel-tined device that produces grooves parallel with the centerline.

Construct longitudinal grooves with a self-propelled machine designed specifically for grooving and texturing pavement. The machine must have tracks to maintain constant speed, provide traction, and maintain accurate tracking along the pavement surface. The machine must have a single row of rectangular spring steel tines. The tines must be from 3/32 to 1/8 inch wide, on 3/4-inch centers, and must have enough length, thickness, and resilience to form grooves approximately 3/16 inch deep. The machine must have horizontal and vertical controls. The machine must apply constant down pressure on the pavement surface during texturing. The machines must not cause raveling.

Construct grooves over the entire pavement width in a single pass except do not construct grooves 3 inches from the pavement edges and longitudinal joints. Final texture must be uniform and smooth. Use a guide to properly align the grooves. Grooves must be parallel and aligned to the pavement edge across the pavement width. Grooves must be from 1/8 to 3/16 inch deep after the pavement has hardened.

For irregular areas and areas inaccessible to the grooving machine, you may hand-construct grooves using the hand method. Hand-constructed grooves must comply with the specifications for machine-constructed grooves.

For ramp termini, use heavy brooming normal to the ramp centerline to produce a coefficient of friction of at least 0.35 determined on the hardened surface under California Test 342.

40-1.03K Curing

Cure the concrete pavement's exposed area under section 90-1.03B using the waterproof membrane method or curing compound method. If using the curing compound method use curing compound no. 1 or 2. When side forms are removed within 72 hours of the start of curing, also cure the concrete pavement edges.

Apply curing compound with mechanical sprayers. Reapply curing compound to saw cuts and disturbed areas.

40-1.03L Protecting Concrete Pavement

Protect concrete pavement under section 90-1.03C.

Maintain the concrete pavement surface temperature at not less than 40 degrees F for the initial 72 hours.

Protect the concrete pavement surface from activities that cause damage and reduce texture and coefficient of friction. Do not allow soil, gravel, petroleum products, concrete, or asphalt mixes on the concrete pavement surface.

Construct crossings for traffic convenience. If authorized, you may use RSC for crossings. Do not open crossings until the Department determines that the pavement's modulus of rupture is at least 550 psi under California Test 523 or California Test 524.

Do not open concrete pavement to traffic or use equipment on the concrete pavement for 10 days after paving nor before the concrete has attained a modulus of rupture of 550 psi based on Department's testing except:

1. If the equipment is for sawing contraction joints
2. If authorized, one side of paving equipment's tracks may be on the concrete pavement after a modulus of rupture of 350 psi has been attained, provided:
   - 2.1. Unit pressure exerted on the concrete pavement by the paver does not exceed 20 psi
   - 2.2. You change the paving equipment tracks to prevent damage or the paving equipment tracks travel on protective material such as planks
   - 2.3. No part of the track is closer than 1 foot from the concrete pavement's edge

If concrete pavement damage including visible cracking occurs, stop operating paving equipment on the concrete pavement and repair the damage.

40-1.03M Early Use of Concrete Pavement

If requesting early use of concrete pavement:

1. Furnish molds and machines for modulus of rupture testing
2. Sample concrete
3. Fabricate beam specimens
4. Test for modulus of rupture under California Test 523

If you request early use, concrete pavement must have a modulus of rupture of at least 350 psi. Protect concrete pavement under section 40-1.03L.

40-1.03N Reserved

40-1.03O Shoulder Rumble Strip

40-1.03O(1) General

Construct shoulder rumble strips by rolling or grinding indentations in new concrete pavement.

Do not construct shoulder rumble strips on structures or approach slabs.

Construct rumble strips within 2 inches of the specified alignment. Rumble strip equipment must be equipped with a sighting device enabling the operator to maintain the rumble strip alignment.

Indentations must not vary from the specified dimensions by more than 1/16 inch in depth nor more than 10 percent in length and width.

Grind or remove and replace noncompliant rumble strip indentations at locations determined by the Engineer. Ground surface areas must be neat and uniform in appearance.

Remove grinding residue under section 42-1.03B.

40-1.03O(2) Rolled-In Indentations

Construct rolled-in indentations before final concrete set. Indentation construction must not displace adjacent concrete.

40-1.03O(3) Ground-In Indentations

Concrete pavement must be hardened before grinding rumble strips indentations. Do not construct indentations until the following occurs:

1. 10 days elapse after concrete placement
2. Concrete has developed a modulus of rupture of 550 psi determined under California Test 523,

40-1.03P Drilling Cores

Drill concrete pavement cores under ASTM C 42/C 42M. Use diamond impregnated drill bits.

Clean, dry, and fill core holes with hydraulic cement grout (nonshrink) or pavement concrete. Coat the core hole walls with epoxy adhesive for bonding new concrete to old concrete under section 95. Finish the backfill to match the adjacent surface elevation and texture.

40-1.03Q Pavement Repair and Replacement

40-1.03Q(1) General

If surface raveling or full-depth cracks occur within one year of Contract acceptance, repair or replace the pavement under section 6-3.06.

Repair and replace pavement in the following sequence:

1. Replace pavement
2. Repair spall, ravel, and working cracks
3. Correct smoothness and coefficient of friction
4. Treat partial depth cracks
5. Replace damaged joint seals under section 41-5

In addition to removing pavement for other noncompliance, remove and replace JPCP slabs that:

1. Have one or more full depth crack
2. Have raveled surfaces such that either:
   2.1. Combined raveled areas are more than 5 percent of the total slab area
   2.2. Single area is more than 4 sq ft

Remove and replace JPCP 3 feet on both sides of a joint with a rejected dowel bar.

40-1.03Q(2) Spall and Ravel Repair

Repair spalled or raveled areas that are:

1. Deeper than 0.05 foot
2. Wider than 0.10 foot
3. Longer than 0.3 foot

Repairs must comply with section 41-4 and be completed before opening pavement to traffic.

40-1.03Q(3) Crack Repair

Treat partial depth cracks for JPCP under section 41-3.

If the joints are sealed, repair working cracks by routing and sealing. Use a powered rotary router mounted on wheels, with a vertical shaft and a routing spindle that casters as it moves along the crack. Form a reservoir 3/4 inch deep by 3/8 inch wide in the crack. Equipment must not cause raveling nor spalling

Treat the contraction joint adjacent to the working crack by either:

1. Epoxy resin under ASTM C 881/C 881M, Type IV, Grade 2
2. Pressure injecting epoxy resin under ASTM C 881/C881M, Type IV, Grade 1

40-1.03Q(4) Smoothness and Friction Correction

Correct pavement that is noncompliant for:

1. Smoothness by grinding under section 42-3
2. Coefficient of friction by grooving or grinding under section 42

Do not start corrective work until:

1. Pavement has cured 10 days
2. Pavement has at least a 550 psi modulus of rupture
3. Your corrective method is authorized

Correct the entire lane width. Begin and end grinding at lines perpendicular to the roadway centerline. The corrected area must have a uniform texture and appearance.

If corrections are made within areas where testing with an IP is required, retest the entire lane length with an IP under sections 40-1.01D(6)(c) and 40-1.01D(7)(b)(vii).

If corrections are made within areas where testing with a 12-foot straightedge is required, retest the corrected area with a straightedge under sections 40-1.01D(6)(c) and 40-1.01D(7)(b)(vii).

Allow 25 days for the Department's coefficient of friction retesting.

40-1.03R–40-1.03U Reserved

40-1.04 PAYMENT

The payment quantity for pavement is based on the dimensions shown.

The deduction for pavement thickness deficiency in each primary area is shown in the following table:

Deduction for Thickness Deficiency

Average thickness deficiency (foot)[a]	Deduction($/sq yd)
0.01	0.90
0.02	2.30
0.03	4.10
0.04	6.40
0.05	9.11

[a]Values greater than 0.01 are rounded to the nearest 0.01 foot.

Shoulder rumble strips are measured by the station along each shoulder on which the rumble strips are constructed without deductions for gaps between indentations.

If the initial cores show that dowel bars or tie bars are within alignment tolerances and the Engineer orders more dowel or tie bar coring, the additional cores are paid for as change order work.

The Department does not pay for additional coring to check dowel or tie bar alignment which you request.

If the Engineer accepts a test strip and it remains as part of the paving surface, the test strip is paid for as the type of pavement involved.

If the curvature of a slab affects tie bar spacing and additional tie bars are required, no additional payment is made for the additional tie bars.

Payment for grinding existing pavement is not included in the payment for the type of pavement involved.

40-2 CONTINUOUSLY REINFORCED CONCRETE PAVEMENT

40-2.01 GENERAL

40-2.01A Summary

Section 40-2 includes specifications for constructing CRCP.

Terminal joints include saw cutting, dowel bars, drill and bond dowel bars, support slab, support slab reinforcement, tack coat, and temporary hot mix asphalt.

Expansion joints include polystyrene, support slab, support slab reinforcement, dowel bars, drill and bond dowel bars, and bond breaker.

Wide flange beam terminals include polyethylene foam, support slab, and support slab reinforcement.

Pavement anchors include cross drains, anchor reinforcement, filter fabric, and permeable material.

40-2.01B Definitions

Reserved

40-2.01C Submittals

Reserved

40-2.01D Quality Control and Assurance

40-2.01D(1) General

Reserved

40-2.01D(2) Testing for Coefficient of Thermal Expansion

For field qualification, test coefficient of thermal expansion under AASHTO T 336. The coefficient of thermal expansion must not exceed 6.0 microstrain/degree Fahrenheit.

40-2.02 MATERIALS

40-2.02A General

Class 1 permeable material, filter fabric, and slotted plastic pipe cross drain as shown for pavement anchors must comply with section 68-3.

40-2.02B Concrete

Concrete for terminal joints, support slabs, and pavement anchors must comply with section 40-1.02.

40-2.02C Transverse Bar Assembly

Instead of transverse bar and other support devices, you may use transverse bar assemblies to support longitudinal bar. Bar reinforcement and wire must comply with section 40-1.02C.

40-2.02D Wide Flange Beam

Wide flange beams and studs must be either rolled structural steel shapes under ASTM A 36/A 36M or structural steel under ASTM A 572/A 572M.

40-2.02E Joints

Joint seals for wide flange beam terminals must comply with section 51-2.02.

Joint seals for transverse expansion joints must comply with section 51-2.02.

Expanded polystyrene for transverse expansion joints must comply with section 51-2.01B(1).

40-2.03 CONSTRUCTION

40-2.03A General

Reserved

40-2.03B Test Strips

Comply with section 40-1.03C except during the evaluation, the Engineer visually checks reinforcement, dowel and tie bar placement.

40-2.03C Construction Joints

Transverse construction joints must be perpendicular to the lane line. Construct joints to allow for lap splices of the longitudinal bar. Comply with the lap splice lengths shown for CRCP.

Clean construction joint surfaces before placing fresh concrete against the joint surfaces. Remove surface laitance, curing compound, and other foreign materials.

40-2.03D Bar Reinforcement

Place bar reinforcement under section 52-1.03D, except you may request to use plastic chairs. Plastic chairs will only be considered for support directly under the transverse bars. Your request to use plastic chairs must include a sample of the plastic chair, the manufacturer's written recommendations for the applicable use and load capacity, chair spacing, and your calculation for the load on a chair for the area of bar reinforcement sitting on it. Vertical and lateral stability of the bar reinforcement and plastic chairs must be demonstrated during construction of the test strip. Obtain authorization before using the proposed plastic chairs for work after the test strip is accepted.

For transverse bar in a curve with a radius under 2,500 feet, place the reinforcement in a single continuous straight line across the lanes and aligned with the radius point as shown.

40-2.03E Wide Flange Beams

Weld stud ends with an electric arc welder completely fusing the studs to the wide flange beam. Replace studs dislodged in shipping or that can be dislodged with a hammer.

40-2.03F Repair and Replacement

40-2.03F(1) General

Requirements for repair of cracks under section 40-1.03Q do not apply to CRCP. High molecular weight methacrylate is not to be applied to cracks in CRCP.

New CRCP will be monitored for 1 year from contract acceptance or relief from maintenance, whichever is less. CRCP that develops raveling areas of 6 inches by 6 inches or greater will require partial depth repair under section 6-3.06. CRCP that develops one or more full-depth transverse cracks with faulting greater than 0.25 inch or one or more full-depth longitudinal cracks with faulting greater 0.50 inch will require full depth repair.

40-2.03F(2) Partial Depth Repair

Partial depth repair must comply with section 41-4 except:

1. Determine a rectangular boundary which extends 6 inches beyond the damaged area. The limits of saw depth must be between 2 inches from the surface to 1/2 inch above the longitudinal bars.
2. If each length of the repair boundaries is equal to or greater than 3 ft, additional reinforcement is needed for the repair area. Submit a plan for authorization before starting the repair.

40-2.03F(3) Full Depth Repair

40-2.03F(3)(a) General

Removal of CRCP must be full depth except for portion of reinforcement to remain. Provide continuity of reinforcement. Comply with section 52-6. Submit a plan for authorization, before starting the repair. Do not damage the base, concrete and reinforcement to remain. Place concrete in the removal area.

40-2.03F(3)(b) Transverse Cracks

Make initial full-depth transverse saw cuts normal to the lane line a distance of 3 feet on each side of the transverse crack.

40-2.03F(3)(c) Longitudinal Cracks

Remove the cracked area normal to the lane line for the full width of the lane a distance of 1 foot beyond the ends of the crack. You may propose alternate limits with your repair plan for authorization.

40-2.03G Reserved

40-2.04 PAYMENT

Not Used

40-3 RESERVED

40-4 JOINTED PLAIN CONCRETE PAVEMENT

40-4.01 GENERAL

40-4.01A Summary

Section 40-4 includes specifications for constructing JPCP.

40-4.01B Definitions

Reserved

40-4.01C Submittals

40-4.01C(1) General

Reserved

40-4.01C(2) Early Age Crack Mitigation System

At least 24 hours before each paving shift, submit the following information as an informational submittal:

1. Early age stress and strength predictions
2. Scheduled sawing and curing activities
3. Contingency plan if cracking occurs

40-4.01C(3)–40-4.01C(8) Reserved

40-4.01D Quality Control and Assurance

40-4.01D(1) General

Reserved

40-4.01D(2) Quality Control Plan

The QC plan must include a procedure for identifying transverse contraction joint locations relative to the dowel bars longitudinal center and a procedure for consolidating concrete around the dowel bars.

40-4.01D(3) Early Age Crack Mitigation System

For JPCP, develop and implement a system for predicting stresses and strength during the initial 72 hours after paving. The system must include:

1. Subscription to a weather service to obtain forecasts for wind speed, ambient temperatures, humidity, and cloud cover
2. Portable weather station with an anemometer, temperature and humidity sensors, located at the paving site
3. Early age concrete pavement stress and strength prediction plan
4. Analyzing, monitoring, updating, and reporting the system's predictions

40-4.01D(4)–40-4.01D(9) Reserved

40-4.02 MATERIALS

Not Used

40-4.03 CONSTRUCTION

40-4.03A General

Transverse contraction joints on a curve must be on a single straight line through the curve's radius point. If transverse joints do not align in a curve, drill a full depth 2” diameter hole under ASTM C 42/C 42M where the joint meets the adjacent slab. Fill the hole with joint filler. If joints are not sealed, avoid joint filler material penetration into the joint.

40-4.03B Repair and Replacement

If replacing concrete, saw cut and remove to full depth.

Saw cut full slabs at the longitudinal and transverse joints. Saw cut partial slabs at joints and at locations determined by the Engineer. Saw cut must be vertical.

After lifting the slab, paint the cut ends of dowels and tie bars.

Construct transverse and longitudinal construction joints between the new slab and existing concrete. If slabs are constrained at both longitudinal edges by existing pavement, use dowel bars instead of tie bars. For longitudinal joints, offset dowel bar holes from original tie bars by 3 inches. For transverse joints, offset dowel bar holes from the original dowel bar by 3 inches.

Drill and bond bars to the existing concrete. Comply with section 41-10. Clean the faces of joints and underlying base from loose material and contaminants. Coat the faces with a double application of pigmented curing compound under section 28-2.03F. For partial slab replacements, place preformed sponge rubber expansion joint filler at new transverse joints under ASTM D 1752. Place concrete in the removal area.

40-4.03C–40-4.03G Reserved

40-4.04 PAYMENT

Not Used

40-5 JOINTED PLAIN CONCRETE PAVEMENT WITH RAPID STRENGTH CONCRETE

Reserved

40-6–40-15 RESERVED

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41 CONCRETE PAVEMENT REPAIR

04-18-14

Replace the headings and paragraphs in section 41 with:

07-19-13

41-1 GENERAL

41-1.01 GENERAL

41-1.01A Summary

Section 41-1 includes general specifications for repairing concrete pavement.

Dowel bars must comply with section 40-1.

41-1.01B Definitions

Reserved

41-1.01C Submittals

At least 15 days before delivering fast-setting concrete, polyester resin binder, or bonding agent to the job site, submit the manufacturer’s recommendations, instructions, and MSDS. Notify the Engineer if polyester resin binder will be stored in containers over 55 gallons.

41-1.01D Quality Control and Assurance

41-1.01D(1) General

Before using polyester concrete, allow 14 days for sampling and testing of the polyester resin binder.

41-1.01D(2) Reserved

41-1.02 MATERIALS

41-1.02A General

Water for washing aggregates, mixing concrete, curing, and coring must comply with section 90-1.02D.

Use the minimum amount of water to produce workable concrete and comply with the manufacturer's instructions.

41-1.02B Fast-Setting Concrete

Fast-setting concrete must be one of the following:

1. Magnesium phosphate concrete that is either:
   1.1. Single component water activated
   1.2. Dual component with a prepackaged liquid activator
2. Modified high-alumina based concrete
3. Portland cement based concrete

Fast-setting concrete must be stored in a cool and dry environment.

If used, the addition of retarders must comply with the manufacturer's instructions.

You may use any accelerating chemical admixtures complying with ASTM C494/C494M, Type C and section 90-1.02E.

Fast-setting concrete properties must have the values shown in the following table:

Fast-Setting Concrete

Property	Test method	Value
Compressive strength[a] (psi, min)		
at 3 hours	California Test 551	3,000
at 24 hours	California Test 551	5,000
Flexural strength[a] (psi, min, at 24 hours)	California Test 551	500
Bond strength[a] (psi, min, at 24 hours)		
Saturated surface dry concrete	California Test 551	300
Dry concrete	California Test 551	400
Water absorption (%, max)	California Test 551	10
Abrasion resistance[a] (g, max, at 24 hours)	California Test 550	25
Drying shrinkage (%, max, at 4 days)	ASTM C596	0.13
Water soluble chlorides[b] (%, max, by weight)	California Test 422	0.05
Water soluble sulfates[b] (%, max, by weight)	California Test 417	0.25
Thermal stability (%, min)	California Test 553	90

[a]Perform test with aggregate filler if used.
[b]Test must be performed on a cube specimen, fabricated under California Test 551, cured at least 14 days, and then pulverized to 100% passing the no. 50 sieve.

Aggregate filler may be used to extend prepackaged concrete. Aggregate filler must:

1. Be clean and uniformly rounded.
2. Have a moisture content of 0.5-percent by weight or less when tested under California Test 226.
3. Comply with sections 90-1.02C(2) and 90-1.02C(3).
4. Not exceed 50 percent of the concrete volume or the maximum recommended by the fast-setting concrete manufacturer, whichever is less.

When tested under California Test 202, aggregate filler must comply with the grading in the following table:

Aggregate Filler Grading

Sieve size	Percentage passing
3/8 inch	100
No. 4	50–100
No. 16	0–5

41-1.02C Polyester Concrete

Polyester concrete consists of polyester resin binder and dry aggregate. The polyester resin binder must be an unsaturated isophthalic polyester-styrene copolymer.

Polyester resin binder properties must have the values shown in the following table:

Polyester Resin Binder

Property	Test method	Value
Viscosity[a] (Pa·s) RVT, No. 1 spindle, 20 RPM at 77 °F	ASTM D2196	0.075–0.200
Specific gravity[a] (77 °F)	ASTM D1475	1.05–1.10
Elongation (%, min) Type I specimen, 0.25 ± 0.03 inch thick Speed of testing = 0.45 inch/minute Condition 18/25/50+5/70: T—23/50	ASTM D638 ASTM D618	35
Tensile strength (psi, min) Type I specimen, 0.25 ± 0.03 inch thick Speed of testing = 0.45 inch/minute Condition 18/25/50+5/70: T—23/50	ASTM D638 ASTM D618	2,500
Styrene content[a] (%, by weight)	ASTM D2369	40–50
Silane coupler (%, min, by weight of polyester resin binder)	--	1.0
PCC saturated surface-dry bond strength at 24 hours and 70 ± 2 °F (psi, min)	California Test 551	500
Static volatile emissions[a] (g/sq m, max)	South Coast Air Quality Management District, Method 309-91[b]	60

[a]Perform the test before adding initiator.
[b]For the test method, go to:
http://www.aqmd.gov/tao/methods/lab/309-91.pdf

Silane coupler must be an organosilane ester, gamma-methacryloxypropyltrimethoxysilane. Promoter must be compatible with suitable methyl ethyl ketone peroxide (MEKP) and cumene hydroperoxide (CHP) initiators.

Aggregate for polyester concrete must comply with section 90-1.02C(1), 90-1.02C(2), and 90-1.02C(3).

When tested under California Test 202, the combined aggregate grading must comply with one of the gradations in the following table:

Combined Aggregate Grading

Sieve size	Percentage passing		
	A	B	C
1/2"	100	100	100
3/8"	83–100	100	100
No. 4	65–82	62–85	45–80
No. 8	45–64	45–67	35–67
No. 16	27–48	29–50	25–50
No. 30	12–30	16–36	15–36
No. 50	6–17	5–20	5–20
No. 100	0–7	0–7	0–9
No. 200	0–3	0–3	0–6

Aggregate retained on the no. 8 sieve must have a maximum of 45 percent crushed particles under California Test 205. Fine aggregate must be natural sand.

The weighted average absorption must not exceed 1 percent when tested under California Tests 206 and 207.

You may submit an alternative grading or request to use manufactured sand as fine aggregate but 100 percent of the combined grading must pass the 3/8 inch sieve. Allow 21 days for authorization.

Polyester concrete must have a minimum compressive strength of 1250 psi at 3 hours and 30 minutes under California Test 551 or ASTM C109.

41-1.02D Bonding Agent

Bonding agent must comply with the concrete manufacturer's recommendations.

41-1.02E Temporary Pavement Structure

Temporary pavement structure consists of RSC or aggregate base with HMA. RSC not conforming to the specifications may serve as temporary pavement structure if:

1. The modulus of rupture is at least 200 psi before opening to traffic
2. RSC thickness is greater than or equal to the existing concrete pavement surface layer
3. RSC is replaced during the next paving shift

Aggregate base for temporary pavement structure must comply with the 3/4-inch maximum grading specified in section 26-1.02B.

04-18-14

HMA must comply with the specifications for minor HMA in section 39.

07-19-13

41-1.02F Reserved

41-1.03 CONSTRUCTION

41-1.03A General

Repair only the portion of pavement where the work will be completed during the same lane closure. If removal is required, remove only the portion of pavement where the repair will be completed during the same traffic closure. Completion of concrete repair includes curing until the concrete attains the specified minimum properties required before opening the repaired pavement to traffic.

If you fail to complete the concrete pavement repair during the same lane closure, construct temporary pavement before opening the lane to traffic.

Before starting repair work, except saw cutting: the equipment, materials, and personnel for constructing temporary pavement structure must be at the job site or an approved location. If HMA can be delivered to the job site within 1 hour, you may request 1-hour delivery as an alternative to having the HMA at the job site.

Maintain the temporary pavement structure and replace it as a first order of work as soon as you resume concrete pavement repair work.

After removing temporary pavement structure, you may stockpile that aggregate base at the job site and reuse it for temporary pavement structure.

41-1.03B Mixing and Applying Bonding Agent

Mix and apply the bonding agent at the job site under the manufacturer's instructions and in small quantities.

Apply bonding agent after cleaning the surface and before placing concrete.

Apply a thin, even coat of bonding agent with a stiff bristle brush until the entire repair surface is scrubbed and coated with bonding agent.

41-1.03C Mixing Concrete

41-1.03C(1) General

Mix concrete in compliance with the manufacturer's instructions. For repairing spalls, mix in a small mobile drum or paddle mixer. Comply with the manufacturer's recommended limits for the quantity of aggregate filler, water, and liquid activator.

Mix the entire contents of prepackaged dual-component magnesium phosphate concrete as supplied by the manufacturer. Use the full amount of each component and do not add water to dual-component magnesium phosphate concrete.

Magnesium phosphate concrete must not be mixed in containers or worked with tools containing zinc, cadmium, aluminum, or copper.

Modified high-alumina based concrete must not be mixed in containers or worked with tools containing aluminum.

41-1.03C(2) Polyester Concrete

When mixing with resin, the moisture content of the combined aggregate must not exceed 1/2 of the average aggregate absorption when tested under California Test 226.

Proportion the polyester resin and aggregate to produce a mixture with suitable workability for the intended work. Only a minimal amount of resin may rise to the surface after finishing.

41-1.03D Placing Concrete

The pavement surface temperature must be at least 40 degrees F before placing concrete. You may propose methods to heat the surfaces.

Place magnesium phosphate concrete on a dry surface.

Place portland cement and modified high-alumina concrete on surfaces treated with a bonding agent recommended by the concrete manufacturer. If no bonding agent is recommended by the manufacturer, place concrete on damp surfaces that are not saturated.

Do not retemper concrete. Use dry finishing tools cleaned with water before working the concrete.

41-1.03E Curing Concrete

Cure concrete under the manufacturer's instructions. When curing compound is used, comply with section 90-1.03B for curing compound no. 1 or 2.

41-1.03F Reserved

41-1.04 PAYMENT

Not Used

41-2 SUBSEALING AND JACKING

41-2.01 GENERAL

41-2.01A Summary

Section 41-2 includes specifications for filling voids under existing concrete pavement.

41-2.01B Definitions

Reserved

41-2.01C Submittals

Submit shipping invoices with packaged or bulk fly ash and cement.

Before grouting activities begin, submit a proposal for the materials to be used. Include authorized laboratory test data for the grout indicating:

1. Time of initial setting under ASTM C266.
2. Compressive strength results at 1, 3, and 7 days for 10, 12, and 14-second grout efflux times.

If requesting a substitution of grout materials, submit a proposal that includes test data.

41-2.01D Quality Control and Assurance

Reserved

41-2.02 MATERIALS

41-2.02A General

Reserved

41-2.02B Grout

Grout must consist of Type II portland cement, fly ash, and water. Use from 2.4 to 2.7 parts fly ash to 1 part portland cement by weight. Use enough water to produce the following grout efflux times determined under California Test 541, Part D:

1. From 10 to 16 seconds for subsealing
2. From 10 to 26 seconds for jacking

Cement for grout must comply with the specifications for Type II portland cement in section 90-1.02B(2).

Fly ash must comply with AASHTO M 295, Class C or Class F. Fly ash sources must be on the Authorized Material List.

You may use chemical admixtures and calcium chloride. Chemical admixtures must comply with section 90-1.02E(2). Calcium chloride must comply with ASTM D98.

Test grout compressive strength under California Test 551, Part 1 at 7-days with 12 seconds efflux time. Follow the procedures for moist cure. The 7-day compressive strength must be at least 750 psi.

41-2.02C Mortar

Mortar must be a prepackaged fast-setting mortar that complies with ASTM C928.

41-2.02D Reserved

41-2.03 CONSTRUCTION

41-2.03A General

Drill holes in the pavement, inject grout, plug the holes, and finish the holes with mortar.

Drill holes through the pavement and underlying base to a depth from 15 to 18 inches below the pavement surface. The hole diameter must match the fitting for the grout injecting equipment.

41-2.03B Injecting Grout

41-2.03B(1) General

Inject grout within 2 days of drilling holes.

Immediately before injecting grout, clean the drilled holes with water at a minimum pressure of 40 psi. The cleaning device must have at least 4 jets that direct water horizontally at the slab-base interface.

Do not inject grout if the atmospheric or subgrade temperature is below 40 degrees F. Do not inject grout in inclement weather. If water is present in the holes, obtain the Engineer's authorization before injecting grout.

Do not inject grout until at least 2 consecutive slabs requiring subsealing are drilled ahead of the grouting activities.

The grout plant must have a positive displacement cement injection pump and a high-speed colloidal mixer capable of operating from 800 to 2,000 rpm. The injection pump must sustain 150 psi if pumping grout with a 12-second efflux time. A pressure gauge must be located immediately adjacent to the supply valve of the grout hose supply valve and positioned for easy monitoring.

Before mixing, weigh dry cement and fly ash if delivered in bulk. If the materials are packaged, each container must weigh the same.

Introduce water to the mixer through a meter or scale.

Inject grout under pressure until the voids under the pavement slab are filled. The injection nozzle must not leak. Do not inject grout if the nozzle is below the bottom of the slab. Inject grout 1 hole at a time.

Stop injecting grout in a hole if either:

1. Grout does not flow under a sustained pump gauge pressure of 150 psi after 7 seconds and there is no indication the slab is moving.
2. Injected grout rises to the surface at any joint or crack, or flows into an adjacent hole.

Dispose of unused grout within 1 hour of mixing.

41-2.03B(2) Subsealing

If a slab raises more than 1/16 inch due to grout injection, stop injecting grout in that hole.

41-2.03B(3) Jacking

The positive displacement pump used for grout injection must be able to provide a sustained gauge pressure of 200 psi. Gauge pressures may be from 200 to 600 psi for brief periods to start slab movement.

You may add additional water to initiate pressure injection of grout. Do not reduce the grout efflux time below 10 seconds.

Raise the slabs uniformly. Use string lines to monitor the pavement movement.

Do not move adjacent slabs not specified for pavement jacking. If you move adjacent slabs, correct the grade within the tolerances for final pavement elevation.

41-2.03B(4) Finishing

Immediately after removing the injection nozzle, plug the hole with a round, tapered wooden plug. Do not remove plugs until adjacent holes are injected with grout and no grout surfaces through previously injected holes.

After grouting, remove grout from drilled holes at least 4 inches below the pavement surface. Clean holes and fill with mortar. Finish filled holes flush with the pavement surface.

41-2.03B(5) Tolerances

The final pavement elevation must be within 0.01 foot of the required grade. If the final pavement elevation is between 0.01 and 0.10 foot higher than the required grade, grind the noncompliant pavement surface under section 42 to within 0.01 foot of the required grade.

If the final pavement elevation is higher than 0.10 foot from the required grade, remove and replace the noncompliant pavement under section 41-9.

41-2.04 PAYMENT

The payment quantity for subsealing is calculated by adding the dry weight of cement and fly ash used for the placed grout. The payment quantity for jacking is calculated by adding the dry weight of cement and fly ash used for the placed grout.

The Department does not pay for wasted grout.

The Department does not adjust the unit price for an increase or decrease in the subsealing quantity.

The Department does not adjust the unit price for an increase or decrease in the jacking quantity.

41-3 CRACK TREATMENT

41-3.01 GENERAL

41-3.01A Summary

Section 41-3 includes specifications for applying high-molecular-weight methacrylate (HMWM) to concrete pavement surface cracks that do not extend the full slab depth.

41-3.01B Definitions

Reserved

41-3.01C Submittals

41-3.01C(1) General

Submit HMWM samples 20 days before use.

If sealant is to be removed, submit the proposed removal method at least 7 days before sealant removal. Do not remove sealant until the proposed sealant removal method is authorized.

41-3.01C(2) Public Safety and Placement Plans

Before starting crack treatment, submit a public safety plan for HMWM and a placement plan for construction activity as shop drawings.

The public safety and placement plans must identify the materials, equipment, and methods to be used.

In the public safety plan, include the MSDS for each component of HMWM and details for:

1. Shipping
2. Storage
3. Handling
4. Disposal of residual HMWM and containers

If the project is in an urban area adjacent to a school or residence, the public safety plan must also include an airborne emissions monitoring plan prepared by a CIH certified in comprehensive practice by the American Board of Industrial Hygiene. Submit a copy of the CIH's certification. The CIH must monitor the emissions at a minimum of 4 points including the mixing point, the application point, and the point of nearest public contact. At work completion, submit a report by the industrial hygienist with results of the airborne emissions monitoring plan.

The placement plan must include:

1. Crack treatment schedule including coefficient of friction testing
2. Methods and materials including:
   - 2.1. Description of equipment for applying HMWM
   - 2.2. Description of equipment for applying sand
   - 2.3. Gel time range and final cure time for resin

Revise rejected plans and resubmit. With each plan rejection, the Engineer gives revision directions including detailed comments in writing. The Engineer notifies you of a plan's acceptance or rejection within 2 weeks of receiving that plan.

41-3.01C(3) Reserved

41-3.01D Quality Control and Assurance

41-3.01D(1) General

Use test tiles to evaluate the HMWM cure time. Coat at least one 4 by 4 inch smooth glazed tile for each batch of HMWM. Place the coated tile adjacent to the area being treated. Do not apply sand to the test tiles.

Use the same type of crack treatment equipment for testing and production.

41-3.01D(2) Test Area

Before starting crack treatment, treat a test area of at least 500 square feet within the project limits at a location accepted by the Engineer. Use test areas outside the traveled way if available.

Treat the test area under weather and pavement conditions similar to those expected during crack treatment production.

The Engineer evaluates the test area based on the acceptance criteria. Do not begin crack treatment until the Engineer accepts the test area.

41-3.01D(3) Reserved

41-3.01D(4) Acceptance Criteria

The Engineer accepts a treated area if:

1. Corresponding test tiles are dry to the touch
2. Treated surface is tack-free and not oily
3. Sand cover adheres enough to resist hand brushing
4. Excess sand is removed
5. Coefficient of friction is at least 0.30 when tested under California Test 342

41-3.02 MATERIALS

HMWM consists of compatible resin, promoter, and initiator. HMWM resin may be prepromoted by mixing promoter and resin together before filling containers. Identify prepromoted resin on the container label.

Adjust the gel time to compensate for temperature changes throughout the application.

HMWM resin properties must have the following values:

Property	Test method	Value
Viscosity [a] (cP, max, Brookfield RVT with UL adapter, 50 RPM at 77 °F)	ASTM D2196	25
Specific gravity[a] (min, at 77 °F)	ASTM D1475	0.90
Flash point [a] (°F, min)	ASTM D3278	180
Vapor pressure[a] (mm Hg, max, at 77 °F)	ASTM D323	1.0
Tack-free time (minutes, max, at 77 °F)	Specimen prepared under California Test 551	400
Volatile content[a] (%, max)	ASTM D2369	30
PCC saturated surface-dry bond strength (psi, min, at 24 hours and 77 ± 2 °F)	California Test 551	500

[a]Perform the test before adding initiator.

Sand must be commercial quality dry blast sand. At least 95 percent of the sand must pass the no. 8 sieve and at least 95 percent must be retained on the no. 20 sieve when tested under California Test 202.

41-3.02D Reserved

41-3.03 CONSTRUCTION

41-3.03A General

Before applying HMWM, clean the pavement surface by abrasive blasting and blow loose material from visible cracks with high-pressure air. Remove concrete curing seals from the pavement to be treated. The pavement must be dry when blast cleaning is performed. If the pavement surface becomes contaminated before applying the HMWM, clean the pavement surface by abrasive blasting.

If performing abrasive blasting within 10 feet of a lane occupied by traffic, operate abrasive blasting equipment with a concurrently operating vacuum attachment.

During pavement treatment, protect pavement joints, working cracks, and surfaces not being treated.

The equipment applying HMWM must combine the components by either static in-line mixers or by external intersecting spray fans. The pump pressure at the spray bars must not cause atomization. Do not use compressed air to produce the spray. Use a shroud to enclose the spray bar apparatus.

You may apply HMWM manually to prevent overspray onto adjacent traffic. If applying resin manually, limit the batch quantity of HMWM to 5 gallons.

Apply HMWM at a rate of 90 square feet per gallon. The prepared area must be dry and the surface temperature must be from 50 to 100 degrees F while applying HMWM. Do not apply HMWM if the ambient relative humidity is more than 90 percent.

Protect existing facilities from HMWM. Repair or replace existing facilities contaminated with HMWM at your expense.

Flood the treatment area with HMWM to penetrate the pavement and cracks. Apply HMWM within 5 minutes after complete mixing. Mixed HMWM viscosity must not increase. Redistribute excess material with squeegees or brooms within 10 minutes of application. Remove excess material from tined grooves.

Wait at least 20 minutes after applying HMWM before applying sand. Apply sand at a rate of approximately 2 pounds per square yard or until refusal. Remove excess sand by vacuuming or sweeping.

Do not allow traffic on the treated surface until:

1. Treated surface is tack-free and non-oily
2. Sand cover adheres enough to resist hand brushing
3. Excess sand is removed
4. Coefficient of friction is at least 0.30 determined under California Test 342

41-3.04 PAYMENT

Not Used

41-4 SPALL REPAIR

41-4.01 GENERAL

Section 41-4 includes specifications for repairing spalls in concrete pavement.

41-4.02 MATERIALS

Repair spalls using polyester concrete with a bonding agent. The bonding agent must comply with the requirements for HMWM in section 41-3.02 except tack-free time requirements do not apply and the HMWM must not contain wax.

Form board must be corrugated cardboard with a 6-mil polyethylene covering.

41-4.03 CONSTRUCTION

41-4.03A General

Prepare spall areas by removing concrete and cleaning. Use a form board to provide compression relief at joints and cracks.

After completing spall repairs do not allow traffic on the repairs for at least 2 hours after the time of final setting under ASTM C403/403M.

41-4.03B Remove Pavement

The Engineer determines the rectangular limits of unsound concrete pavement. Before removing pavement, mark the saw cut lines and spall repair area on the pavement surface.

Do not remove pavement until the Engineer verbally authorizes the saw cut area.

Use a power-driven saw with a diamond blade.

Remove pavement as shown and:

1. From the center of the repair area towards the saw cut
2. To the full saw cut depth
3. At least 2 inches beyond the saw cut edge to produce a rough angled surface

Produce a rough surface by chipping or other removal methods that do not damage the pavement remaining in-place. Completely remove any saw overcuts. Pneumatic hammers used for concrete removal must weigh 15 lbs or less.

If you damage concrete pavement outside the removal area, enlarge the area to remove the damaged pavement.

If dowel bars are exposed during removal, remove concrete from the exposed surface and cover with duct tape.

41-4.03C Cleaning

After pavement has been removed, clean the exposed faces of the concrete by:

1. Sand or water blasting. Water blasting equipment must be capable of producing a blast pressure of 3,000 to 6,000 psi.

2. Blowing the exposed concrete area with compressed air free of moisture and oil to remove debris after blasting. Air compressors must deliver air at a minimum of 120 cfm and develop 90 psi of nozzle pressure.

41-4.03D Form Board Installation

After cleaning, place the form board to match the existing joint or crack alignment. Extend the form board at least 3 inches beyond each end of the repair and at least 1 inch deeper than the repair. Remove the form board before sealing joints or cracks.

41-4.03E–41-4.03I Reserved

41-4.04 PAYMENT

Payment is calculated based on the authorized saw cut area.

The Department does not adjust the unit price for an increase or decrease in the spall repair quantity.

41-5 JOINT SEALS

41-5.01 GENERAL

41-5.01A Summary

Section 41-5 includes specifications for sealing concrete pavement joints or replacing existing concrete pavement joint seals. Pavement joints include isolation joints.

41-5.01B Definitions

Reserved

41-5.01C Submittals

At least 15 days before delivery to the job site, submit a certificate of compliance, MSDS, manufacturer's recommendations, and instructions for storage and installation of:

1. Liquid joint sealant.
2. Backer rods. Include the manufacturer data sheet verifying compatibility with the liquid joint sealant.
3. Preformed compression joint seal. Include the manufacturer data sheet used to verify the seal for the joint dimensions shown.
4. Lubricant adhesive.

Asphalt rubber joint sealant containers must comply with ASTM D6690. Upon delivery of asphalt rubber joint sealant to the job site, submit a certified test report for each lot based on testing performed within 12 months.

Submit a work plan for removing pavement and joint materials. Allow 10 days for authorization. Include descriptions of the equipment and methods for removal of existing pavement and joint material.

41-5.01D Quality Control and Assurance

41-5.01D(1) General

Before sealing joints, arrange for a representative from the manufacturer to provide training on cleaning and preparing the joint and installing the liquid joint sealant or preformed compression joint seal. Do not seal joints until your personnel and the Department's personnel have been trained.

The Engineer accepts joint seals based on constructed dimensions and visual inspection of completed seals for voids.

41-5.01D(2) Reserved

41-5.02 MATERIALS

41-5.02A General

Use the type of seal material described.

Silicone or asphalt rubber joint sealant must not bond or react with the backer rod.

41-5.02B Silicone Joint Sealant

Silicone joint sealant must be on the Authorized Material List.

41-5.02C Asphalt Rubber Joint Sealant

Asphalt rubber joint sealant must:

1. Be paving asphalt mixed with not less than 10 percent ground rubber by weight. Ground rubber must be vulcanized or a combination of vulcanized and devulcanized materials that pass a no. 8 sieve.
2. Comply with ASTM D6690 for Type II.
3. Be capable of melting at a temperature below 400 degrees F and applied to cracks and joints.

41-5.02D Backer Rods

Backer rods must:

1. Comply with ASTM D5249:
   1.1. Type 1 for asphalt rubber joint sealant
   1.2. Type 1 or Type 3 for silicone joint sealant
2. Be expanded, closed-cell polyethylene foam
3. Have a diameter at least 25 percent greater than the saw cut joint width

41-5.02E Preformed Compression Joint Seals

Preformed compression joint seals must:

1. Comply with ASTM D2628
2. Have 5 or 6 cells, except seals 1/2 inch wide or less may have 4 cells

Lubricant adhesive used to install seals must comply with ASTM D2835.

41-5.02F–41-5.02K Reserved

41-5.03 CONSTRUCTION

41-5.03A General

If joint sealing is described for new concrete pavement, do not start joint sealing activities until the pavement has been in place for at least 7 days. Seal new concrete pavement joints at least 7 days after concrete pavement placement if shown.

Remove existing pavement and joint material by sawing, rectangular plowing, cutting, or manual labor. Saw cut the reservoir before cleaning the joint. Use a power-driven saw with a diamond blade.

If you damage a portion of the pavement to remain in place, repair the pavement under section 41-4.

41-5.03B Joint Cleaning

41-5.03B(1) General

Clean the joint after removal and any repair is complete before installing joint seal material. Cleaning must be completed no more than 4 hours before installing backer rods, liquid joint seal, or preformed compression seals using the following sequence:

1. Removing debris
2. Drying
3. Sandblasting
4. Air blasting
5. Vacuuming

Clean in 1 direction to minimize contamination of surrounding areas.

41-5.03B(2) Removing Debris

Remove debris including dust, dirt, and visible traces of old sealant from the joint after sawing, plowing, cutting, or manual removal. Do not use chemical solvents to wash the joint.

41-5.03B(3) Drying

After removing debris, allow the reservoir surfaces to dry or remove moisture and dampness at the joint with compressed air that may be moderately hot.

41-5.03B(4) Sandblasting

After the joint is dry, sandblast the reservoir to remove remaining residue using a 1/4-inch diameter nozzle and 90 psi minimum pressure. Do not sandblast straight into the reservoir. Angle the sandblasting nozzle within 1 to 2 inches from the concrete and make at least 1 pass to clean each reservoir face.

41-5.03B(5) Air Blasting

After sandblasting, air blast the reservoir to remove sand, dirt, and dust 1 hour before sealing the joint. Use compressed air free of oil and moisture delivered at a minimum rate of 120 cfm and 90 psi nozzle pressure.

41-5.03B(6) Vacuuming

After air blasting, use a vacuum sweeper to remove debris and contaminants from the pavement surfaces surrounding the joint.

41-5.03B(7) Reserved

41-5.03C Installing Liquid Joint Sealant

Where backer rods are shown, place the rods before installing liquid joint sealant. Place backer rods under the manufacturer's instructions unless otherwise specified. The pavement and reservoir surfaces must be dry and the ambient air temperature must be at least 40 degrees F and above the dew point. The reservoir surface must be free of residue or film. Do not puncture the backer rod.

Immediately after placing the backer rod, install liquid joint sealant under the manufacturer's instructions unless otherwise specified. Before installing, demonstrate that fresh liquid sealant is ejected from the nozzle free of cooled or cured material. For asphalt rubber joint sealant, the pavement surface temperature must be at least 50 degrees F before installing.

Pump liquid joint sealant through a nozzle sized for the width of the reservoir so that liquid joint sealant is placed directly onto the backer rod. The installer must draw the nozzle toward his body and extrude liquid joint sealant evenly. Liquid joint sealant must maintain continuous contact with the reservoir walls during extrusion.

After placing liquid joint sealant, recess it to the depth shown within 10 minutes of installation and before a skin begins to form.

After each joint is sealed, remove excess liquid joint sealant on the pavement surface. Do not allow traffic over the sealed joints until the liquid joint sealant is set, tack free, and firm enough to prevent embedment of roadway debris.

41-5.03D Installing Preformed Compression Joint Seals

Install preformed compression joint seals using lubricant adhesive as shown and under the manufacturer's instructions.

Install longitudinal seals before transverse seals. Longitudinal seals must be continuous except splicing is allowed at intersections with transverse seals. Transverse seals must be continuous for the entire transverse length of concrete pavement except splices are allowed for widening and staged construction. With a sharp instrument, cut across the longitudinal seal at the intersection with transverse construction joints. If the longitudinal seal does not relax enough to properly install the transverse seal, trim the longitudinal seal to form a tight seal between the 2 joints.

If splicing is authorized, comply with the manufacturer's instructions.

Use a machine specifically designed for preformed compression joint seal installation. The machine must install the seal:

1. To the specified depth
2. To make continuous contact with the joint walls
3. Without cutting, nicking, or twisting the seal
4. Without stretching the seal more than 4 percent

Cut preformed compression joint seal material to the exact length of the pavement joint to be sealed. The Engineer measures this length. After you install the preformed compression joint seal, the Engineer

measures the excess length of material at the joint end. The Engineer divides the excess length by the measured cut length to determine the stretch percentage.

Seals must be compressed from 30 to 50 percent of the joint width when complete in place.

41-5.03E Reserved

41-5.04 PAYMENT

Not Used

41-6 CRACK AND SEAT

41-6.01 GENERAL

41-6.01A Summary

Section 41-6 includes specifications for cracking, seating, and preparing the surface of existing concrete pavement.

41-6.01B Definitions

Reserved

41-6.01C Submittals

Submit each core in a plastic bag or tube for acceptance at the time of sampling. Mark each core with a location description.

41-6.01D Quality Control and Assurance

41-6.01D(1) General

If cracking is noncompliant:

1. Stop crack and seat work
2. Modify your equipment and procedures and crack the noncompliant pavement again
3. Construct another test section
4. Take additional core samples to verify compliance
5. Construct an inspection strip if the concrete pavement has HMA on the surface

41-6.01D(2) Test Section

The Engineer determines and marks a test section up to 1000 square feet within the crack and seat area shown. Construct the test section and obtain the Engineer's verbal authorization before starting crack and seat work.

Immediately before cracking the test section, apply water to the pavement surface so that cracking can be readily evaluated. Crack the test section and vary impact energy and striking patterns to verify your procedure.

41-6.01D(3) Coring

Drill cores at least 6 inches in diameter under ASTM C42 to verify cracking in the Engineer’s presence. Take at least 2 cores per test section and 1 core per lane mile for each pavement cracking machine used. The Engineer determines the core locations.

41-6.01D(4) Reserved

41-6.02 MATERIALS

41-6.02A General

Use fast-setting or polyester concrete to fill core holes.

41-6.03 CONSTRUCTION

41-6.03A Cracking

Crack existing concrete pavement using the procedures and equipment from the authorized test section.

Do not allow flying debris during cracking operations.

Crack existing concrete pavement into segments that nominally measure 6 feet transversely by 4 feet longitudinally. If the existing pavement is already cracked into segments, crack it into equal-sized square

or rectangular pieces that nominally measure not more than 6 feet transversely and from 3 to 5 feet longitudinally. Do not impact the pavement within 1 foot of another break line, pavement joint, or edge of pavement.

Cracks must be vertical, continuous, and penetrate the full depth of pavement. Cracks must be within 6 inches of vertical along the full depth of pavement. Do not cause surface spalling over 0.10-foot deep or excessive shattering of the pavement or base.

Cracking equipment must impact the pavement with a variable force in a controlled location. Do not use unguided free-falling weights such as "headache balls."

If the concrete pavement has no more than 0.10 foot of asphalt concrete on the surface, you may crack the pavement without removing the asphalt concrete. After cracking, construct an inspection strip by removing at least 500 square feet of asphalt concrete at a location determined by the Engineer. Construct additional inspection strips to demonstrate compliance where ordered by the Engineer.

After cracking, allow public traffic on the cracked or initial pavement layer for no more than 15 days.

41-6.03B Seating

Seat cracked concrete by making at least 5 passes over the cracked concrete with either:

04-18-14

1. Oscillating type pneumatic-tired roller at least 4 feet wide. Pneumatic tires must be of equal size, diameter, type, and ply. The tires must be inflated to 60 psi minimum and maintained so that the air pressure does not vary more than 5 psi. The roller's gross static weight must be at least 15 tons.

07-19-13

2. Vibratory pad-foot roller exerting a dynamic centrifugal force of at least 10 tons

A pass is 1 movement of a roller in either direction at 5 mph or less.

After all segments have been seated, clean loose debris from joints and cracks using compressed air free of moisture and oil.

Reseat any segment of cracked pavement that has not been overlaid within 24 hours of seating.

41-6.03C Surface Preparation

Before opening cracked and seated pavement to traffic or overlaying:

04-18-14

1. Fill joints, cracks, and spalls wider than 3/4 inch and deeper than 1 inch by applying tack coat and placing minor HMA under section 39. Use the no. 4 gradation.

07-19-13

2. Remove all loose debris and sweep the pavement.

41-6.03D Reserved

41-6.04 PAYMENT

Crack and seat existing concrete pavement is measured from the area of pavement cracked and seated. No deduction is made for existing cracked segments. The Department does not pay for HMA used to fill joints, cracks, and spalls.

41-7 TRANSITION TAPER

41-7.01 GENERAL

Section 41-7 includes specifications for constructing transition tapers in existing pavement.

41-7.02 MATERIALS

Not Used

41-7.03 CONSTRUCTION

Construct transition tapers by either grinding or removing and replacing the existing concrete. Do not allow flying debris during the construction of tapers.

Grinding must comply with section 42.

Replacement concrete must comply with section 41-9 except place concrete to the taper level shown and finish the surface with a coarse broom.

04-18-14

If the transition taper will be overlaid with HMA that is not placed before opening to traffic and there is a grade difference of more than 0.04 foot, construct a temporary taper by placing minor HMA that complies with section 39. Remove the temporary HMA taper before constructing the transition taper.

07-19-13

41-7.04 PAYMENT

Pavement transition tapers are measured using the dimensions shown. The Department does not pay for temporary HMA tapers.

41-8 DOWEL BAR RETROFIT

Reserved

41-9 INDIVIDUAL SLAB REPLACEMENT WITH RAPID STRENGTH CONCRETE

41-9.01 GENERAL

41-9.01A Summary

Section 41-9 includes specifications for removing existing concrete pavement and constructing individual slab replacement with rapid strength concrete (ISR—RSC).

41-9.01B Definitions

concrete raveling: Disintegration of the concrete surface layer from aggregate loss.

early age: Any age less than 10 times the time of final setting for concrete determined under ASTM C403/C403M.

full-depth crack: Crack that runs from one edge of the concrete slab to the opposite or adjacent side of the slab.

opening age: Age when the minimum modulus of rupture specified for opening to traffic and equipment is attained.

time of final setting: Elapsed time required to develop a concrete penetration resistance that is at least 4,000 psi under ASTM C403/C403M.

41-9.01C Submittals

41-9.01C(1) General

At least 15 days before delivery to the job site, submit manufacturer's recommendations, MSDS and instructions for storage and installation of joint filler material.

At least 45 days before starting ISR—RSC work submit a sample of cement from each proposed lot and samples of proposed admixtures in the quantities ordered by the Engineer.

During ISR—RSC placement operations, submit uniformity reports for hydraulic cement at least once every 30 days to the Engineer and METS, attention Cement Laboratory. Uniformity reports must comply with ASTM C917 except testing age and water content may be modified to suit the particular material.

Except for modulus of rupture tests, submit QC test result forms within 48 hours of the paving shift. Submit modulus of rupture results within:

1. 15 minutes of opening age test completion
2. 24 hours of 3-day test completion

41-9.01C(2) Quality Control Plan

If the quantity of ISR—RSC is at least 300 cu yd, submit a QC plan at least 20 days before placing trial slabs. If the quantity of ISR—RSC is less than 300 cu yd, submit proposed forms for RSC inspection, sampling, and testing.

41-9.01C(3) Mix Design

At least 10 days before use in a trial slab, submit a mix design. The maximum ambient temperature range for a mix design is 18 degrees F. Submit more than 1 mix design based on ambient temperature variations anticipated during RSC placement. Each mix design must include:

1. Mix design identification number
2. Aggregate source
3. Opening age
4. Aggregate gradation
5. Types of cement and chemical admixtures
6. Mix proportions
7. Maximum time allowed between batching and placing
8. Range of effective ambient temperatures
9. Time of final setting
10. Modulus of rupture development data from laboratory-prepared samples, including tests at:
    - 10.1. 1 hour before opening age
    - 10.2. Opening age
    - 10.3. 1 hour after opening age
    - 10.4. 1 day
    - 10.5. 3 days
    - 10.6. 7 days
    - 10.7. 28 days
11. Shrinkage test data
12. Any special instructions or conditions such as water temperature requirements

41-9.01C(4) Reserved

41-9.01D Quality Control and Assurance

41-9.01D(1) General

Designate a QC manager and assistant QC managers to administer the QC plan. The QC managers must hold current American Concrete Institute (ACI) certification as a Concrete Field Testing Technician-Grade I and a Concrete Laboratory Testing Technician-Grade II, except the assistant QC managers may hold Concrete Laboratory Testing Technician-Grade I instead of Grade II.

The QC manager responsible for the production period involved must review and sign the sampling, inspection, and test reports before submitting them. The QC manager must be present for:

1. Each stage of mix design
2. Trial slab construction
3. Production and construction of RSC
4. Meetings with the Engineer relating to production, placement, or testing

The QC manager must not be a member of this project's production or paving crews, an inspector, or a tester. The QC manager must have no duties during the production and placement of RSC except those specified.

Testing laboratories and equipment must comply with the Department's Independent Assurance Program. At the time of the QC plan submittal, the Department evaluates the quality control samplers and testers.

41-9.01D(2) Just-in-time Training

Reserved

41-9.01D(3) Quality Control Plan

Establish, implement, and maintain a QC plan for pavement The QC plan must describe the organization and procedures used to:

1. Control the production process
2. Determine if a change to the production process is needed
3. Implement a change

The QC plan must include:

1. Names, qualifications, and certifications of QC personnel, including:
    1.1. QC manager
    1.2. Assistant QC managers
    1.3. Samplers and testers
2. Outline of procedure for the production, transportation, placement, and finishing of RSC
3. Outline of procedure and forms for concrete QC, sampling, and testing to be performed during and after RSC construction, including testing frequencies for modulus of rupture
4. Contingency plan for identifying and correcting problems in production, transportation, placement, or finishing RSC including:
    4.1. Action limits
    4.2. Suspension limits that do not exceed specified material requirements
    4.3. Detailed corrective action if limits are exceeded
    4.4. Temporary pavement structure provisions, including:
        4.4.1. The quantity and location of standby material
        4.4.2. Determination of need
5. Location of your quality control testing laboratory and testing equipment during and after paving operations
6. List of the testing equipment to be used, including the date of last calibration
7. Production target values for material properties that impact concrete quality or strength including cleanness value and sand equivalent
8. Outline procedure for placing and testing trial slabs, including:
    8.1. Locations and times
    8.2. Production procedures
    8.3. Placing and finishing methods
    8.4. Sampling methods, sample curing, and sample transportation
    8.5. Testing and test result reporting
9. Name of source plant with approved Material Plant Quality Program (MPQP)
10. Procedures or methods for controlling pavement quality including:
    10.1. Materials quality
    10.2. Contraction and construction joints
    10.3. Protecting pavement before opening to traffic

41-9.01D(4) Prepaving Conference

Schedule a prepaving conference and provide a facility to meet with the Engineer.

Prepaving conference attendees must sign an attendance sheet provided by the Engineer. The prepaving conference must be attended by your:

1. Project superintendent
2. Project manager
3. QC manager
4. Workers and your subcontractor's workers, including:
    4.1. Foremen
    4.2. Concrete plant manager
    4.3. Concrete plant operator
    4.4. Concrete plant inspectors
    4.5. Personnel performing saw cutting and joint sealing
    4.6. Paving machine operators
    4.7. Inspectors
    4.8. Samplers
    4.9. Testers

The purpose of the prepaving conference is to familiarize personnel with the project's specifications. Discuss the QC plan and processes for constructing each item of work, including:

1. Production
2. Transportation
3. Trial slabs
4. Pavement structure removal
5. Placement

6. Contingency plan
7. Sampling
8. Testing
9. Acceptance

Do not start trial slabs or paving activities until the listed personnel have attended the prepaving conference.

41-9.01D(5) Trial Slabs

Before starting individual slab replacement work, complete 1 trial slab for each mix design.

Place trial slabs near the job site at a mutually-agreed location that is neither on the roadway nor within the project limits. Trial slabs must be 10 by 20 feet and at least 10 inches thick.

During trial slab construction, sample and split the aggregate for grading, cleanness value, and sand equivalent testing.

Fabricate and test beams under California Test 524 to determine the modulus of rupture values.

Cure beams fabricated for early age testing such that the monitored temperatures in the beams and the slab are always within 5 degrees F of each other.

Monitor and record the internal temperatures of trial slabs and early age beams at intervals of at least 5 minutes. Install thermocouples or thermistors connected to strip-chart recorders or digital data loggers to monitor the temperatures. Temperature recording devices must be accurate to within 2 degrees F. Measure internal temperatures at 1 inch from the top, 1 inch from the bottom, and no closer than 3 inches from any edge until early age testing is completed.

Cure beams fabricated for 3-day testing under California Test 524 except place them into sand at a time that is from 5 to 10 times the time of final setting measured under ASTM C403/403M or 24 hours, whichever is earlier.

Trial slabs must have an opening age modulus of rupture of not less than 400 psi and a 3-day modulus of rupture of not less than 600 psi.

After authorization, remove and dispose of trial slabs and testing materials.

41-9.01D(6) Quality Control Testing

41-9.01D(6)(a) General

Provide continuous process control and quality control sampling and testing throughout RSC production and placement. Notify the Engineer at least 2 business days notice before any sampling and testing. Establish a testing facility at the job site or at an authorized location.

Sample under California Test 125.

During ISR—RSC placement, sample and fabricate beams for modulus of rupture testing within the first 30 cubic yards, at least once every 130 cu yd, and within the final truckload. Submit split samples and fabricate test beams for the Department’s testing unless the Engineer informs you otherwise.

Determine the modulus of rupture at opening age under California Test 524, except beam specimens may be fabricated using an internal vibrator under ASTM C 31. Cure beams under the same conditions as the pavement until 1 hour before testing. Test 3 beam specimens in the presence of the Engineer and average the results. A single test represents no more than that day's production or 130 cu yd, whichever is less.

Determine the modulus of rupture at other ages using beams cured and tested under California Test 524 except place them in sand from 5 to 10 times the time of final setting under ASTM C403/C403M or 24 hours, whichever is earlier.

41-9.01D(6)(b) Rapid Strength Concrete

Your quality control must include testing RSC for the properties at the frequencies shown in the following table:

RSC Minimum Quality Control

Property	Test method	Minimum testing frequency[a]
Cleanness value	California Test 227	650 cu yd or 1 per shift
Sand equivalent	California Test 217	650 cu yd or 1 per shift
Aggregate gradation	California Test 202	650 cu yd or 1 per shift
Air content	California Test 504	130 cu yd or 2 per shift
Yield	California Test 518	2 per shift
Slump or penetration	ASTM C143 or California Test 533	1 per 2 hours of paving
Unit weight	California Test 518	650 cubic yards or 2 per shift
Aggregate Moisture Meter Calibration[b]	California Test 223 or California Test 226	1 per shift
Modulus of rupture	California Test 524	Comply with section 41-9.01D(6)(a)

[a]Test at the most frequent interval.
[b]Check calibration of the plant moisture meter by comparing moisture meter readings with California Test 223 or California Test 226 test results

Maintain control charts to identify potential problems and causes. Post a copy of each control chart at a location determined by the Engineer.

Individual measurement control charts must use the target values in the mix proportions as indicators of central tendency.

Develop linear control charts for:

1. Cleanness value
2. Sand equivalent
3. Fine and coarse aggregate gradation
4. Air content
5. Penetration

Control charts must include:

1. Contract number
2. Mix proportions
3. Test number
4. Each test parameter
5. Action and suspension limits
6. Specification limits
7. Quality control test results

For fine and coarse aggregate gradation control charts, record the running average of the previous 4 consecutive gradation tests for each sieve and superimpose the specification limits.

For air content control charts, the action limit is ±1.0 percent and the suspension limit is ±1.5 percent of the specified values. If no value is specified, apply the air content value used in the approved mix design.

As a minimum, a process is out of control if any of the following occurs:

1. For fine and coarse aggregate gradation, 2 consecutive running averages of 4 tests are outside the specification limits
2. For individual penetration or air content measurements:
   2.1. One point falls outside the suspension limit line
   2.2. Two points in a row fall outside the action limit line

Stop production and take corrective action for out of control processes or the Engineer rejects subsequent RSC.

Before each day's concrete pavement placement and at intervals not to exceed 4 hours of production, use a tachometer to test and record vibration frequency for concrete consolidation vibrators.

41-9.01D(6)(c) Reserved

41-9.01D(7) Acceptance Criteria

41-9.01D(7)(a) General

The final texture of ISR—RSC must pass visual inspection and have a coefficient of friction of at least 0.30 determined under California Test 342.

Allow at least 25 days for the Department to schedule testing for coefficient of friction. Notify the Engineer when the pavement is scheduled to be opened to traffic.

41-9.01D(7)(b) Modulus of Rupture

ISR—RSC is accepted based on your testing for modulus of rupture at opening age and the Department's testing for modulus of rupture at 3 days.

ISR—RSC must have a modulus of rupture at opening age that is at least 400 psi and a modulus of rupture at 3 days that is at least 600 psi.

Calculate the test result as the average from testing 3 beams for each sample. The test result represents 1 paving shift or 130 cu yd, whichever is less.

41-9.01D(7)(c) Concrete Pavement Smoothness

The Department tests for concrete pavement smoothness using a 12-foot straightedge. Straightedge smoothness specifications do not apply to the pavement surface placed within 12 inches of existing concrete pavement except parallel to the centerline at the midpoint of a transverse construction joint.

The concrete pavement surface must not vary from the lower edge of a 12-foot straightedge by more than:

1. 0.01 feet when parallel to the centerline
2. 0.02 feet when perpendicular to the centerline extending from edge to edge of a traffic lane

41-9.01D(7)(d) Cracking and Raveling

The Engineer rejects an ISR—RSC slab under section 6-3.06 if within 1 year of contract acceptance there is either:

1. Partial or full-depth cracking
2. Concrete raveling consisting of either:
   - 2.1. Combined raveled areas more than 5 percent of each ISR—RSC slab area
   - 2.2. Any single raveled area of more than 4 sq ft

41-9.01D(8) Reserved

41-9.02 MATERIALS

41-9.02A General

Reserved

41-9.02B Rapid Strength Concrete

RSC for ISR—RSC must comply with section 90-3.

Use either the 1-1/2 inch maximum or the 1-inch maximum combined grading specified in section 90-1.02C(4)(d).

Air content must comply with the minimum requirements in section 40-1.02B(4).

41-9.02C Base Bond Breaker

Use base bond breaker no. 3, 4, or 5 under section 36-2.

41-9.02D Reserved

41-9.03 CONSTRUCTION

41-9.03A General

Complete ISR—RSC adjacent to new pavement or existing pavement shown for construction as a 1st order of work. Replace individual slabs damaged during construction before placing final pavement delineation.

41-9.03B Removing Existing Pavement

Remove pavement under section 15-2.02. The Engineer determines the exact ISR—RSC limits after overlying layers are removed.

After removing pavement to the depth shown, grade to a uniform plane. Water as needed and compact the material remaining in place to a firm and stable base. The finished surface of the remaining material must not extend above the grade established by the Engineer.

41-9.03C Drill and Bond Dowel Bars

Drill existing concrete and bond dowel bars under section 41-10 if described. Do not install dowel bars in contraction joints.

41-9.03D Base Bond Breaker

Place base bond breaker before placing ISR—RSC. Comply with section 36-2.

41-9.03E Placing Rapid Strength Concrete

Do not place RSC if the ambient air temperature is forecast by the National Weather Service to be less than 40 degrees F within 72 hours of final finishing.

Before placing RSC against existing concrete, place 1/4-inch thick commercial quality polyethylene flexible foam expansion joint filler across the original transverse and longitudinal joint faces and extend the full depth of pavement to the top of the base layer. Place the top of the joint filler flush with the top of the pavement. Secure joint filler to the joint face of the existing pavement to prevent the joint filler from moving during the placement of RSC.

Use metal or wood side forms. Wood side forms must not be less than 1-1/2 inches thick. Side forms and connections must be of sufficient rigidity that movement will not occur under forces from equipment or RSC. Clean and oil side forms before each use. Side forms must remain in place until the pavement edge no longer requires the protection of forms.

After you place RSC, consolidate it using high-frequency internal vibrators adjacent to forms and across the full paving width. Place RSC as nearly as possible to its final position. Do not use vibrators for extensive shifting of concrete pavement.

Spread and shape RSC with powered finishing machines supplemented by hand finishing. After you mix and place RSC, do not add water to the surface to facilitate finishing. You may request authorization to use surface finishing additives. Submit the manufacturer's instructions with your request.

Place consecutive concrete loads without interruption. Do not allow cold joints where a visible lineation forms after concrete is placed, sets, and hardens before additional concrete placed.

Where the existing transverse joint spacing in an adjacent lane exceeds 15 feet, construct an additional transverse contraction joint midway between the existing joints. Complete sawing of contraction joints within 2 hours of completion of final finishing.

Cut contraction joints a minimum of 1/3 the slab depth.

41-9.03F Final Finishing

After preliminary finishing, round the edges of the initial paving width to a 0.04-foot radius. Round transverse and longitudinal construction joints to a 0.02-foot radius. Mark each ISR—RSC area with a stamp. The stamp mark must show the month, day, and year of placement and contract number. Level the location of the stamp with a steel trowel below the pavement texture. Orient the stamp mark so it can be read from the outside edge of ISR—RSC.

Before curing, texture the pavement. Perform initial texturing with a burlap drag or broom device that produces striations parallel to the centerline. Perform final texturing with a steel-tined device that produces grooves parallel with the centerline.

Tines must be from 3/32 to 1/8 inch wide on 3/4-inch centers and have enough length, thickness, and resilience to form grooves from 1/8 to 3/16 inch deep after the concrete has hardened. Grooves must extend over the entire pavement width except do not construct grooves 3 inches from longitudinal pavement edges or joints.

Final texture must be uniform and smooth. Grooves must be parallel and aligned to the pavement edge across the pavement width. The groove alignment must not vary more than 0.1 foot for every 12 foot length.

Protect RSC under section 90-1.03C.

41-9.03G Temporary Pavement Structure

Temporary pavement structure must be RSC or 3-1/2 inch thick HMA over aggregate base.

41-9.03H Noncompliant Individual Slab Replacement

Replace an ISR—RSC slab with any of the following:

1. One or more full-depth cracks.
2. Concrete raveling.
3. Noncompliant smoothness except you may request authorization for grinding under section 42 and retesting. Grinding that causes a depression will not be considered. Smoothness must be corrected within 48 hours of placing ISR—RSC.
4. Noncompliant modulus of rupture.

If the modulus of rupture at opening age is at least 400 psi and the modulus of rupture at 3 days is at least 500 psi but less than 600 psi, you may request authorization to leave the ISR—RSC in place and accept the specified deduction.

If pavement is noncompliant for coefficient of friction, groove or grind the pavement under section 42. Comply with section 40-1.03Q(4) and groove or grind before the installation of any required joint seal or edge drains adjacent to the areas to the noncompliant area.

If an ISR—RSC slab has partial depth cracking, treat it with high-molecular-weight methacrylate under section 41-3.

41-9.03I Replace Pavement Delineation

Replace traffic stripes, pavement markings, and markers that are removed, obliterated, or damaged by ISR—RSC under sections 84 and 85.

41-9.03J Reserved

41-9.04 PAYMENT

Replace base is not included in the payment for individual slab replacement (RSC).

Drill and bond dowel bars are not included in payment for individual slab replacement (RSC).

For individual slab replacement (RSC) with a modulus of rupture at opening age that is at least 400 psi and a modulus of rupture at 3 days that is greater than or equal to 500 psi but less than 550 psi, the Department deducts 10 percent of the payment for individual slab replacement (RSC).

For individual slab replacement (RSC) with a modulus of rupture at opening age that is at least 400 psi and a modulus of rupture at 3 days that is greater than or equal to 550 psi but less than 600 psi, the Department deducts 5 percent of the payment for individual slab replacement (RSC).

41-10 DRILL AND BOND BARS

41-10.01 GENERAL

41-10.01A Summary

Section 41-10 includes specifications for drilling, installing, and bonding tie bars and dowel bars in concrete pavement.

41-10.01B Definitions

Reserved

41-10.01C Submittals

Submit a certificate of compliance for:

1. Tie bars
2. Dowel bars
3. Dowel bar lubricant
4. Chemical adhesive
5. Epoxy powder coating

At least 15 days before delivery to the job site, submit the manufacturer's recommendations and instructions for storage, handling, and use of chemical adhesive.

41-10.01D Quality Control and Assurance

41-10.01D(1) General

Drill and bond bar is accepted based on inspection before concrete placement.

41-10.01D(2) Reserved

41-10.02 MATERIALS

41-10.02A General

Dowel bar lubricant must comply with section 40-1.02D.

Chemical adhesive for drilling and bonding bars must be on the Authorized Material List. The Authorized Material List indicates the appropriate chemical adhesive system for concrete temperature and installation conditions.

Each chemical adhesive system container must clearly and permanently show the following:

1. Manufacturer's name
2. Model number of the system
3. Manufacture date
4. Batch number
5. Expiration date
6. Current International Conference of Building Officials Evaluation Report number
7. Directions for use
8. Storage requirement
9. Warnings or precautions required by state and federal laws and regulations

41-10.02B Reserved

41-10.03 CONSTRUCTION

41-10.03A General

Drill holes for bars. Clean drilled holes in compliance with the chemical adhesive manufacturer’s instructions. Holes must be dry at the time of placing the chemical adhesive and bars. Use a grout retention ring when drilling and bonding dowel bars. Immediately after inserting the bar into the chemical adhesive, support the bar to prevent movement until chemical adhesive has cured the minimum time recommended by the manufacturer.

Apply dowel bar lubricant to the entire exposed portion of the dowel bar.

If the Engineer rejects a bar installation: stop paving, drilling, and bonding activities. Adjust your procedures and obtain the Engineer's verbal authorization before resuming paving, drilling, and bonding.

Cut the rejected bar flush with the pavement joint surface and coat the exposed end of the bar with chemical adhesive. Offset the new hole 3 inches horizontally from the rejected hole's center.

41-10.03B Tie Bar Tolerance

Place tie bars within the tolerances shown in the following table:

Tie Bar Tolerances

Dimension	Tolerance
Horizontal skew (vertical skew: bar length)	1:6
Vertical skew (vertical skew: bar length)	1:6
Longitudinal translation (inch)	±1
Horizontal offset (embedment, inch)	±1
Height relative to the adjacent bar	±1
Vertical Depth (clearance from the pavement surface or bottom, inches, min)	3

41-10.03C Dowel Bar Tolerance

Place dowel bars within the tolerances specified in section 40-1.01D(7)(b)(v).

41-10.03D Reserved

41-10.04 PAYMENT

Not Used

41-11–41-15 RESERVED

^^

42 GROOVE AND GRIND CONCRETE

07-19-13

Replace the paragraph of section 42-1.01A with:

07-19-13

Section 42-1 includes general specifications for grooving and grinding concrete.

Replace the headings and paragraphs in section 42-3 with:

07-19-13

42-3.01 GENERAL

42-3.01A Summary

Section 42-3 includes specifications for grinding the surfaces of pavement, bridge decks, and approach slabs.

42-3.01B Definitions

Reserved

42-3.01C Submittals

Reserved

42-3.01D Quality Control and Assurance

Reserved

42-3.02 MATERIALS

Not Used

42-3.03 CONSTRUCTION

42-3.03A General

Grind surfaces in the longitudinal direction of the traveled way and grind the full lane width. Begin and end grinding at lines perpendicular to the roadway centerline.

Grinding must result in a parallel corduroy texture with grooves from 0.08 to 0.12 inch wide and from 55 to 60 grooves per foot of width. Grooves must be from 0.06 to 0.08 inch from the top of the ridge to the bottom of the groove.

Grind with abrasive grinding equipment using diamond cutting blades mounted on a self-propelled machine designed for grinding and texturing concrete pavements.

42-3.03B Pavement

Grind existing concrete pavement that is adjacent to an individual slab replacement. Grind the replaced individual slab and all the existing slabs immediately surrounding it. Grind after the individual slab is replaced.

Grind existing concrete pavement that is adjacent to new lanes of concrete pavement. Grind before paving.

After grinding, the existing pavement must comply with requirements for smoothness and coefficient of friction in section 40 except:

1. At the midpoint of a joint or crack, test smoothness with a straightedge. Both sides must have uniform texture.
2. Straightedge and inertial profiler requirements do not apply to areas abnormally depressed from subsidence or other localized causes. End smoothness testing 15 feet before and resume 15 feet after these areas.
3. Cross-slope must be uniform and have positive drainage across the traveled way and shoulder.

As an alternative to grinding existing concrete pavement, you may replace the existing pavement. The new concrete pavement must be the same thickness as the removed pavement. Replace existing pavement between longitudinal joints or pavement edges and transverse joints. Do not remove portions of slabs.

Replacement of existing concrete pavement must comply with requirements for individual slab replacement in section 41-9.

42-3.03C Bridge Decks, Approach Slabs, and Approach Pavement

Grind bridge decks, approach slabs, and approach pavement only if described.

The following ground areas must comply with the specifications for smoothness and concrete cover over reinforcing steel in section 51-1.01D(4):

1. Bridge decks
2. Approach slabs
3. Adjacent 50 feet of approach pavement

After grinding, the coefficient of friction must comply with section 51-1.01D(4).

42-3.04 PAYMENT

Grinding existing approach slabs and adjacent 50 feet of approach pavement is paid for as grind existing bridge deck.

The Department does not pay for grinding replacement concrete pavement or for additional grinding to comply with smoothness requirements.

Add to section 42:

07-19-13

42-4–42-9 RESERVED

^^^

section 90, except they must not contain chloride ions in excess of 0.25 percent by weight. Do not exceed 5 gallons of water per 94 lb of cement.

Mix the grout as follows:

1. Add water to the mixer followed by cement and any admixtures or fine aggregate.
2. Mix the grout with mechanical mixing equipment that produces a uniform and thoroughly mixed grout.
3. Agitate the grout continuously until the grout is pumped.
4. Do not add water after the initial mixing.

Add to section 46-1.03B:

04-20-12

Dispose of drill cuttings under section 19-2.03B.

Add to the end of section 46-1.03C:

07-19-13

Grouting equipment must be:

1. Capable of grouting at a pressure of at least 100 psi
2. Equipped with a pressure gage having a full-scale reading of not more than 300 psi

07-19-13

Delete the 3rd paragraph of section 46-2.01A.

Add to the beginning of section 46-2.01C:

07-19-13

Submittals for strand tendons, bar tendons, bar couplers, and anchorage assemblies must comply with section 50-1.01C.

Add to section 46-2.01D:

07-19-13

46-2.01D(3) Steel

Strand tendons, bar tendons, bar couplers, and anchorage assemblies must comply with section 50-1.01D.

46-2.01D(4) Grout

The Department tests the efflux time of the grout under California Test 541.

Add to the beginning of section 46-2.02B:

07-19-13

Strand tendons, bar tendons, and bar couplers must comply with section 50-1.02B.

Replace the 1st paragraph of section 46-2.02E with:

07-19-13

The efflux time of the grout immediately after mixing must be at least 11 seconds.

Add between the 13th and 14th paragraphs of section 46-2.03A:

07-19-13

If hot weather conditions will contribute to quick stiffening of the grout, cool the grout by authorized methods as necessary to prevent blockages during pumping activities.

Add between the 1st and 2nd paragraphs of section 46-2.03D:

07-19-13

Secure the ends of strand tendons with a permanent type anchorage system that:

1. Holds the prestressing steel at a force producing a stress of at least 95 percent of the specified ultimate tensile strength of the steel
2. Permanently secures the ends of the prestressing steel

Replace the 2nd sentence of the 1st paragraph of section 46-3.02A with:

07-19-13

The epoxy-coated prefabricated reinforcing bar must comply with section 52-2.03, except the epoxy thickness must be from 10 to 12 mils.

Replace the 2nd paragraph of section 46-3.02B with:

07-19-13

Concrete anchors on bearing plates must comply with the specifications for studs in clause 7 of AWS D1.1.

07-19-13

Delete the 1st paragraph of section 46-3.02E.

^^

47 EARTH RETAINING SYSTEMS

07-19-13

Replace the 2nd paragraph of section 47-2.01D with:

02-17-12

Coupler test samples must comply with minimum tensile specifications for steel wire in ASTM A 82/A 82M. Total wire slip must be at most 3/16 inch when tested under the specifications for tension testing of round wire test samples in ASTM A 370.

Replace "78-80" in the 1st table in the 2nd paragraph of section 47-2.02C with:

10-19-12

78-100

Replace the value for the sand equivalent requirement in the 2nd table in the 3rd paragraph of section 47-2.02C with:

01-20-12

12 minimum

Replace the 1st paragraph of section 47-2.02E with:

02-17-12

Steel wire must comply with ASTM A 82/A 82M. Welded wire reinforcement must comply with ASTM A 185/A 185M.

Replace section 47-3 with:

07-19-13

47-3 REINFORCED CONCRETE CRIB WALLS

47-3.01 General

Section 47-3 includes specifications for constructing reinforced concrete crib walls.

Reinforced concrete crib walls must comply with section 51.

Reinforcement must comply with section 52.

Concrete crib walls consist of a series of rectangular cells composed of interlocking, precast, reinforced concrete headers, stretchers, and blocks.

47-3.02 Materials

47-3.02A General

Pads shown to be placed between bearing surfaces must either be (1) neoprene complying with the specifications for strip waterstops in section 51-2.05 or (2) commercial quality no. 30 asphalt felt. The protective board is not required for neoprene pads.

47-3.02B Crib Members

47-3.02B(1) General

All members may be manufactured to dimensions 1/8 inch greater in thickness than shown. The thickness of the lowest step must not be less than the dimension shown.

Stretchers may be manufactured 1/2 inch less in length than shown.

When an opening is shown in the face of the wall, special length stretchers and additional headers may be necessary.

For non-tangent wall alignments, special length stretchers may be required.

For non-tangent wall alignments and at locations where filler blocks are required, special length front face closure members may be required.

47-3.02B(2) Reinforcement

Reinforcing wire must comply with ASTM A 496/A 496M.

For hoops or stirrups use either (1) reinforcing wire or (2) deformed steel welded wire reinforcement. The size must be equivalent to the reinforcing steel shown. Deformed steel welded wire reinforcement must comply with ASTM A 497/A 497M.

47-3.02B(3) Concrete

Concrete test cylinders must comply with section 90-1.01D(5), except when the penetration of fresh concrete is less than 1 inch, the concrete in the test mold must be consolidated by vibrating the mold equivalent to the consolidating effort being used to consolidate the concrete in the members.

Cure crib members under section 51-4.02C.

When removed from forms, the members must present a true surface of even texture, free from honeycombs and voids larger than 1 inch in diameter and 5/16 inch in depth. Clean and fill other pockets with mortar under sections 51-1.02F and 51-1.03E(2).

External vibration resulting in adequate consolidation may be used.

If the Engineer determines that rock pockets are of the extent or character as to affect the strength of the member or to endanger the life of the steel reinforcement, replace the member.

Finish concrete-to-concrete bearing surfaces to a smooth plane. Section 51-1.03F does not apply to concrete crib members.

47-3.03 Construction

Place reinforced concrete crib walls to the lines and grades established by the Engineer. The foundation must be accepted by the Engineer before any crib members are placed.

The gap between bearing surfaces must not exceed 1/8 inch.

Where a gap of 1/16 inch to 1/8 inch exists or where shown, place a 1/16-inch pad of asphalt felt or sheet neoprene between the bearing surfaces.

47-3.04 Payment

The area of reinforced concrete crib wall is measured on the batter at the outer face for the height from the bottom of the bottom stretcher to the top of the top stretcher and for a length measured from end to end of each section of wall.

Add between the 3rd and 4th paragraphs of section 47-5.01:

10-19-12

Reinforcement must comply with section 52.

Add to section 47-6.01A:

10-19-12

The alternative earth retaining system must comply with the specifications for the type of wall being constructed.

Replace "sets" at each occurrence in the 1st paragraph of section 47-6.01C with:

04-19-13

copies

^^

48 TEMPORARY STRUCTURES

07-19-13

Replace "previously welded splice" and its definition in section 48-2.01B with:

04-19-13

previously welded splice: Splice made in a falsework member in compliance with AWS D1.1 or other recognized welding standard before contract award.

Add to section 48-2.01B:

07-19-13

independent support system: Support system that is in addition to the falsework removal system employing methods of holding falsework from above by winches, hydraulic jacks with prestressing steel, HS rods, or cranes.

04-19-13

Delete "field" in the 1st sentence of the 5th paragraph of section 48-2.01C(1).

Replace item 1 in the list in the 6th paragraph of section 48-2.01C(1) with:

04-19-13

1. Itemize the testing, inspection methods, and acceptance criteria used

Replace "sets" at each occurrence in the 4th paragraph of section 48-2.01C(2) with:

07-19-13

copies

Replace the 7th paragraph of section 48-2.01C(2) with:

09-16-11

If you submit multiple submittals at the same time or additional submittals before review of a previous submittal is complete:

1. You must designate a review sequence for submittals
2. Review time for any submittal is the review time specified plus 15 days for each submittal of higher priority still under review

Add to section 48-2.01C(2):

07-19-13

Shop drawings and calculations for falsework removal systems employing methods of holding falsework from above by winches, hydraulic jacks with prestressing steel, HS rods, or cranes must include:

1. Design code used for the analysis of the structural members of the independent support system
2. Provisions for complying with current Cal/OSHA requirements
3. Load tests and ratings within 1 year of intended use of hydraulic jacks and winches
4. Location of the winches, hydraulic jacks with prestressing steel, HS rods, or cranes
5. Analysis showing that the bridge deck and overhang are capable of supporting all loads at all time
6. Analysis showing that winches will not overturn or slide during all stages of loading
7. Location of deck and soffit openings if needed
8. Details of repair for the deck and soffit openings after falsework removal

Replace the 1st paragraph of section 48-2.01D(2) with:

04-19-13

Welding must comply with AWS D1.1 or other recognized welding standard, except for fillet welds where the load demands are 1,000 lb or less per inch for each 1/8 inch of fillet weld.

Replace the 1st through 3rd sentences in the 2nd paragraph of section 48-2.01D(2) with:

04-19-13

Perform NDT on welded splices using UT or RT. Each weld and any repair made to a previously welded splice must be tested.

Replace the 3rd paragraph of section 48-2.01D(2) with:

04-19-13

For previously welded splices, perform and document all necessary testing and inspection required to certify the ability of the falsework members to sustain the design stresses.

Add to section 48-2.01D(3)(a):

07-19-13

Falsework removal system employing methods of holding falsework from above and members of the independent support system must support the sum of the actual vertical and horizontal loads due to falsework materials, equipment, construction sequence or other causes, and wind loading. Identifiable mechanical devices used in the falsework removal plan must meet applicable industry standards and manufacturer instructions for safe load carrying capacity. Unidentifiable winches must be capable of carrying twice the design load.

The load used for the analysis of overturning moment and sliding of the winch system must be 150 percent of the design load.

Add to section 48-2.03D:

07-19-13

Falsework removal employing methods of holding falsework by winches, hydraulic jacks with prestressing steel, HS rods, or cranes must also be supported by an independent support system when the system is not actively lowering the falsework at vehicular, pedestrian, or railroad traffic openings.

Bridge deck openings used to facilitate falsework removal activities must be formed and located away from the wheel path. The formed openings must be wedge shaped with a 5-inch maximum diameter at the top and a 3-inch maximum diameter at the bottom.

Anchor 10-inch-square aluminum or galvanized steel wire, 1/4-inch-mesh hardware cloth with a 0.025-inch minimum wire diameter firmly to the inside of the soffit openings. Construct a 1/2-inch drip groove to the outside of soffit openings.

Clean and roughen openings made in the bridge deck. Fill the deck openings with rapid setting concrete complying with section 15-5.02.

^^

49 PILING

07-19-13

Replace "sets" in the 1st paragraph of section 49-1.01C(2) with:

04-19-13

copies

Replace "set" in the 2nd paragraph of section 49-1.01C(2) with:

04-19-13

copy

Replace "Load Applied to Pile by Hydraulic Jack(s) Acting at One End of Test Beam(s) Anchored to the Pile" in the 5th paragraph of section 49-1.01D(2) with:

07-20-12

"Tensile Load Applied by Hydraulic Jack(s) Acting Upward at One End of Test Beam(s)"

Add to section 49-1.03:

04-20-12

Dispose of drill cuttings under section 19-2.03B.

Replace the paragraph of section 49-2.01A(1) with:

07-19-13

Section 49-2.01 includes general specifications for fabricating and installing driven piles.

Epoxy-coated bar reinforcing steel used for pile anchors must comply with section 52-2.02.

Replace the 2nd paragraph of section 49-2.01D with:

01-20-12

Furnish piling is measured along the longest side of the pile from the specified tip elevation shown to the plane of pile cutoff.

Replace the paragraph of section 49-2.02A(1) with:

07-19-13

Section 49-2.02 includes specifications for fabricating and installing steel pipe piles.

Replace the definitions in section 49-2.02A(2) with:

07-19-13

shop welding: Welding performed at a plant on the Department's Authorized Facility Audit List.

field welding: Welding not performed at a plant on the Department's Authorized Facility Audit List.

Replace item 2 in the list in the paragraph of section 49-2.02A(3)(b) with:

07-19-13

2. Certified mill test reports for each heat number of steel used in pipe piles being furnished.

Replace the paragraph of section 49-2.02A(4)(a) with:

07-19-13

Section 11-3.02 does not apply to shop welds in steel pipe piles fabricated at a facility on the Department's Authorized Facility Audit List.

For groove welds using submerged arc welding from both sides without backgouging, qualify the WPS under Table 4.5 of AWS D1.1.

Replace "0.45" in the 2nd paragraph of section 49-2.02B(1)(a) with:

07-19-13

0.47

Replace the 1st paragraph of section 49-2.02B(1)(b) with:

07-19-13

Welds must comply with AWS D1.1. Circumferential welds must be CJP welds.

07-19-13

Delete the 5th paragraph of section 49-2.02B(1)(b).

Add to section 49-2.02B(1):

07-19-13

49-2.02B(1)(d) Reserved

Replace "4.8.4" in item 2.3 in the list in the 2nd paragraph of section 49-2.02B(2) with:

07-19-13

4.9.4

07-19-13

Delete the 3rd paragraph of section 49-2.02C(2).

Replace the paragraph of section 49-2.03A(1) with:

07-19-13

Section 49-2.03 includes specifications for fabricating and installing structural shape steel piles.

Replace the paragraph of section 49-2.03A(3) with:

07-19-13

Submit a certified material test report and a certificate of compliance that includes a statement that all materials and workmanship incorporated in the work and all required tests and inspections of this work have been performed as described.

Replace the 1st paragraph of section 49-2.03B with:

07-19-13

Structural shape steel piles must comply with ASTM A 36/A 36M, ASTM A 572/A 572M, ASTM A 709/A 709M, or ASTM A 992/A 992M.

Replace "sets" in the 1st paragraph of section 49-2.04A(3) with:

04-19-13

copies

07-19-13

Delete the 1st paragraph of section 49-2.04A(4).

Replace the 3rd and 4th paragraphs of section 49-2.04B(2) with:

10-19-12

Piles in a corrosive environment must be steam or water cured under section 90-4.03.

If piles in a corrosive environment are steam cured, either:

1. Keep the piles continuously wet for at least 3 days. The 3 days includes the holding and steam curing periods.
2. Apply curing compound under section 90-1.03B(3) after steam curing.

Replace the 1st paragraph of section 49-3.01A with:

07-19-13

Section 49-3.01 includes general specifications for constructing CIP concrete piles.

Add to section 49-3.01A:

01-20-12

Concrete must comply with section 51.

Replace the 1st paragraph of section 49-3.01C with:

01-20-12

Except for CIDH concrete piles constructed under slurry, construct CIP concrete piles such that the excavation methods and the concrete placement procedures provide for placing the concrete against undisturbed material in a dry or dewatered hole.

Replace "Reserved" in section 49-3.02A(2) with:

01-20-12

dry hole:

1. Except for CIDH concrete piles specified as end bearing, a drilled hole that:
   1.1. Accumulates no more than 12 inches of water in the bottom of the drilled hole during a period of 1 hour without any pumping from the hole during the hour.
   1.2. Has no more than 3 inches of water in the bottom of the drilled hole immediately before placing concrete.
2. For CIDH concrete piles specified as end bearing, a drilled hole free of water without the use of pumps.

Replace "Reserved" in section 49-3.02A(3)(a) with:

01-20-12

If plastic spacers are proposed for use, submit the manufacturer's data and a sample of the plastic spacer. Allow 10 days for review.

Replace item 5 in the list in the 1st paragraph of section 49-3.02A(3)(b) with:

10-19-12

5. Methods and equipment for determining:
   5.1. Depth of concrete
   5.2. Theoretical volume of concrete to be placed, including the effects on volume if casings are withdrawn
   5.3. Actual volume of concrete placed

Add to the list in the 1st paragraph of section 49-3.02A(3)(b):

01-18-13

8. Drilling sequence and concrete placement plan.

Replace item 2 in the list in the 1st paragraph of section 49-3.02A(3)(g) with:

01-20-12

2. Be sealed and signed by an engineer who is registered as a civil engineer in the State. This requirement is waived for either of the following conditions:
   2.1. The proposed mitigation will be performed under the current Department-published version of *ADSC Standard Mitigation Plan 'A' - Basic Repair* without exception or modification.
   2.2. The Engineer determines that the rejected pile does not require mitigation due to structural, geotechnical, or corrosion concerns, and you elect to repair the pile using the current

Department-published version of *ADSC Standard Mitigation Plan 'B' - Grouting Repair* without exception or modification.

Replace "49-2.03A(4)(d)" in the 1st paragraph of section 49-3.02A(4)(d)(i) with:

07-19-13

49-3.02A(4)(d)

Add to the beginning of section 49-3.02A(4)(d)(ii):

07-19-13

If the drilled hole is dry or dewatered without the use of temporary casing to control ground water, installation of inspection pipes is not required.

Replace item 1 in the list in the 1st paragraph of section 49-3.02A(4)(d)(ii) with:

01-20-12

1. Inspection pipes must be schedule 40 PVC pipe complying with ASTM D 1785 with a nominal pipe size of 2 inches. Watertight PVC couplers complying with ASTM D 2466 are allowed to facilitate pipe lengths in excess of those commercially available. Log the location of the inspection pipe couplers with respect to the plane of pile cutoff.

Add to section 49-3.02A(4)(d)(iv):

01-20-12

If the Engineer determines it is not feasible to use one of ADSC's standard mitigation plans to mitigate the pile, schedule a meeting and meet with the Engineer before submitting a nonstandard mitigation plan.

The meeting attendees must include your representatives and the Engineer's representatives involved in the pile mitigation. The purpose of the meeting is to discuss the type of pile mitigation acceptable to the Department.

Provide the meeting facility. The Engineer conducts the meeting.

Replace the 1st paragraph of section 49-3.02B(5) with:

07-19-13

Grout must consist of cementitious material and water, and may contain an admixture if authorized. Do not exceed 5 gallons of water per 94 lb of cement.

Cementitious material must comply with section 90-1.02B, except SCMs are not required.

Water must comply with section 90-1.02D. If municipally supplied potable water is used, the testing specified in section 90-1.02D is waived.

Admixtures must comply with section 90, except admixtures must not contain chloride ions in excess of 0.25 percent by weight.

Use aggregate to extend the grout as follows:

1. Aggregate must consist of at least 70 percent fine aggregate and approximately 30 percent pea gravel, by weight.
2. Fine aggregate must comply with section 90-1.02C(3).
3. Size of pea gravel must be such that 100 percent passes the 1/2-inch sieve, at least 85 percent passes the 3/8-inch sieve, and not more than 5 percent passes the no. 8 sieve.
4. Minimum cementitious material content of the grout must not be less than 845 lb/cu yd of grout.

Mix the grout as follows:

1. Add water to the mixer followed by cementitious material, aggregates, and any admixtures.
2. Mix the grout with mechanical mixing equipment that produces a uniform and thoroughly mixed grout.
3. Agitate the grout continuously until the grout is pumped.
4. Do not add water after initial mixing.

Replace section 49-3.02B(8) with:

01-20-12

49-3.02B(8) Spacers

Spacers must comply with section 52-1.03D, except you may use plastic spacers.

Plastic spacers must:

1. Comply with sections 3.4 and 3.5 of the Concrete Reinforcing Steel Institute's *Manual of Standard Practice*
2. Have at least 25 percent of their gross plane area perforated to compensate for the difference in the coefficient of thermal expansion between the plastic and concrete
3. Be of commercial quality

Add between the 1st and 2nd paragraphs of section 49-3.02C(2):

07-19-13

For CIDH concrete piles with a pile cap, the horizontal tolerance at the center of each pile at pile cut-off is the larger of 1/24 of the pile diameter or 3 inches. The horizontal tolerance for the center-to-center spacing of 2 adjacent piles is the larger of 1/24 of the pile diameter or 3 inches.

Add to section 49-3.02C(4):

01-20-12

Unless otherwise shown, the bar reinforcing steel cage must have at least 3 inches of clear cover measured from the outside of the cage to the sides of the hole or casing.

Place spacers at least 5 inches clear from any inspection tubes.

Place plastic spacers around the circumference of the cage and at intervals along the length of the cage, as recommended by the manufacturer.

07-19-13

For a single CIDH concrete pile supporting a column:

1. If the pile and the column share the same reinforcing cage diameter, this cage must be accurately placed as shown
2. If the pile reinforcing cage is larger than the column cage and the concrete is placed under dry conditions, maintain a clear horizontal distance of at least 3.5 inches between the two cages
3. If the pile reinforcing cage is larger than the column cage and the concrete is placed under slurry, maintain a clear horizontal distance of at least 5 inches between the two cages

Replace section 49-3.02C(6) with:

07-19-13

49-3.02C(6) Construction Joint

Section 49-3.02C(6) applies to CIDH concrete piles where a construction joint is shown.

If a permanent steel casing is not shown, you must furnish and install a permanent casing. The permanent casing must:

1. Be watertight and of sufficient strength to prevent damage and to withstand the loads from installation procedures, drilling and tooling equipment, lateral concrete pressures, and earth pressures.
2. Extend at least 5 feet below the construction joint. If placing casing into rock, the casing must extend at least 2 feet below the construction joint.
3. Not extend above the top of the drilled hole or final grade whichever is lower.
4. Not increase the diameter of the CIDH concrete pile more than 2 feet.
5. Be installed by impact or vibratory hammers, oscillators, rotators, or by placing in a drilled hole. Casings placed in a drilled hole must comply with section 49-3.02C(5).

Section 49-2.01A(4)(b) does not apply to permanent casings specified in this section.

Add to section 49-4.01:

07-19-13

Steel soldier piles must comply with section 49-2.03.

Replace the headings and paragraphs in section 49-4.02 with:

07-19-13

Concrete anchors must comply with the specifications for studs in clause 7 of AWS D1.1.

^^

50 PRESTRESSING CONCRETE

07-19-13

Replace "sets" at each occurrence in the 2nd and 3rd paragraphs of section 50-1.01C(3) with:

04-19-13

copies

Add to section 50-1.01C(3):

07-19-13

Include a grouting plan with your shop drawing submittal. The grouting plan must include:

1. Detailed grouting procedures
2. Type, quantity, and brand of materials to be used
3. Type of equipment to be used including provisions for backup equipment
4. Types and locations of grout inlets, outlets, and vents
5. Methods to clean ducts before grouting
6. Methods to control the rate of flow within ducts
7. Theoretical grout volume calculations for each duct
8. Duct repair procedures due to an air pressure test failure
9. Mixing and pumping procedures
10. Direction of grouting
11. Sequence of use of inlets and outlets
12. Procedure for handling blockages
13. Proposed forms for recording grouting information
14. Procedure for secondary grouting
15. Names of people who will perform grouting activities including their relevant experience and certifications

Add to section 50-1.01C:

07-19-13

50-1.01C(5) Grout

Submit a daily grouting report for each day grouting is performed. Submit the report within 3 days after grouting. The report must be signed by the technician supervising the grouting activity. The report must include:

1. Identification of each tendon
2. Date grouting occurred
3. Time the grouting started and ended
4. Date of placing the prestressing steel in the ducts
5. Date of stressing
6. Type of grout used
7. Injection end and applied grouting pressure
8. Actual and theoretical quantity of grout used to fill duct
9. Ratio of actual to theoretical grout quantity
10. Records of air, grout, and structure surface temperatures during grouting.
11. Summary of tests performed and results, except submit compressive strength and chloride ion test results within 48 hours of test completion
12. Names of personnel performing the grouting activity
13. Summary of problems encountered and corrective actions taken
14. Summary of void investigations and repairs made

Replace the introductory clause in the 1st paragraph of section 50-1.01C(4) with:

07-19-13

Submit test samples for the materials shown in the following table to be used in the work:

Add between "the" and "test samples" in the 1st paragraph of section 50-1.01D(2):

07-19-13

prestressing steel

Replace the 3rd paragraph of section 50-1.01D(2) with:

10-19-12

The Department may verify the prestressing force using the Department's load cells.

Replace the 3rd paragraph in section 50-1.01D(3) with:

07-19-13

Each pressure gage must be fully functional and have an accurately reading, clearly visible dial or display. The dial must be at least 6 inches in diameter and graduated in 100 psi increments or less.

Add between the 5th and 6th paragraphs of section 50-1.01D(3):

07-19-13

Each jack and its gages must be calibrated as a unit.

Replace the 6th paragraph in section 50-1.01D(3) with:

07-19-13

Each jack used to tension prestressing steel permanently anchored at 25 percent or more of its specified minimum ultimate tensile strength must be calibrated by METS within 1 year of use and after each repair. You must:

1. Schedule the calibration of the jacking equipment with METS
2. Mechanically calibrate the gages with a dead weight tester or other authorized means before calibration of the jacking equipment by METS
3. Verify that the jack and supporting systems are complete, with proper components, and are in good operating condition
4. Provide labor, equipment, and material to (1) install and support the jacking and calibration equipment and (2) remove the equipment after the calibration is complete
5. Plot the calibration results

Each jack used to tension prestressing steel permanently anchored at less than 25 percent of its specified minimum ultimate tensile strength must be calibrated by an authorized laboratory within 6 months of use and after each repair.

Add to section 50-1.01D:

07-19-13

50-1.01D(4) Pressure Testing Ducts

For post-tensioned concrete bridges, pressure test each duct with compressed air after stressing. To pressure test the ducts:

1. Seal all inlets, outlets, and grout caps.
2. Open all inlets and outlets on adjacent ducts.
3. Attach an air compressor to an inlet at 1 end of the duct. The attachment must include a valve that separates the duct from the air source.
4. Attach a pressure gage to the inlet at the end of the duct.
5. Pressurize the duct to 50 psi.
6. Lock-off the air source.
7. Record the pressure loss after 1 minute.
8. If there is a pressure loss exceeding 25 psi, repair the leaks with authorized methods and retest.

Compressed air used to clear and test the ducts must be clean, dry, and free of oil or contaminants.

50-1.01D(5) Duct Demonstration of Post-Tensioned Members

Before placing forms for deck slabs of box girder bridges, demonstrate that any prestressing steel placed in the ducts is free and unbonded. If no prestressing steel is in the ducts, demonstrate that the ducts are unobstructed.

If prestressing steel is installed after the concrete is placed, demonstrate that the ducts are free of water and debris immediately before installing the steel.

Before post-tensioning any member, demonstrate that the prestressing steel is free and unbonded in the duct.

The Engineer must witness all demonstrations.

50-1.01D(6) Void Investigation

In the presence of the Engineer, investigate the ducts for voids between 24 hours and 72 hours after grouting completion. As a minimum, inspect the inlet and outlet ports at the anchorages and at high points in the tendons for voids after removal. Completely fill any voids found with secondary grout.

50-1.01D(7) Personnel Qualifications

Perform post-tensioning field activities, including grouting, under the direct supervision of a technician certified as a level 2 Bonded PT Field Specialist through the Post-Tensioning Institute. Grouting activities may be performed under the direct supervision of a technician certified as a Grouting Technician through the American Segmental Bridge Institute.

Replace the 6th paragraph of section 50-1.02B with:

07-19-13

Package the prestressing steel in containers or shipping forms that protect the steel against physical damage and corrosion during shipping and storage.

Replace the 13th paragraph of section 50-1.02B with:

07-19-13

Prestressing steel is rejected if surface rust either:

1. Cannot be removed by hand-cleaning with a fine steel wool pad
2. Leaves pits visible to the unaided eye after cleaning

Replace the 4th paragraph of section 50-1.02C with:

07-19-13

Admixtures must comply with section 90, except admixtures must not contain chloride ions in excess of 0.25 percent by weight.

07-19-13

Delete the 5th paragraphs of section 50-1.02C.

Add to section 50-1.02C:

07-19-13

Secondary grout must:

1. Comply with ASTM C 1107
2. Not have a deleterious effect on the steel, concrete, or bond strength of the steel to concrete

Replace item 9 including items 9.1 and 9.2 in the list in the 1st paragraph of section 50-1.02D with:

07-19-13

9. Have an inside cross-sectional area of at least 2.5 times the net area of the prestressing steel for multistrand tendons

Replace "3/8" in item 10 in the list in the 1st paragraph of section 50-1.02D with:

07-19-13

1/2

07-19-13

Delete the 2nd sentences in the 1st paragraph of section 50-1.02E.

Replace section 50-1.02F with:

07-19-13

50-1.02F Permanent Grout Caps

Permanent grout caps for anchorage systems of post-tensioned tendons must:

1. Be glass-fiber-reinforced plastic with antioxidant additives. The environmental stress-cracking failure time must be at least 192 hours under ASTM D 1693, Condition C.

2. Completely cover and seal the wedge plate or anchorage head and all exposed metal parts of the anchorage against the bearing plate using neoprene O-ring seals.
3. Have a grout vent at the top of the cap.
4. Be bolted to the anchorage with stainless steel complying with ASTM F 593, alloy 316. All fasteners, including nuts and washers, must be alloy 316.
5. Be pressure rated at or above 150 psi.

Add to section 50-1.02:

09-16-11

50-1.02G Sheathing

Sheathing for debonding prestressing strand must:

1. Be split or un-split flexible polymer plastic tubing
2. Have a minimum wall thickness of 0.025 inch
3. Have an inside diameter exceeding the maximum outside diameter of the strand by 0.025 to 0.14 inch

Split sheathing must overlap at least 3/8 inch.

Waterproofing tape used to seal the ends of the sheathing must be flexible adhesive tape.

The sheathing and waterproof tape must not react with the concrete, coating, or steel.

Replace the 2nd paragraph of section 50-1.03A(3) with:

07-19-13

After installation, cover the duct ends and vents to prevent water or debris from entering.

Add to section 50-1.03A(3):

07-19-13

Support ducts vertically and horizontally during concrete placement at a spacing of at most 4 feet.

07-19-13

Delete "at least" in the 1st paragraph of section 50-1.03B(1).

Add to section 50-1.03B(1):

01-20-12

After seating, the maximum tensile stress in the prestressing steel must not exceed 75 percent of the minimum ultimate tensile strength shown.

07-19-13

Delete the 1st through 4th paragraphs of section 50-1.03B(2)(a).

Replace "temporary tensile strength" in the 7th paragraph of section 50-1.03B(2)(a) with:

07-19-13

temporary tensile stress

Add to section 50-1.03B(2)(a):

07-19-13

If prestressing strand is installed using the push-through method, use guide caps at the front end of each strand to protect the duct from damage.

Add to the list in the 2nd paragraph of section 50-1.03B(2)(c):

07-19-13

3. Be equipped with permanent grout caps

Replace section 50-1.03B(2)(d) with:

07-19-13

50-1.03B(2)(d) Bonding and Grouting

50-1.03B(2)(d)(i) General

Bond the post-tensioned prestressing steel to the concrete by completely filling the entire void space between the duct and the prestressing steel with grout.

Ducts, vents, and grout caps must be clean and free from water and deleterious materials that would impair bonding of the grout or interfere with grouting procedures. Compressed air used for cleaning must be clean, dry, and free of oil or contaminants.

Prevent the leakage of grout through the anchorage assembly by positive mechanical means.

Before starting daily grouting activities, drain the pump system to remove any water from the piping system.

Break down and thoroughly clean the pump and piping system after each grouting session.

After completing duct grouting activities:

1. Abrasive blast clean and expose the aggregate of concrete surfaces where concrete is to be placed to cover and encase the anchorage assemblies
2. Remove the ends of vents 1 inch below the roadway surface

50-1.03B(2)(d)(ii) Mixing and Proportioning

Proportion solids by weight to an accuracy of 2 percent.

Proportion liquids by weight or volume to an accuracy of 1 percent.

Mix the grout as follows:

1. Add water to the mixer followed by the other ingredients.
2. Mix the grout with mechanical mixing equipment that produces a uniform and thoroughly mixed grout without an excessive temperature increase or loss of properties of the mixture.
3. Do not exceed 5 gal of water per 94 lb of cement or the quantity of water in the manufacturer's instructions, whichever is less.
4. Agitate the grout continuously until the grout is pumped. Do not add water after the initial mixing.

50-1.03B(2)(d)(iii) Placing

Pump grout into the duct within 30 minutes of the 1st addition of the mix components.

Inject grout from the lowest point of the duct in an uphill direction in 1 continuous operation maintaining a one-way flow of the grout. You may inject from the lowest anchorage if complete filling is ensured.

Before injecting grout, open all vents.

Continuously discharge grout from the vent to be closed. Do not close any vent until free water, visible slugs of grout, and entrapped air have been ejected and the consistency of the grout flowing from the vent is equivalent to the injected grout.

Pump the grout at a rate of 16 to 50 feet of duct per minute.

Conduct grouting at a pressure range of 10 to 50 psi measured at the grout inlet. Do not exceed maximum pumping pressure of 150 psi at the grout inlet.

As grout is injected, close the vents in sequence in the direction of flow starting with the closest vent.

Before closing the final vent at the grout cap, discharge at least 2 gal of grout into a clean receptacle.

Bleed all high point vents.

Lock a pressure of 5 psi into the duct by closing the grout inlet valve.

50-1.03B(2)(d)(iv) Weather Conditions

If hot weather conditions will contribute to quick stiffening of the grout, cool the grout by authorized methods as necessary to prevent blockages during pumping activities.

If freezing weather conditions are anticipated during and following the placement of grout, provide adequate means to protect the grout in the ducts from damage by freezing.

50-1.03B(2)(d)(v) Curing

During grouting and for a period of 24 hours after grouting, eliminate vibration from contractor controlled sources within 100 feet of the span in which grouting is taking place, including from moving vehicles, jackhammers, large compressors or generators, pile driving activities, soil compaction, and falsework removal. Do not vary loads on the span.

For PC concrete members, do not move or disturb the members after grouting for 24 hours. If ambient temperature drops below 50 degrees F, do not move or disturb the members for 48 hours.

Do not remove or open valves until grout has cured for at least 24 hours.

50-1.03B(2)(d)(vi) Grouting Equipment

Grouting equipment must be:

1. Capable of grouting at a pressure of at least 100 psi
2. Equipped with a pressure gage having a full-scale reading of not more than 300 psi
3. Able to continuously grout the longest tendon on the project in less than 20 minutes

Grout must pass through a screen with clear openings of 1/16 inch or less before entering the pump.

Fit grout injection pipes, ejection pipes, and vents with positive mechanical shutoff valves capable of withstanding the pumping pressures. Do not remove or open valves until the grout has set. If authorized, you may substitute mechanical valves with suitable alternatives after demonstrating their effectiveness.

Provide a standby grout mixer and pump.

50-1.03B(2)(d)(vii) Grout Storage

Store grout in a dry environment.

50-1.03B(2)(d)(viii) Blockages

If the grouting pressure reaches 150 psi, close the inlet and pump the grout at the next vent that has just been or is ready to be closed as long as a one-way flow is maintained. Do not pump grout into a succeeding outlet from which grout has not yet flowed.

When complete grouting of the tendon cannot be achieved by the steps specified, stop the grouting operation.

50-1.03B(2)(d)(ix) Secondary Grouting

Perform secondary grouting by vacuum grouting under the direct supervision of a person who has been trained and has experience in the use of vacuum grouting equipment and procedures.

The vacuum grouting process must be able to determine the size of the void and measure the volume of grout filling the void.

Vacuum grouting equipment must consist of:

1. Volumeter for the measurement of void volume
2. Vacuum pump with capacity of at least 10 cfm and equipped with a flow meter capable of measuring the amount of grout being injected

50-1.03B(2)(d)(x) Vertical Tendon Grouting

Provide a standpipe at the upper end of the tendon to collect bleed water and allow it to be removed from the grout. The standpipe must be large enough to prevent the grout elevation from dropping below the highest point of the upper anchorage device. If the grout level drops to the highest point of the upper anchorage device, immediately add grout to the standpipe.

Remove the standpipe after the grout has hardened.

For vertical tendons in excess of 100 feet high or if grouting pressure exceeds 145 psi, inject grout at a higher vent from which grout has already flowed to maintain one-way flow.

50-1.03B(2)(d)(xi) Vents

Place vents at the following locations:

1. Anchorage areas at both ends of the tendon
2. Each high point
3. 4 feet upstream and downstream of each crest of a high point
4. Each change in the cross section of duct

Add to section 50-1.03B(2):

09-16-11

50-1.03B(2)(e) Debonding Prestressing Strands

Where shown, debond prestressing strands by encasing the strands in plastic sheathing along the entire length shown and sealing the ends of the sheathing with waterproof tape.

Distribute the debonded strands symmetrically about the vertical centerline of the girder. The debonded lengths of pairs of strands must be equal.

Do not terminate debonding at any one cross section of the member for more than 40 percent of the debonded strands or 4 strands, whichever is greater.

Thoroughly seal the ends with waterproof tape to prevent the intrusion of water or cement paste before placing the concrete.

^^

51 CONCRETE STRUCTURES

04-18-14

Replace the paragraphs of section 51-1.01A with:

10-19-12

Section 51-1 includes general specifications for constructing concrete structures.

Earthwork for the following concrete structures must comply with section 19-3:

1. Sound wall footings
2. Sound wall pile caps
3. Culverts
4. Barrier slabs
5. Junction structures
6. Minor structures

7. Pipe culvert headwalls, endwalls, and wingwalls for a pipe with a diameter of 5 feet or greater

Falsework must comply with section 48-2.

Joints must comply with section 51-2.

Elastomeric bearing pads must comply with section 51-3.

Reinforcement for the following concrete structures must comply with section 52:

1. Sound wall footings
2. Sound wall pile caps
3. Barrier slabs
4. Junction structures
5. Minor structures
6. PC concrete members

You may use RSC for a concrete structure only where the specifications allow the use of RSC.

Replace "sets" in the 1st paragraph of section 51-1.01C(2) with:

07-19-13

copies

Replace the heading of section 51-1.01D(4) with:

04-19-13

Testing Concrete Surfaces

Add to section 51-1.01D(4)(a):

04-19-13

The Engineer tests POC deck surfaces for smoothness and crack intensity.

Add to the list in the 1st paragraph of section 51-1.01D(4)(b):

04-19-13

3. Completed deck surfaces, including ramps and landings of POCs

Replace the 4th paragraph in section 51-1.01D(4)(b) with:

04-19-13

Except for POCs, surface smoothness is tested using a bridge profilograph under California Test 547. Two profiles are obtained in each lane approximately 3 feet from the lane lines and 1 profile is obtained in each shoulder approximately 3 feet from the curb or rail face. Profiles are taken parallel to the direction of traffic.

Add between the 5th and 6th paragraphs of section 51-1.01D(4)(b):

04-19-13

POC deck surfaces must comply with the following smoothness requirements:

1. Surfaces between grade changes must not vary more than 0.02 foot from the lower edge of a 12-foot-long straightedge placed parallel to the centerline of the POC
2. Surface must not vary more than 0.01 foot from the lower edge of a 6-foot-long straightedge placed perpendicular to the centerline of the POC

Add to section 51-1.01D(4)(d):

04-19-13

The Engineer measures crack intensity of POC deck surfaces after curing, before prestressing, and before falsework release. Clean the surface for the Engineer to measure surface crack intensity.

In any 100 sq ft portion of a new POC deck surface, if there are more than 10 feet of cracks having a width at any point of over 0.02 inch, treat the deck with methacrylate resin under section 15-5.05. Treat the entire deck width between the curbs to 5 feet beyond where the furthest continuous crack emanating from the 100 sq ft section is 0.02 inch wide. Treat the deck surface before grinding.

Replace the 2nd paragraph of section 51-1.02B with:

07-19-13

Except for minor structures, the minimum required 28-day compressive strength for concrete in structures or portions of structures is the compressive strength described or 3,600 psi, whichever is greater.

Add to section 51-1.03C(2)(c)(i):

04-20-12

Permanent steel deck forms are only allowed where shown or if specified as an option in the special provisions.

Replace the 3rd paragraph of section 51-1.03C(2)(c)(ii) with:

04-20-12

Compute the physical design properties under AISI's *North American Specification for the Design of Cold-Formed Steel Structural Members*.

Replace the 8th paragraph of section 51-1.03D(1) with:

10-19-12

Except for concrete placed as pipe culvert headwalls and endwalls, slope paving and aprons, and concrete placed under water, consolidate concrete using high-frequency internal vibrators within 15 minutes of placing concrete in the forms. Do not attach vibrators to or hold them against forms or reinforcing steel. Do not displace reinforcement, ducts, or prestressing steel during vibrating.

Add to section 51-1.03E(5):

08-05-11

Drill the holes without damaging the adjacent concrete. If reinforcement is encountered during drilling before the specified depth is attained, notify the Engineer. Unless coring through the reinforcement is authorized, drill a new hole adjacent to the rejected hole to the depth shown.

Add to section 51-1.03F(5)(a):

04-19-13

For approach slabs, sleeper slabs, and other roadway surfaces of concrete structures, texture the roadway surface as specified for bridge deck surfaces in section 51-1.03F(5)(b).

Replace "Reserved" in section 51-1.03F(5)(b) with:

04-20-12

51-1.03F(5)(b)(i) General

Except for bridge widenings, texture the bridge deck surfaces longitudinally by grinding and grooving or by longitudinal tining.

10-19-12

For bridge widenings, texture the deck surface longitudinally by longitudinal tining.

04-20-12

In freeze-thaw areas, do not texture PCC surfaces of bridge decks.

51-1.03F(5)(b)(ii) Grinding and Grooving

When texturing the deck surface by grinding and grooving, place a 1/4 inch of sacrificial concrete cover on the bridge deck above the finished grade shown. Place items to be embedded in the concrete based on the final profile grade elevations shown. Construct joint seals after completing the grinding and grooving.

Before grinding and grooving, deck surfaces must comply with the smoothness and deck crack treatment requirements.

Grind and groove the deck surface as follows:

1. Grind the surface to within 18 inches of the toe of the barrier under section 42-3. Grinding must not reduce the concrete cover on reinforcing steel to less than 1-3/4 inches.
2. Groove the ground surfaces longitudinally under section 42-2. The grooves must be parallel to the centerline.

51-1.03F(5)(b)(iii) Longitudinal Tining

When texturing the deck surface by longitudinal tining, perform initial texturing with a burlap drag or broom device that produces striations parallel to the centerline. Perform final texturing with spring steel tines that produce grooves parallel with the centerline.

The tines must:

1. Be rectangular in cross section
2. Be from 3/32 to 1/8 inch wide on 3/4-inch centers
3. Have enough length, thickness, and resilience to form grooves approximately 3/16 inch deep

Construct grooves to within 6 inches of the layout line of the concrete barrier toe. Grooves must be from 1/8 to 3/16 inch deep and 3/16 inch wide after concrete has hardened.

For irregular areas and areas inaccessible to the grooving machine, you may hand construct grooves. Hand-constructed grooves must comply with the specifications for machine-constructed grooves.

Tining must not cause tearing of the deck surface or visible separation of coarse aggregate at the surface.

Add to section 51-1.03F:

04-19-13

51-1.03F(6) Finishing Pedestrian Overcrossing Surfaces

Construct deck surfaces, including ramps and landings of POCs to the grade and cross section shown. Surfaces must comply with the specified smoothness, surface texture, and surface crack requirements.

The Engineer sets deck elevation control points for your use in establishing the grade and cross section of the deck surface. The grade established by the deck elevation control points includes all camber allowances. Except for landings, elevation control points include the beginning and end of the ramp and will not be closer together than approximately 8 feet longitudinally and 4 feet transversely to the POC centerline. Landing elevation control points are at the beginning and the end of the landing.

Broom finish the deck surfaces of POCs. Apply the broom finish perpendicular to the path of travel. You may apply water mist to the surface immediately before brooming.

Clean any discolored concrete by abrasive blast cleaning or other authorized methods.

Replace the paragraphs of section 51-1.04 with:

10-19-12

If concrete involved in bridge work is not designated by type and is not otherwise paid for under a separate bid item, the concrete is paid for as structural concrete, bridge.

The payment quantity for structural concrete includes the volume in the concrete occupied by bar reinforcing steel, structural steel, prestressing steel materials, and piling.

The payment quantity for seal course concrete is the actual volume of seal course concrete placed except the payment quantity must not exceed the volume of concrete contained between vertical planes 1 foot outside the neat lines of the seal course shown. The Department does not adjust the unit price for an increase or decrease in the seal course concrete quantity.

Structural concrete for pier columns is measured as follows:

1. Horizontal limits are vertical planes at the neat lines of the pier column shown.
2. Bottom limit is the bottom of the foundation excavation in the completed work.
3. Upper limit is the top of the pier column concrete shown.

The payment quantity for drill and bond dowel is determined from the number and depths of the holes shown.

Replace section 51-2.01B(2) with:

04-19-13

51-2.01B(2) Reserved

04-19-13

Delete the 4th paragraph of section 51-2.01C.

Replace "SSPC-QP 3" in the 1st paragraph of section 51-2.02A(2) with:

10-19-12

AISC-420-10/SSPC-QP 3

Replace the 2nd and 3rd paragraphs of section 51-2.02B(3)(b) with:

04-20-12

Concrete saws for cutting grooves in the concrete must have diamond blades with a minimum thickness of 3/16 inch. Cut both sides of the groove simultaneously for a minimum 1st pass depth of 2 inches. The completed groove must have:

1. Top width within 1/8 inch of the width shown or ordered
2. Bottom width not varying from the top width by more than 1/16 inch for each 2 inches of depth
3. Uniform width and depth

Cutting grooves in existing decks includes cutting any conflicting reinforcing steel.

Replace "sets" in the 1st and 2nd paragraphs of section 51-2.02D(1)(c)(ii) with:

04-19-13

copies

Replace "set" in the 7th paragraph of section 51-2.02D(1)(c)(ii) with:

04-19-13

copy

Add to the 1st paragraph of section 51-2.02D(3):

04-19-13

POC deck surfaces must comply with section 51-1.03F(6) before placing and anchoring joint seal assemblies.

Replace "sets" in the 2nd paragraph of section 51-2.02E(1)(c) with:

04-19-13

copies

Replace "set" in the 6th paragraph of section 51-2.02E(1)(c) with:

04-19-13

copy

Replace the 2nd paragraph of section 51-2.02E(1)(e) with:

08-05-11

Except for components in contact with the tires, the design loading must be the AASHTO LRFD Bridge Design Specifications Design Truck with 100 percent dynamic load allowance. Each component in contact with the tires must support a minimum of 80 percent of the AASHTO LRFD Bridge Design Specifications Design Truck with 100 percent dynamic load allowance. The tire contact area must be 10 inches measured normal to the longitudinal assembly axis by 20 inches wide. The assembly must provide a smooth-riding joint without slapping of components or tire rumble.

Replace "sets" in the 1st and 2nd paragraphs of section 51-2.02F(1)(c) with:

04-19-13

copies

Add between the 1st and 2nd paragraphs of section 51-4.01A:

10-19-12

Prestressing concrete members must comply with section 50.

04-20-12

Delete the 2nd paragraph of section 51-4.01A.

Replace the 3rd paragraph of section 51-4.01C(2) with:

04-20-12

For segmental or spliced-girder construction, shop drawings must include the following additional information:

1. Details showing construction joints or closure joints
2. Arrangement of bar reinforcing steel, prestressing tendons, and pressure-grouting pipe
3. Materials and methods for making closures
4. Construction joint keys and surface treatment
5. Other requested information

For segmental girder construction, shop drawings must include concrete form and casting details.

Replace "sets" in the 1st paragraph of section 51-4.01C(3) with:

04-19-13

copies

10-19-12

Delete the 1st and 2nd paragraphs of section 51-4.02A.

Replace the 3rd paragraph of section 51-4.02B(2) with:

04-20-12

For segmental or spliced-girder construction, materials for construction joints or closure joints at exterior girders must match the color and texture of the adjoining concrete.

Add to section 51-4.02B(2):

04-20-12

At spliced-girder closure joints:

1. If shear keys are not shown, the vertical surfaces of the girder segment ends must be given a coarse texture as specified for the top surface of PC members.
2. Post-tensioning ducts must extend out of the vertical surface of the girder segment closure end sufficiently to facilitate splicing of the duct.

For spliced girders, pretension strand extending from the closure end of the girder segment to be embedded in the closure joint must be free of mortar, oil, dirt, excessive mill scale and scabby rust, and other coatings that would destroy or reduce the bond.

Add to section 51-4.03B:

04-20-12

The specifications for prestressing force distribution and sequencing of stressing in the post-tensioning activity in 50-1.03B(2)(a) do not apply if post-tensioning of spliced girders before starting deck construction is described. The composite deck-girder structure must be post-tensioned in a subsequent stage.

Temporary spliced-girder supports must comply with the specifications for falsework in section 48-2.

Before post-tensioning of spliced girders, remove the forms at CIP concrete closures and intermediate diaphragms to allow inspection for concrete consolidation.

Add to section 51-5.01A:

07-19-13

Structure excavation and backfill must comply with section 19-3.

Treated permeable base must comply with section 29.

Replace the paragraph in section 51-5.02G with:

04-18-14

HMA for a temporary roadway structural section must comply with the specifications for minor HMA in section 39.

07-19-13

Delete the 1st paragraph of section 51-5.03B(3).

07-19-13

Delete the 2nd paragraph of section 51-5.03D(1).

Add between the 1st and 2nd paragraphs of section 51-7.01A:

10-19-12

Minor structures include:

1. Pipe culvert headwalls and endwalls for a pipe with a diameter less than 5 feet
2. Drainage inlets
3. Other structures described as minor structures

10-19-12

Delete the 4th paragraph of section 51-7.01A.

Replace the 1st and 2nd paragraphs of section 51-7.01B with:

10-19-12

Concrete must comply with the specifications for minor concrete.

Add to section 51:

10-19-12

51-8–51-15 RESERVED

^^

52 REINFORCEMENT

01-18-13

Add to section 52-1.01A:

07-20-12

Splicing of bar reinforcement must comply with section 52-6.

Replace the 1st and 2nd paragraphs of section 52-1.02B with:

10-19-12

Reinforcing bars must be deformed bars complying with ASTM A 706/A 706M, Grade 60, except you may use:

1. Deformed bars complying with ASTM A 615/A 615M, Grade 60, in:
   - 1.1. Junction structures
   - 1.2. Sign and signal foundations
   - 1.3. Minor structures

1.4. Concrete crib members
1.5. Mechanically-stabilized-embankment concrete panels
1.6. Masonry block sound walls

2. Deformed or plain bars complying with ASTM A 615/A 615M, Grade 40 or 60, in:
   2.1. Slope and channel paving
   2.2. Concrete barriers Type 50 and 60
3. Plain bars for spiral or hoop reinforcement in structures and concrete piles

Add to the list in the 3rd paragraph of section 52-1.02B:

04-20-12

9. Shear reinforcement stirrups in PC girders

Replace the 6th paragraph of section 52-6.01D(4)(a) with:

01-18-13

Before performing service splice or ultimate butt splice testing, perform total slip testing on the service splice or ultimate butt splice test samples under section 52-6.01D(4)(b).

Replace section 52-6.02D with:

10-21-11

52-6.02D Ultimate Butt Splice Requirements

When tested under California Test 670, ultimate butt splice test samples must demonstrate necking as either of the following:

1. For "Necking (Option I)," the test sample must rupture in the reinforcing bar outside of the affected zone and show visible necking.
2. For "Necking (Option II)," the largest measured strain must be at least:
   2.1. Six percent for no. 11 and larger bars
   2.2. Nine percent for no. 10 and smaller bars

Replace the 2nd and 3rd paragraphs of section 52-6.03B with:

01-18-13

Do not splice the following by lapping:

1. No. 14 bars
2. No. 18 bars
3. Hoops
4. Reinforcing bars where you cannot provide a minimum clear distance of 2 inches between the splice and the nearest adjacent bar

^^

53 SHOTCRETE

07-19-13

Replace the 2nd and 3rd paragraphs of section 53-2.01D(1) with:

07-19-13

Obtain and test all cores for compressive strength under ASTM C 42/C 42M at an authorized laboratory. The compressive strength is the average strength of the 3 cores.

Shotcrete must have a minimum compressive strength of 3,600 psi, unless otherwise described. The shotcrete must attain the minimum compressive strength at 28 days, except 42 days are allowed for shotcrete with a described minimum compressive strength greater than 3,600 psi.

^^

54 WATERPROOFING

04-18-14

Add between "be" and "3/8 inch" in the 3rd paragraph of section 54-4.02C:

04-20-12

at least

04-18-14

Delete the 3rd paragraph of section 54-5.01A.

04-18-14

Delete "and HMA" in the 2nd paragraph of section 54-5.03.

Replace the last paragraph of section 54-5.03 with:

04-18-14

Do not allow traffic on the seal until HMA is placed over it.

Add to section 54:

07-19-13

54-7 SILANE WATERPROOFING TREATMENT

Reserved

54-8–54-10 RESERVED

^^

55 STEEL STRUCTURES

07-19-13

07-19-13

Delete the 3rd paragraph in section 55-1.01C(1).

Replace the 3rd sentence of the 4th paragraph in section 55-1.01C(1) with:

07-19-13

For ASTM F 1554 anchor bolts, include chemical composition and carbon equivalence for each heat of steel.

Add to section 55-1.01C(1):

07-19-13

For HS connections, submit a record of which lots are used in each joint as an informational submittal.

Replace "sets" at each occurrence in the 1st paragraph of section 55-1.01C(2) with:

04-19-13

copies

Replace the list in the 2nd paragraph of section 55-1.01C(2) with:

07-19-13

1. Sequence of shop and field assembly and erection. For continuous members, include proposed steel erection procedures with calculations that show girder capacity and geometry will be correct.
2. Welding sequences and procedures.
3. Layout drawing of the entire structure with locations of butt welded splices.
4. Locations of temporary supports and welds.
5. Vertical alignment of girders at each stage of erection.
6. Match-marking diagrams.
7. Details for connections not shown or dimensioned on the plans.
8. Details of allowed options incorporated in the work.
9. Direction of rolling of plates where orientation is specified.
10. Distortion control plan.
11. Dimensional tolerances. Include measures for controlling accumulated error to meet overall tolerances.
12. Material specification and grade listed on the bill of materials.
13. Identification of tension members and fracture critical members.
14. Proposed deviations from plans, specifications, or previously submitted shop drawings.
15. Contract plan sheet references for details.

Replace items 2 and 3 in the list in the 1st paragraph of section 55-1.01C(3) with:

07-19-13

2. Tension flanges and webs of horizontally curved girders
3. Hanger plates

Replace the 2nd paragraph of section 55-1.01C(3) with:

07-19-13

Furnish plates, shapes, or bars with extra length to provide for removal of check samples.

07-19-13

Delete the 1st and 2nd sentences in the 3rd paragraph of section 55-1.01C(3).

Replace the 4th paragraph of section 55-1.01C(3) with:

07-19-13

Remove material for test samples in the Engineer's presence. Test samples for plates over 24 inches wide must be 10 by 12 inches with the long dimension transverse to the direction of rolling. Test samples for other products must be 12 inches long taken in the direction of rolling with a width equal to the product width.

Replace the 1st sentence of the 6th paragraph in section 55-1.01C(3) with:

07-19-13

Results of check testing are delivered to you within 20 days of receipt of samples at METS.

07-19-13

Delete the 2nd paragraph of section 55-1.01D(1).

Replace the 2nd sentence of the 4th paragraph in section 55-1.01D(1) with:

07-19-13

The calibration must be performed by an authorized repair and calibration center approved by the tool manufacturer.

Add to section 55-1.01D(1):

07-19-13

For bolts installed as snug tight, rotational capacity testing and installation tension testing are not required.

In addition to NDT requirements in AWS D1.5, ultrasonically test 25 percent of all main member tension butt welds in material over 1/2 inch thick.

Perform NDT on 100 percent of each pin as follows:

1. MT under ASTM A 788, S 18, with no linear indication allowed exceeding 3 mm
2. UT under ASTM A 788, S 20, level S and level DA in two perpendicular directions

The Engineer determines the location of all NDT testing for welding.

07-19-13

Delete the 2nd paragraph of section 55-1.01D(3)(a).

Replace section 55-1.01D(4)(b) with:

07-19-13

Perform rotational capacity testing on each rotational capacity lot under section 55-1.01D(3)(b) at the job site before installation.

Replace the 1st sentence of the 2nd paragraph in section 55-1.01D(4)(c) with:

07-19-13

Test 3 representative HS fastener assemblies under section 8 of *Specification for Structural Joints Using High-Strength Bolts* of the RCSC.

Replace the 1st paragraph in section 55-1.01D(4)(d) with:

07-19-13

Perform fastener tension testing to verify minimum tension in HS bolted connections no later than 48 hours after all fasteners in a connection have been tensioned.

Replace the 3rd paragraph in section 55-1.01D(4)(d) with:

07-19-13

Test 10 percent of each type of fastener assembly in each HS bolted connection for minimum tension using the procedure described in section 10 of *Specification for Structural Joints Using High-Strength Bolts* of the RCSC. Check at least 2 assemblies per connection. For short bolts, determine the inspection torque using steps 1 through 7 of "Arbitration of Disputes, Torque Method-Short Bolts" in *Structural Bolting Handbook* of the Steel Structures Technology Center.

Replace the 1st table in the 1st paragraph of section 55-1.02A(1) with:

07-19-13

Structural Steel

Material	Specification
Carbon steel	ASTM A 709/A 709M, Grade 36 or {ASTM A36/A36M}[a]
HS low alloy columbium vanadium steel	ASTM A 709/A 709M, Grade 50 or {ASTM A 992/A 992M or ASTM A 572/A 572M, Grade 50}[a]
HS low alloy structural steel	ASTM A 709/A 709M, Grade 50W or Grade HPS 50W, or {ASTM A 588/A 588M}[a]
HS low alloy structural steel plate	ASTM A 709/A 709M, Grade HPS 70W
High-yield strength quenched and tempered alloy steel plate suitable for welding	ASTM A 709/A 709M, Grade 100, Grade 100W, or Grade HPS 100W, or {ASTM A 514/A 514M}[a]

[a]Grades you may substitute for the equivalent ASTM A 709 steel subject to the modifications and additions specified and to the requirements of ASTM A 709.

Replace the 2nd table in the 1st paragraph of section 55-1.02A(1) with:

07-19-13

Fasteners

Material	Specification
Steel fastener components for general applications:	
Bolts and studs	ASTM A 307
Anchor bolts	ASTM F 1554[a]
HS bolts and studs	ASTM A 449, Type 1[a]
HS threaded rods	ASTM A 449, Type 1 [a]
HS nonheaded anchor bolts	ASTM F 1554, Grade 105, Class 2A[a]
Nuts	ASTM A 563, including appendix X1[b]
Washers	ASTM F 844
Hardened Washers	ASTM F 436, Type 1, including S1 supplementary requirements
Components of HS steel fastener assemblies for use in structural steel joints:	
Bolts	ASTM A 325, Type 1
Tension control bolts	ASTM F 1852, Type 1
Nuts	ASTM A 563, including appendix X1[b]
Hardened washers	ASTM F 436, Type 1, Circular, including S1 supplementary requirements
Direct tension indicators	ASTM F 959, Type 325, zinc-coated

[a]Use hardened washers.
[b]Zinc-coated nuts tightened beyond snug or wrench tight must be furnished with a dry lubricant complying with supplementary requirement S2 in ASTM A 563.

Replace the 3rd table in the 1st paragraph of section 55-1.02A(1) with:

07-19-13

Other Materials

Material	Specification
Carbon steel for forgings, pins, and rollers	ASTM A 668/A 668M, Class D
Alloy steel for forgings	ASTM A 668/A 668M, Class G
Pin nuts	ASTM A 709/A 709M or ASTM A 563, including appendix X1[a]
Carbon-steel castings	ASTM A 27/A 27M, Grade 65-35, Class 1
Malleable iron castings	ASTM A 47/A 47M, Grade 32510
Gray iron castings	ASTM A 48, Class 30B
Carbon steel structural tubing	ASTM A 500/A 500M, Grade B, ASTM A 501, ASTM A 847/A 847M, or ASTM A 1085
Steel pipe[b]	ASTM A 53, Type E or S, Grade B; ASTM A 106, Grade B; or ASTM A 139, Grade B
Stud connectors	ASTM A 108

[a]Zinc-coated nuts tightened beyond snug or wrench tight must be furnished with a dry lubricant complying with supplementary requirement S2 in ASTM A 563.
[b]Hydrostatic testing will not apply.

Replace the table in the 1st paragraph in section 55-1.02A(2) with:

07-19-13

Material complying with ASTM A 709/A 709M	CVN impact value (ft-lb at temperature)
Grade 36	15 at 40 ºF
Grade 50[a] (Thickness up to 2 inches)	15 at 40 ºF
Grade 50W[a] (Thickness up to 2 inches)	15 at 40 ºF
Grade 50[a] (Thickness over 2 inches up to 4 inches)	20 at 40 ºF
Grade 50W[a] (Thickness over 2 inches up to 4 inches)	20 at 40 ºF
Grade HPS 50W[a] (Thickness up to 4 inches)	20 at 10 ºF
Grade HPS 70W (Thickness up to 4 inches)	25 at -10 ºF
Grade 100 (Thickness of 2-1/2 inches or less)	25 at 0 ºF
Grade 100W (Thickness over 2-1/2 inches up to 4 inches)	35 at 0 ºF
Grade HPS 100W (Thickness of 2-1/2 inches or less)	25 at -30 ºF
Grade HPS 100W (Thickness over 2-1/2 inches up to 4 inches)	35 at -30 ºF

[a]If the material yield strength is more than 65,000 psi, reduce the temperature for the CVN impact value 15 degrees F for each increment of 10,000 psi above 65,000 psi.

Replace the 1st sentence of the 1st paragraph in section 55-1.02A(5) with:

07-19-13

Steel, gray iron, and malleable iron castings must have continuous fillets cast in place in reentrant angles.

07-19-13

Delete the 3rd and 4th sentences in the 2nd paragraph in section 55-1.02A(5).

Replace the 1st paragraph of section 55-1.02B(1) with:

07-19-13

Section 55-1.02B(1) applies to work performed at the source and at the job site.

Replace the 4th paragraph in section 55-1.02B(1) with:

07-19-13

Ends of girder stiffeners shown as tight-fit must bear on the girder flange with at least point bearing. Local clearances between the end of the stiffener and the girder flange must be at most 1/16 inch.

Replace the 1st sentence of the 5th paragraph in section 55-1.02B(1) with:

07-19-13

Fabricate floor beams, stringers, and girders having end connection angles to exact length back to back of connection angles.

Add to the 7th paragraph in section 55-1.02B(1):

07-19-13

Use low-stress stamps for fracture critical members and tension members.

Replace the 2nd sentence of the 9th paragraph in section 55-1.02B(1) with:

07-19-13

Slightly round edges and sharp corners, including edges marred, cut, or roughened during handling or erection.

Replace the 3rd paragraph in section 55-1.02B(2) with:

07-19-13

Instead of machining, you may heat straighten steel not in contact with other metal bearing surfaces if the above tolerances are met.

Replace item 2 in the list in the 1st paragraph of section 55-1.02B(3) with:

07-19-13

2. Radius of bend measured to the concave face must comply with *Manual of Steel Construction* of the AISC

Replace the 1st sentence of the 2nd paragraph in section 55-1.02B(3) with:

07-19-13

Plates to be bent to a smaller radius than specified in *Manual of Steel Construction* of the AISC must be bent hot.

Replace the introductory clause of the 2nd paragraph of section 55-1.02B(4) with:

07-19-13

Threads for pin ends and pin nuts 1-1/2 inches or more in diameter must comply with the following:

Replace the 3rd paragraph in section 55-1.02B(5) with:

07-19-13

Holes for pins must be:

1. True to the diameter specified.
2. At right angles to the member axis.
3. Parallel with each other except for pins where nonparallel holes are required.
4. Smooth and straight with the final surface produced by a finishing cut.

Replace the 1st paragraph in section 55-1.02B(6)(c) with:

07-19-13

Bolted connections using HS fastener assemblies must comply with *Specification for Structural Joints Using High-Strength Bolts* of the RCSC.

Replace the 7th paragraph in section 55-1.02B(6)(c) with:

07-19-13

For all bolts, thread stickout after tensioning must be at least flush with the outer nut face. At least 3 full threads must be located within the grip of the connection.

07-19-13

Delete the 3rd paragraph in section 55-1.02B(7)(a).

Add to section 55-1.02B(7)(a):

07-19-13

For welds indicated to be subject to tensile forces that are to receive RT, grind smooth and flush on both sides of welds before testing.

For groove weld surface profiles that interfere with NDT procedures, grind welds smooth and blend with the adjacent material.

For fillet weld surface profiles that interfere with NDT procedures, grind welds and blend the toes smoothly with the adjacent base metal.

Add to section 55-1.02B(7):

07-19-13

55-1.02B(7)(c) Steel Pedestrian Bridges

Reserved

Replace the 1st paragraph in section 55-1.02B(9) with:

07-19-13

Prepare and paint contact surfaces of HS bolted connections before assembly. Thoroughly clean all other surfaces of metal in contact to bare metal before assembly. Remove all rust, mill scale, and foreign material.

Replace the 1st sentence of the 4th paragraph in section 55-1.02B(9) with:

07-19-13

Preassemble truss work in lengths of at least 3 abutting panels and adjust members for line and camber.

Replace the 1st sentence of the 5th paragraph in section 55-1.02B(9) with:

07-19-13

Preassemble bolted splice joints for plate girders in lengths of at least 3 abutting sections and adjust abutting sections for line and camber.

Replace the 6th paragraph in section 55-1.02B(9) with:

07-19-13

Preassemble prepared splice joints for welded girders with abutting members and adjust for line and camber.

Replace the paragraphs in section 55-1.03C(1) with:

07-19-13

Reserved

Replace the 3rd sentence of the 1st paragraph in section 55-1.03C(2) with:

07-19-13

Attain full bearing on the concrete under bearing assemblies.

Replace the 3rd paragraph in section 55-1.03C(2) with:

07-19-13

During welding, protect bearings and bearing surfaces using authorized methods.

Replace section 55-1.03C(4) with:

07-19-13

55-1.03C(4) Continuous Members

Unless otherwise shown, structural steel girders are designed for continuity in supporting girder dead load. If erection procedures provide girder continuity for dead load, preassemble members with field joints in a no-load condition in a horizontal or an upright condition.

You may erect structural steel girders such that dead load girder continuity is not provided. If erection procedures do not provide girder continuity for dead load:

1. You may increase cross-sectional areas or change grades of steel to provide the specified capacity if authorized.
2. After erection, the erected structure must have a load-carrying capacity at least equal to the structure shown.

^^

56 SIGNS

07-19-13

Replace the 4th paragraph of section 56-3.01A with:

07-19-13

The types of sign structures include:

1. Truss

2. Bridge mounted
3. Tubular

Replace "sets" in the 1st paragraph of section 56-3.01C(2) with:

04-19-13

copies

07-20-12

Delete the 7th paragraph of section 56-3.02K(2).

Replace the 1st paragraph of section 56-3.02M(1) with:

07-19-13

Galvanize all ferrous metal parts of the following sign structure types:

1. Truss
2. Bridge mounted
3. Tubular

Add between the 1st and 2nd paragraphs of section 56-3.02M(1):

04-19-13

Clean and paint all ferrous metal parts of tubular sign structures after galvanizing, including the areas to be covered by sign panels. Do not paint sign structures other than tubular type unless specified in the special provisions.

Replace the headings and paragraphs in section 56-3.02M(3) with:

04-19-13

Where specified, clean and paint sign structures under section 59-5.

^^

57 WOOD AND PLASTIC LUMBER STRUCTURES

04-19-13

Replace "51-2.01C(3)" in the 1st paragraph of section 57-2.01C(3)(a) with:

10-19-12

57-2.01C(3)

Replace "sets" at each occurrence in the 1st paragraph of section 57-3.01C with:

04-19-13

copies

^^

58 SOUND WALLS

04-19-13

10-19-12

Delete the 3rd paragraph of section 58-1.01.

Replace the 1st paragraph of section 58-2.01D(5)(a) with:

08-05-11

You must employ a special inspector and an authorized laboratory to perform Level 1 inspections and structural tests of masonry to verify the masonry construction complies with section 1704, "Special Inspections," and section 2105, "Quality Assurance," of the 2007 CBC.

10-19-12

Delete the 1st paragraph of section 58-2.02F.

Replace "sets" at each occurrence in the 1st paragraph of section 58-4.01C with:

04-19-13

copies

^^

59 PAINTING

11-15-13

Replace "SSPC-SP 10" at each occurrence in section 59 with:

10-19-12

SSPC-SP 10/NACE no. 2

Replace "SSPC-SP 6" at each occurrence in section 59 with:

10-19-12

SSPC-SP 6/NACE no. 3

Replace "SSPC-CS 23.00" at each occurrence in section 59 with:

10-19-12

SSPC-CS 23.00/AWS C 2.23M/NACE no. 12

Replace "*Specification for Structural Joints Using ASTM A325 or A 490 Bolts*" in the 1st paragraph of section 59-2.01C(1) with:

07-19-13

Specification for Structural Joints Using High-Strength Bolts

Replace "SSPC-QP 3 or AISC SPE, Certification P-1 Enclosed" in item 3 in the list in the 1st paragraph of section 59-2.01D(1) with:

10-19-12

AISC-420-10/SSPC-QP 3 (Enclosed Shop)

Replace "*Specification for Structural Joints Using ASTM A325 or A 490 Bolts*" in the 1st paragraph of section 59-2.02 with:

07-19-13

Specification for Structural Joints Using High-Strength Bolts

Replace the paragraphs in section 59-2.03A with:

10-19-12

Clean and paint all exposed structural steel and other metal surfaces.

You must provide enclosures for cleaning and painting structural steel. Cleaning and painting of new structural steel must be performed in an Enclosed Shop as defined in AISC-420-10/SSPC-QP 3. Maintain atmospheric conditions inside enclosures within specified limits.

Except for blast cleaning within closed buildings, perform blast cleaning and painting during daylight hours.

Add to section 59-2.03B:

07-19-13

59-2.03B(3) Containment Systems

59-2.03B(3)(a) General

Construct containment systems when disturbing existing paint systems during bridge rehabilitation.

The containment system must be one of the following:

1. Ventilated containment system
2. Vacuum-shrouded surface preparation equipment and drapes and ground covers
3. Equivalent containment system if authorized

The containment system must contain all water, resulting debris, and visible dust produced when the existing paint system is disturbed.

Properly maintain the containment system while work is in progress and do not change the containment system unless authorized.

Containment systems over railroad property must provide the minimum clearances as specified in section 5-1.20C for the passage of railroad traffic.

59-2.03B(3)(b) Ventilated Containment Systems

59-2.03B(3)(b)(i) General

If flexible framing is used, support and fasten it to (1) prevent the escape of abrasive and blast materials due to whipping from traffic or wind and (2) maintain clearances.

If the wind speed reaches 50 mph or greater, relieve the wind pressure on the containment system using an authorized method.

59-2.03B(3)(b)(ii) Design Criteria

Scaffolding or supports for the ventilated containment system must not extend below the vertical clearance level nor to the ground line at locations within the roadbed.

For truss-type bridges, all connections of the ventilated containment system to the existing structure must be made through the deck, girder, stringer, or floor beam system. No connections are allowed that will cause bending stresses in a truss member.

The ventilated containment system must comply with section 7-1.02K(6)(e).

The minimum total design load for the ventilated containment system must consist of the sum of the dead and live vertical loads.

Dead and live loads are as follows:

1. Dead load must consist of the actual load of the ventilated containment system
2. Live loads for bridges with only spot blast cleaning work must consist of:
   2.1. Uniform load of at least 25 psf applied over the supported area
   2.2. Moving concentrated load of 1000 lb to produce maximum stress in the main supporting elements of the ventilated containment system
3. Live loads for bridges with 100 percent blast cleaning to bare metal must consist of:
   3.1. Uniform load of at least 45 psf, which includes 20 psf of sand load, applied over the supported area
   3.2. Moving concentrated load of 1000 lb to produce maximum stress in the main supporting elements of the ventilated containment system

Assumed horizontal loads do not need to be included in the design of the ventilated containment system.

Maximum allowable stresses must comply with section 48-2.01D(3)(c).

59-2.03B(3)(b)(iii) Ventilation

The ventilation system in the ventilated containment system must be of the forced input airflow type with fans or blowers.

Negative air pressure must be employed within the ventilated containment system and will be verified by visual methods by observing the concave nature of the ventilated containment system while taking into account wind effects or by using smoke or other visible means to observe airflow. The input airflow must be properly balanced with the exhaust capacity throughout the range of operations.

The exhaust airflow of the ventilation system in the ventilated containment system must be forced into wet or dry dust collectors or bag houses.

Replace item 1 in the list in the 2nd paragraph of section 59-2.03C(1) with:

10-19-12

1. Apply a stripe coat of undercoat paint on all edges, corners, seams, crevices, interior angles, junctions of joining members, weld lines, and similar surface irregularities. The stripe coat must completely hide the surface being covered. If spot blast cleaning portions of the bridge, apply the stripe coat of undercoat paint before each undercoat and follow with the undercoat as soon as practical. If removing all existing paint from the bridge, apply the undercoat first as soon as practical and follow with the stripe coat of undercoat paint for each undercoat.

Replace the heading of section 59-2.03C(2) with:

04-19-13

Zinc Coating System

Add to section 59-2.03C(2)(a):

04-19-13

Coatings for new structural steel and connections between new and existing structural steel must comply with the requirements shown in the following table:

Zinc Coating System

Description	Coating	Dry film thickness (mils)
All new surfaces:		
Undercoat	Inorganic zinc primer, AASHTO M 300 Type I or II	4–8
Finish coat[a]	Exterior grade latex[b], 2 coats	2 minimum each coat, 4–8 total
Total thickness, all coats		8–14
Connections to existing structural steel:[c]		
Undercoat	Inorganic zinc primer, AASHTO M 300 Type I or II	4–8
Finish coat[a]	Exterior grade latex[b], 2 coats	2 minimum each coat, 4–8 total
Total thickness, all coats		8–14

[a]If no finish coats are described, a final coat of inorganic zinc primer is required.
[b]Exterior grade latex must comply with section 91-2.02 unless otherwise specified.
[c]Includes the following locations:

1. New and existing contact surfaces
2. Existing member surfaces under new HS bolt heads, nuts, or washers
3. Bare surfaces of existing steel after trimming, cutting, drilling, or reaming
4. Areas within a 4-inch radius from the point of application of heat for welding or flame cutting

Replace "*Specification for Structural Joints Using ASTM A325 or A 490 Bolts*" in the 7th paragraph of section 59-2.03C(2)(b)(i) with:

07-19-13

Specification for Structural Joints Using High-Strength Bolts

Add to section 59-2.03C:

04-19-13

59-2.03C(3) Moisture-Cured Polyurethane Coating System

Reserved

59-2.03C(4) State Specification Paint Waterborne Coating System

59-2.03C(4)(a) General

The State Specification PWB coating system for existing structural steel must comply with the requirements shown in the following table:

State Specification PWB Coating System

Surface	Description	State Specification PWB Coating	Dry film thickness (mils)
Surfaces cleaned to bare metal[a]:	1st undercoat	145	2–3
	2nd undercoat	146	2–3
	1st finish coat	171	1.5–3
	2nd finish coat	172	1.5–3
	Total thickness, all coats	--	7–12
Existing painted surfaces to be topcoated:	Undercoat	146	2–3
	1st finish coat	171	1.5–3
	2nd finish coat	172	1.5–3
	Total thickness, new coats	--	5–9

[a]Includes locations of spot blast cleaning

59-2.03C(4)(b) Finish Coats

11-15-13

Reserved

Add to section 59-5.01:

04-19-13

Where specified, prepare and paint sign structures under sections 59-2 and 59-3.

Instead of submitting proof of the certification complying with SSPC-QP 1, you may submit documentation with the painting quality work plan showing compliance with the requirements in section 3 of SSPC-QP 1.

Instead of submitting proof of the certification complying with SSPC-QP 2, you may submit documentation with the painting quality work plan showing compliance with the requirements in sections 4.2 through 4.4 of SSPC-QP 2, Category A.

Instead of submitting proof of the certification complying with AISC-420-10/SSPC-QP 3 (Enclosed Shop), you may submit documentation with the painting quality work plan showing compliance with the requirements in sections 5 through 18 of AISC-420-10/SSPC-QP3.

Replace the paragraphs of section 59-5.03 with:

04-19-13

59-5.03A General

You may prepare and paint sign structures before or after erection. After erection, repair damaged paint to the satisfaction of the Engineer.

The total dry film thickness of finish coats on contact surfaces of galvanized HS bolted connections (1) must be from 1 to 4 mils and (2) may be applied in 1 application.

59-5.03B Undercoating of Ungalvanized Surfaces

Blast-cleaned surfaces must receive a single undercoat consisting of an inorganic zinc coating as specified in AASHTO M 300, Type I or Type II, except:

1. The first 2 sentences of section 5.6 do not apply
2. Section 5.6.1 does not apply

If you propose to use a coating that is not on the Authorized Material List, submit the required documentation specified in section 5.6 of AASHTO M 300. Allow 30 days for the Engineer's review.

59-5.03C Testing of Inorganic Zinc Coating

Perform adhesion and hardness testing no sooner than 72 hours after application of the single undercoat of inorganic zinc coating.

59-5.03D Finish Coating

The exposed area of inorganic zinc coating must receive a minimum of 2 finish coats of exterior grade latex paint.

The 1st finish coat color must match no. 24558 of FED-STD-595. The 2nd finish coat color must match no. 24491 of FED-STD-595. The total dry film thickness of the applications of the 2nd finish coat must be not less than 2 mils.

Replace section 59-7 with:

07-19-13

59-7 STAINING CONCRETE AND SHOTCRETE

59-7.01 GENERAL

59-7.01A General

59-7.01A(1) Summary

Section 59-7.01 includes specifications for preparing and staining concrete and shotcrete surfaces using an acid stain.

59-7.01A(2) Definitions

Reserved

59-7.01A(3) Submittals

Submit stain manufacturer's product data and application instructions at least 7 days before starting staining activities.

59-7.01A(4) Quality Control and Assurance

Reserved

59-7.01B Materials

59-7.01B(1) General

Reserved

59-7.01B(2) Stain

Stain must:

1. Be a water-based solution of inorganic metallic salts
2. Contain dilute acid that penetrates and etches the concrete or shotcrete surface
3. Be a commercial quality product designed specifically for exterior applications
4. Produce abrasion-resistant color deposits

59-7.01B(3) Sealer

Reserved

59-7.01B(4) Joint Sealing Compound

Reserved

59-7.01C Construction

59-7.01C(1) General

Seal joints between concrete and shotcrete surfaces to be stained and adjacent metal with joint sealing compound before applying the stain.

Test surfaces for acceptance of the stain before applying the stain. Clean surfaces that resist accepting the stain and retest until passing.

Apply the stain under the manufacturer's instructions.

Before staining, the concrete or shotcrete surfaces must be:

1. At least 28 days old
2. Prepared under SSPC-SP 13/NACE no. 6
3. Thoroughly dry

Apply the stain uniformly to avoid excessive rundown. Work the stain into the concrete using a nylon bristle brush in a circular motion.

After the last coat of stain has dried, rinse stained surfaces with water and wet scrub with a stiff bristle nylon brush until the rinse water runs clear. Collect all rinse water.

Protect adjacent surfaces during staining.

Thoroughly cure each application of the stain and correct skips, holidays, thin areas, or other deficiencies before the next application.

Drips, puddles, or other irregularities must be worked into the concrete or shotcrete surface.

59-7.01C(2) Test Panel

For staining concrete or shotcrete, stain a test panel complying with section 51-1.01D(3).

For staining sculpted shotcrete, stain a test panel complying with section 53-3.01D(3).

The test panel must be:

1. Stained using the same personnel, materials, equipment and methods to be used in the work
2. Accessible for viewing
3. Displayed in an upright position near the work
4. Authorized for staining before starting the staining work

If ordered, construct additional test panels until a satisfactory color is attained.

The Engineer uses the authorized stained test panel to determine the acceptability of the stained surface.

Dispose of the test panels after the staining work is complete and authorized. Notify the Engineer before disposing of the test panels.

59-7.01D Payment

Not Used

59-7.02 SCULPTED SHOTCRETE AND TEXTURED CONCRETE

59-7.02A General

59-7.02A(1) Summary

Section 59-7.02 includes specifications for preparing and staining sculpted shotcrete and textured concrete surfaces using an acid stain.

59-7.02A(2) Definitions

Reserved

59-7.02A(3) Submittals

59-7.02A(3)(a) General

Reserved

59-7.02A(3)(b) Experience Qualifications

Submit the following documentation of the staining subcontractor's experience at least 10 days before the preconstruction meeting:

1. Summary of the staining subcontractor's experience that demonstrates compliance with section 59-7.02A(4)(b).
2. List of at least 3 projects completed in the last 5 years that demonstrate the staining subcontractor's ability to stain textured concrete or sculpted shotcrete surfaces similar to the textured concrete or sculpted shotcrete for this project. For each project include:
   2.1. Project description

2.2. Name and phone number of the owner
2.3. Staining completion date
2.4. Color photos of the completed stained surface

59-7.02A(3)(c) Installation Plan

Submit an installation plan at least 10 days before the preconstruction meeting. The installation plan must include details for preparing and staining the textured concrete or sculpted shotcrete to achieve the required color, including:

1. Number of applications that will be used to apply the stain
2. For each application of the stain, a description of:
   2.1. Manufacturer, color, finish, and percentage strength mixture of the stain that will be applied
   2.2. Methods and tools that will be used to apply the stain
3. Methods for protecting adjacent surfaces during staining
4. Rinse water collection plan for containing all liquid, effluent, and residue resulting from preparing and staining textured concrete or sculpted shotcrete

59-7.02A(4) Quality Control and Assurance

59-7.02A(4)(a) General

Reserved

59-7.02A(4)(b) Contractor Qualifications

The staining subcontractor must:

1. Have experience in staining textured concrete or sculpted shotcrete surfaces to simulate the appearance of natural rock formations or stone masonry
2. Have successfully completed at least 3 projects in the past 5 years involving staining of concrete or sculpted shotcrete surfaces similar to the textured concrete or sculpted shotcrete for this project

59-7.02A(4)(c) Preconstruction Meeting

Before starting staining activities, conduct a meeting to discuss the installation plan. Meeting attendees must include the Engineer and all staining subcontractors.

59-7.02B Materials

Not Used

59-7.02C Construction

Not Used

59-7.02D Payment

Prepare and stain concrete and prepare and stain shotcrete are measured by the area of the vertical or sloped wall face stained.

Replace "solider" in the 5th paragraph of section 59-9.03 with:

04-19-13

soldier

Replace section 59-11 with:

07-19-13

59-11 STAINING GALVANIZED SURFACES

Reserved

Replace section 59-12 with:

07-19-13

59-12 ROCK STAINING

59-12.01 GENERAL

59-12.01A Summary

Section 59-12 includes specifications for applying stain to the exterior surface of landscape boulders, native rock that has been damaged or scarred, rock energy dissipaters, rock slope protection and gabion surfaces.

59-12.01B Submittals

Submit the following:

1. Work plan showing methods to control overspray and spillage, and to protect adjacent surfaces
2. Product data including the manufacturer's product sheet and the instructions for the application of the stain

59-12.01C Quality Control and Assurance

59-12.01C(1) General

Reserved

59-12.01C(2) Test Plot

Apply the stain to a test plot rock area of at least 3 by 3 feet at a location designated by the Engineer. Notify the Engineer at least 7 days before staining the test plot. Prepare and stain the test plot with the same materials, tools, equipment, and methods to be used in staining the final surfaces. Separate test plots are required for staining rock slope protection and native rock.

If ordered, prepare additional test plots. Additional test plots are change order work.

Obtain authorization of the test plot before starting the staining work. Use the authorized test plot as the standard for comparison in determining acceptability of staining. If the test plot is not incorporated into the work and the Engineer determines it is no longer needed, dispose of it.

59-12.02 MATERIALS

59-12.02A General

Reserved

59-12.02B Stain

Reserved

59-12.03 CONSTRUCTION

59-12.03A General

Reserved

59-12.03B Preparation

Before applying the stain:

1. Identify and obtain authorization for the areas to be stained
2. Remove oils, dirt, and other contaminants from the surfaces to be stained
3. Dry all surfaces to be stained

59-12.03C Application

After the areas to be stained have been identified, prepared, and the test plot authorized, stain the exposed surfaces under the manufacturer's instructions to achieve a color consistent with, or as close as possible to, the authorized test area color.

Control overspray and protect adjacent surfaces.

Keep stained surfaces dry for at least 20 days following the application of the stain.

59-12.04 PAYMENT

Rock stain areas are measured along the slope face.

^^^

Replace section 65-2.02D with:

07-19-13

65-2.02D Reserved

^^

68 SUBSURFACE DRAINS

04-18-14

Replace the 5th paragraph of section 68-4.02E with:

04-18-14

HMA for backfilling trenches must comply with HMA for miscellaneous areas as specified in section 39.

^^

70 MISCELLANEOUS DRAINAGE FACILITIES

07-19-13

Replace section 70-5.02A(2) with:

01-20-12

70-5.02A(2) Plastic Flared End Sections

Plastic flared end sections must comply with ASTM D 3350.

Replace "40-1.03N" in item 2.4 of the 1st paragraph of section 70-5.06C with:

07-19-13

40-1.03K

Replace the 2nd, 3rd, and 4th paragraphs of section 70-7.02B with:

01-18-13

Before shipping, the exterior surfaces of the casing must be cleaned, primed, and coated to comply with ANSI/AWWA C213 or ANSI/AWWA C214.

Wrapping tape for repairing damaged coating and wrapping field joints and fittings must be a pressure-sensitive PVC or polyethylene tape with a minimum thickness of 50 mils, 2 inches wide.

Add to section 70-7.03:

01-18-13

Repair damaged coating on the casing and wrap field joints and fittings with wrapping tape as follows:

1. Before wrapping, thoroughly clean and prime the pipe casing, joints, and fittings under the tape manufacturer's instructions.
2. Wrap the tape tightly with 1/2 uniform lap, free from wrinkles and voids to provide not less than a 100-mil thickness.
3. Wrapping at joints must extend at least 6 inches over adjacent pipe casing coverings. Apply tension such that the tape will conform closely to contours of the joint.

Add to section 70:

07-19-13

70-8–70-15 RESERVED

^^^

Add to section 72-11.01B:

01-18-13

Expanded polystyrene and premolded expansion joint filler must comply with section 51-2.

07-19-13

Delete the 2nd paragraph of section 72-11.01C(1).

07-19-13

Delete the 7th paragraph of section 72-11.01C(1).

Add between the 7th and 8th paragraphs of section 72-11.01C(1):

07-19-13

Schedule the construction of the slope paving such that the work, including placing and finishing concrete and applying curing compound, is completed on the same day that the work is started.

Replace the 8th paragraph of section 72-11.01C(1) with:

07-19-13

If the Engineer determines that the size of the slope paving is too large to be constructed without an intermediate construction joint, place a joint at an authorized location. Complete a section of concrete bounded by permissible construction joints within the same day.

Replace the 1st paragraph of section 72-11.01C(2) with:

01-18-13

Construct and finish minor concrete slope paving under section 51-1.

Replace the 3rd paragraph of section 72-11.01C(2) with:

07-19-13

After striking-off to grade, hand float the concrete with floats that are at least 4 inches wide and 30 inches long. Broom the entire surface with a stiff-bristled broom to produce a uniform surface. Brooming must be done when the surface is sufficiently set to prevent deep scarring and must be accomplished by drawing the broom down the slope, leaving marks parallel to the slope. The Engineer may order you to apply a fine spray of water to the surface immediately before brooming.

07-19-13

Delete the 3rd paragraph of section 72-11.01D.

^^

73 CONCRETE CURBS AND SIDEWALKS

07-19-13

Replace the paragraph in section 73-1.01A with:

07-19-13

Section 73-1 includes general specifications for constructing minor concrete items including concrete curbs, sidewalks, gutter depressions, driveways, island paving, and curb ramps; for installing detectable warning surfaces and precast parking bumpers; and for texturing and coloring concrete surfaces.

^^

74 PUMPING EQUIPMENT AND CONTROLS

04-19-13

Replace the 1st paragraph of section 74-1.01C(3) with:

04-19-13

Submit at least 5 copies of product data to OSD, Documents Unit. Each copy must be bound together and include an index stating equipment names, manufacturers, and model numbers. Two copies will be returned. Notify the Engineer of the submittal. Include in the notification the date and contents of the submittal.

Replace the 1st sentence of the 1st paragraph in section 74-2.01D(2) with:

01-20-12

Drainage pumps must be factory certified under ANSI/HI 14.6.

^^

75 MISCELLANEOUS METAL

07-19-13

Add between 2nd and 3rd paragraphs of section 75-1.03A:

07-19-13

Fabricate expansion joint armor from steel plates, angles, or other structural shapes. Shape the armor to the section of the concrete deck and match-mark it in the shop. Straighten warped sections of expansion joint armor before placing. Secure the expansion joint armor in the correct position during concrete placement.

Replace "SSPC-QP 3" in the 3rd paragraph of section 75-1.03E(4) with:

10-19-12

AISC-420-10/SSPC-QP3

Replace "metal beam guard railing" in the table in the 1st paragraph of section 75-1.05 with:

07-19-13

guardrail

^^

Replace section 78 with:

07-20-12

78 INCIDENTAL CONSTRUCTION

07-20-12

78-1 GENERAL

Section 78 includes specifications for incidental bid items that are not closely associated with other sections.

78-2–78-50 RESERVED

^^

80 FENCES

10-19-12

Add to section 80-2.02D:

10-19-12

Vertical stays must:

1. Comply with ASTM A641
2. Be 12-1/2 gage
3. Have a Class 3 zinc coating

Replace item 1 in the list in section 80-2.02E with:

10-19-12

Comply with ASTM A 116, Type Z, Grade 60, Class 1

Add after "galvanized wire" in the 1st paragraph of section 80-2.02F:

10-19-12

complying with ASTM A 641

Replace the 3rd and 4th paragraphs of section 80-2.02F with:

10-19-12

Each staple used to fasten barbed wire and wire mesh fabric to wood posts must:

1. Comply with ASTM F 1667
2. Be at least 1-3/4 inches long
3. Be manufactured from 9-gage galvanized wire

Wire ties used to fasten barbed wire and wire mesh to metal posts must be at least 11-gage galvanized wire complying with ASTM F 626. Clips and hog rings used for metal posts must be at least 9-gage galvanized wire complying with ASTM F 626.

Replace the 8th through 14th paragraphs of section 80-2.03 with:

10-19-12

Attach the wire mesh and barbed wire to each post.

Securely fasten tension wires to wood posts. Make a single or double loop around each post at each attachment point and staple the wire to the post. Use wire ties, hog rings, or wire clips to fasten the wires to the metal posts.

Connect each wood brace to its adjacent post with a 3/8 by 4-inch steel dowel. Twist the tension wires until the installation is rigid.

Stretch barbed wire and wire mesh fabric and fasten to each wood or steel end, corner, or gate post. Apply tension according to the manufacturer's instructions using a mechanical stretcher or other device designed for such use. If no tension is specified by the manufacturer, use 250 pounds for the required tension. Evenly distribute the pull over the longitudinal wires in the wire mesh such that no more than 50 percent of the original depth of the tension curves is removed. Do not use a motorized vehicle, truck, or tractor to stretch the wire.

Attach barbed wire and wire mesh fabric to the private-property side of posts. On curved alignments, place the wire mesh and barbed wire on the face of the post against which the normal pull of the wire

mesh and wire will be exerted. Terminate the wire mesh and barbed wire at each end, corner, pull, and gate post in the new fence line. Attach wire mesh and barbed wire to each wood or steel end, corner, pull, or gate post by wrapping each horizontal strand around the post and tying it back on itself with at least 4 tightly-wound wraps.

At line posts, fasten the wire mesh to the post at the top and bottom and at intermediate points not exceeding 10 inches apart. Fasten each line of barbed wire to each line post. Use wire ties or clips to fasten the wires to metal posts under the post manufacturer's instructions. Drive staples crosswise with the grain of the wood and pointed slightly downward. Drive staples just short of actual contact with the wires to allow free longitudinal movement of those wires and to prevent damage to the wire's protective coating. Secure all wires to posts to maintain horizontal alignment.

Splices in barbed wire and wire mesh are allowed provided there are no more than 2 splices per 50 feet of fence. Use commercially-available galvanized mechanical wire splices or a wire splice created by tying off wire. Install mechanical wire splices with a tool designed for that purpose under the manufacturer's instructions. Tie off the wire as follows:

1. Carry the ends of each wire 3 inches past the tied-off knot location and wrap around the wire for at least 6 turns in opposite directions.
2. Remove the splice tool and close the space by pulling the end of the wires together.
3. Cut the unused ends of the wire close and neat.

Add to "≤ 6" in the table in the 4th paragraph of section 80-3.02B:

10-19-12

feet

^^^

Replace "Metal rail posts, box spacers, and" in item 1 in the list in the 25th paragraph of section 83-1.02B with:

07-19-13

Metal box spacers and

07-19-13

Delete items 6 and 7 in the list in the 25th paragraph of section 83-1.02B.

Replace "Type WB" at each occurrence in section 83-1.02B(2) with:

07-19-13

Type WB-31

Replace the heading of section 83-1.02B(3) with:

07-19-13

Temporary Midwest Guardrail System

Replace "80-2.02" in the 2nd paragraph of section 83-1.02E with:

10-19-12

80-3.02B

Replace "sets" in the 10th paragraph of section 83-1.02G(2) with:

07-19-13

copies

Replace the 1st sentence of the 1st paragraph of section 83-1.03 with:

11-15-13

Except for guardrail within the pay limits of a terminal system, a transition railing (Type WB-31), an end anchor assembly, or a rail tensioning assembly, midwest guardrail system is measured along the face of the rail element from end post to end post of the completed railing.

Add to section 83-2.02D(1):

10-21-11

For a concrete barrier transition:

1. Remove portions of the existing concrete barrier where shown under section 15-3
2. Roughen the contact surface of the existing concrete barrier
3. Drill and bond dowels into the existing concrete barrier under section 51-1

Add to section 83-2.02:

10-19-12

83-2.02H–83-2.02M Reserved

^^

84 TRAFFIC STRIPES AND PAVEMENT MARKINGS

05-30-14

Replace section 84-1.01C with:

05-30-14

84-1.01C Submittals

For glass beads used in drop-on applications and in thermoplastic formulations, submit a certificate of compliance and test results for each lot of beads specifying the EPA test methods used and tracing the lot to the specific test sample. The testing for lead and arsenic content must be performed by an independent testing laboratory.

Submit retroreflectivity readings for traffic stripes and pavement markings at locations with deficient retroreflectivity determined by the Engineer.

84-1.01D Quality Control and Assurance

Test each lot of glass beads for arsenic and lead under EPA Test Method 3052 and 6010B or 6010C.

Applied traffic stripes and pavement markings must be retroreflective. Within 30 days of applying traffic stripes and pavement markings, the retroreflectivity of the stripes and markings must be a minimum of 250 $mcd \cdot m^{-2} \cdot lx^{-1}$ for white and 125 $mcd \cdot m^{-2} \cdot lx^{-1}$ for yellow when measured under ASTM E1710.

The Engineer will perform a nighttime, drive-through, visual inspection of the retroreflectivity of the traffic stripes and pavement markings and notify you of any locations with deficient retroreflectivity. Measure the retroreflectivity of the deficient areas using a retroreflectometer under ASTM E1710 and the sampling protocol specified in ASTM D7585.

Replace the paragraph in section 84-1.02 with:

05-30-14

Glass beads applied to paint must comply with State Specification 8010-004.

Glass beads applied to molten thermoplastic material must be Type 2 beads complying with AASHTO M 247. The glass beads must have a coating that promotes adhesion of the beads to thermoplastic.

At least 75 percent of the beads by count must be true spheres that are colorless and do not exhibit dark spots, air inclusions, or surface scratches when viewed under 20X magnification.

Each lot of glass beads used in pavement markings must contain less than 200 ppm each of arsenic and lead when tested under EPA Test Method 3052 and 6010B or 6010C.

Replace the 1st paragraph in section 84-2.04 with:

01-20-12

A double extruded thermoplastic traffic stripe consisting of two 4-inch wide yellow stripes is measured as 2 traffic stripes.

A double sprayable thermoplastic traffic stripe consisting of two 4-inch wide yellow stripes is measured as 1 traffic stripe.

Add to section 84:

01-20-12

84-6 THERMOPLASTIC TRAFFIC STRIPES AND PAVEMENT MARKINGS WITH ENHANCED WET NIGHT VISIBILITY

Reserved

84-7–84-10 RESERVED

^^

86 ELECTRICAL SYSTEMS

11-15-13

Replace the paragraphs in section 86-1.01 with:

07-19-13

Section 86 includes general specifications for constructing and rehabilitating electrical systems.

Electrical systems must comply with the material and installation specifications in section 86-2.

Section 86-3 includes specifications for constructing controller assemblies.

Section 86-4 includes specifications for constructing traffic signal faces, programmed visibility signal faces, pedestrian signal faces, flashing beacons, ramp metering signs, and signal mounting assemblies.

Section 86-5 includes specifications for constructing vehicle detectors and pedestrian push button assemblies.

Section 86-6 includes specifications for constructing lighting systems.

Section 86-7 includes specifications for constructing rehabilitating electrical equipment.

Comply with Part 4 of the *California MUTCD*. Nothing in section 86 is to be construed as to reduce the minimum standards in this manual.

The locations shown for electrical systems are approximate; the Engineer determines the final locations.

Replace the paragraphs in section 86-1.015 with:

07-19-13

actuation: Actuation as defined in the *California MUTCD*.

channel: Discrete information path.

controller assembly: Assembly for controlling a system's operations, consisting of a controller unit and auxiliary equipment housed in a rainproof cabinet.

controller unit: Part of the controller assembly performing the basic timing and logic functions.

detector: Detector as defined in the *California MUTCD*.

electrolier: Assembly of a lighting standard and luminaire.

flasher: Device for opening and closing signal circuits at a repetitive rate.

flashing beacon control assembly: Assembly of switches, circuit breakers, terminal blocks, flasher, wiring, and other necessary electrical components housed in a single enclosure for operating a beacon.

inductive loop detector: Detector capable of being actuated by an inductance change caused by a vehicle passing or standing over the loop.

lighting standard: Pole and mast arm supporting the luminaire.

luminaire: Assembly that houses the light source and controls the light emitted from the light source.

magnetic detector: Detector capable of being actuated by an induced voltage caused by a vehicle passing through the earth's magnetic field.

powder coating: Coating applied electrostatically using exterior-grade UV-stable polymer powder.

pretimed controller assembly: Assembly operating traffic signals under a predetermined cycle length.

pull box: A box with a cover that is installed in an accessible place in a run of conduit to facilitate the pulling in of wires or cables.

signal face: Signal face as defined in the *California MUTCD*.

signal head: Signal head as defined in the *California MUTCD*.

signal indication: Signal indication as defined in the *California MUTCD*.

signal section: Signal section as defined in the *California MUTCD*.

signal standard: Pole and mast arm supporting 1 or more signal faces with or without a luminaire mast arm.

traffic-actuated controller assembly: Assembly for operating traffic signals under the varying demands of traffic as registered by detector actuation.

traffic phase: Signal phase as defined in the *California MUTCD*.

vehicle: Vehicle as defined in the *California Vehicle Code*.

Replace the paragraphs in section 86-1.02 with:

07-19-13

Comply with 8 CA Code of Regs § 2299 et seq.

Electrical equipment must comply with one or more of the following standards:

1. ANSI
2. ASTM
3. EIA
4. NEMA
5. NETA
6. UL
7. Public Utilities Commission, General Order No. 95, "Rules for Overhead Electrical Sign Construction"
8. Public Utilities Commission, General Order No. 128, "Rules for Construction of Underground Electric Supply and Communication Systems"

Materials and workmanship must comply with:

1. FCC rules
2. ITE standards
3. NEC
4. California Electrical Code

Electrical equipment and materials must be NRTL certified wherever applicable.

Replace the paragraphs in section 86-1.03 with:

07-19-13

Submit a schedule of values within 15 days after Contract approval.

Determine the quantities required to complete the work. Submit the quantities as part of the schedule of values.

Provide a schedule of values for each lump sum bid item.

Do not include costs for the traffic control system in the schedule of values.

The schedule of values must include the type, size, and installation method for:

1. Foundations
2. Standards and poles

3. Conduit
4. Pull boxes
5. Conductors and cables
6. Service equipment enclosures
7. Telephone demarcation cabinets
8. Vehicle signal heads and hardware
9. Pedestrian signal heads and hardware
10. Push buttons
11. Loop detectors
12. Luminaires and lighting fixtures
13. Materials shown in the quantity tables on plan sheets labeled *E*

Replace the paragraphs in section 86-1.04 with:

07-19-13

Within 15 days of Contract approval, submit a list of equipment and materials that you propose to install. Submit the list before shipping equipment or materials to the job site. The list must include the following information:

1. Manufacturer's name
2. Make and model number
3. Month and year of manufacture
4. Lot and serial numbers
5. Dimensions
6. List of components
7. Manufacturer's installation instructions
8. Contract number
9. Your contact information

Supplement the list with 2 copies of the following data:

1. Schematic wiring diagrams
2. Scale drawings of cabinets showing location and spacing of shelves, terminal blocks, and equipment, including dimensions
3. Operation manual

Electrical equipment constructed as shown does not require detailed drawings and diagrams.

Submit 3 sets of computer-generated schematic wiring diagrams for the cabinet.

Place the schematic wiring diagram in a heavy-duty plastic envelope and attach it to the inside of the cabinet door.

Prepare diagrams, plans, and drawings using graphic symbols in IEEE 315, "Graphic Symbols for Electrical and Electronic Diagrams."

Replace the 5th paragraph of section 86-2.04B(2) with:

07-19-13

HS bolts, nuts, and flat washers used to connect slip base plates must comply with the requirements for HS fastener assemblies for use in structural steel joints in section 55-1.02A(1) except rotational capacity testing and tension testing are not required.

07-19-13

Delete the row for standard Type 36-20A in the table in the 6th paragraph of section 86-2.04B(2).

Replace the 10th paragraph of section 86-2.04B(2) with:

07-19-13

Bolted connections attaching signal or luminaire arm to the pole must be considered slip critical. Galvanized faying surfaces of plates on luminaire arm, signal arm, and pole must be roughened by hand using a wire brush before assembly and must comply with requirements for Class C surface conditions for slip-critical connections in *Specification for Structural Joints Using High-Strength Bolts* of the RCSC. Coatings for faying surfaces must comply with the RCSC specification for Class B coatings.

Replace the 1st sentence of item 8 in the list in the 1st paragraph of section 86-2.04B(3) with:

07-19-13

During manufacturing, longitudinal seams on vertical tubular members of cantilevered support structures must be within 90 degrees circumferentially of the center of the longest mast arm connection.

07-19-13

Delete item 15.3 in the list in the 1st paragraph of section 86-2.04B(3).

Add between "Exposed" and "conduit" in the 2nd paragraph of section 86-2.05B:

07-19-13

Type 1

Replace the 1st sentence of the 10th paragraph of section 86-2.05C with:

07-19-13

After installing conduit, install the pull tape.

Replace the 1st sentence of the 15th paragraph of section 86-2.05C with:

11-15-13

Conduit runs shown to be located behind curbs may be installed in the street within 3 feet of and parallel to the face of the curb by the trenching in pavement method.

Replace the 1st and 2nd sentences of the 2nd paragraph of section 86-2.05D with:

07-19-13

Install an expansion-deflection fitting for expansion joints with a 1-1/2-inch movement rating. The fitting must be watertight and include a molded neoprene sleeve, a bonding jumper, and 2 silicon bronze or zinc-plated iron hubs.

Replace section 86-2.06 with:

07-19-13

86-2.06 PULL BOXES

86-2.06A General

86-2.06A(1) Cover Marking

The cover marking must be clearly defined, uniform in depth, and parallel to either the long or short sides of the cover.

Marking letters must be 1 to 3 inches high.

Before galvanizing steel or cast iron cover, apply marking by one of the following methods:

1. Use cast iron strip at least 1/4 inch thick with letters raised a minimum of 1/16 inch. Fasten strip to cover with 1/4-inch flathead stainless steel machine bolts and nuts. Peen bolts after tightening.
2. Use sheet steel strip at least 0.027 inch thick with letters raised a minimum of 1/16 inch. Fasten strip to cover by spot welding, tack welding, or brazing, with 1/4-inch stainless steel rivets or 1/4-inch roundhead stainless steel machine bolts and nuts. Peen bolts after tightening.
3. Bead weld the letters on cover such that the letters are raised a minimum of 3/32 inch.

86-2.06A(2) Installation and Use

Space pull boxes no more than 200 feet apart. You may install additional pull boxes to facilitate the work.

You may use a larger standard size pull box than that shown on the plans or specified.

A pull box in ground or sidewalk area must be installed as follows:

1. Embed bottom of the pull box in crushed rock.
2. Place a layer of roofing paper on the crushed rock.
3. Place grout over the layer of roofing paper. Grout must be 0.50 to 1 inch thick and sloped toward the drain hole.
4. Make a 1-inch drain hole in the center of the pull box through the grout and roofing paper.
5. Place grout between the pull box and the pull box extension, and around conduits.

The top of the pull box must be flush with the surrounding grade or the top of an adjacent curb, except in unpaved areas where the pull box is not immediately adjacent to and protected by a concrete foundation, pole, or other protective construction. Place the pull box 1-1/4 inches above the surrounding grade. Where practical, place a pull box shown in the vicinity of curbs or adjacent to a standard on the side of the foundation facing away from traffic. If a pull box is installed in a sidewalk area, adjust the depth of the pull box so that the top of the pull box is flush with the sidewalk.

Reconstruct the sump of an existing pull box if disturbed by your activities. Remove old grout and replace with new if the sump was grouted.

86-2.06B Non–Traffic Pull Boxes

Reserved

86-2.06C Traffic Pull Boxes

The traffic pull box and cover must comply with ASTM C857, "Standard Practice for Minimum Structural Design Loading for Underground Precast Concrete Utility Structures," for HS20 loading. You must be able to place the load anywhere on the box and cover for 1 minute without causing cracks or permanent deformations.

Frame must be anchored to the box with 1/4 by 2-1/4 inch concrete anchors. Four concrete anchors must be included for No. 3-1/2(T) pull box; one placed in each corner. Six concrete anchors must be included for No. 5(T) and No. 6(T) pull boxes; one placed in each corner and one near the middle of each of the longer sides.

Nuts must be zinc-plated carbon steel, vibration resistant, and have a wedge ramp at the root of the thread.

After installation of traffic pull box, install the steel cover and keep it bolted down when your activities are not in progress at the pull box. When the steel cover is placed for the final time, the cover and Z bar frame must be cleaned of debris and tightened securely.

Steel cover must be countersunk approximately 1/4 inch to accommodate the bolt head. When tightened, the bolt head must not exceed more than 1/8 inch above the top of the cover.

Concrete placed around and under traffic pull boxes must be minor concrete.

Replace the 11th row in the table in the 1st paragraph of section 86-2.08B with:

07-19-13

Grounded circuit conductor	Pedestrian push buttons	Wht	Blk	NBR	14
	Signals and multiple lighting	Wht	None	NBR	10
	Flashing beacons and sign lighting	Wht	None	NBR	12
	Lighting control	Wht	None	C-3	14
	Service	Wht	None	NBR	14

Replace the 1st sentence of the 1st paragraph of section 86-2.08C with:

07-19-13

Circuit conductors, connectors, and terminals must be UL or NRTL listed and rated for 600 V(ac) operation.

Add to the beginning of section 86-2.09A:

07-19-13

Provide enough traffic signal light conductors for functional operation of the signal. Provide 3 spare conductors in all conduits containing traffic signal light conductors.

Replace the paragraphs in section 86-2.09C with:

07-19-13

Connectors must be crimp type. Use a manufacturer-recommended tool for connectors and terminals to join conductors. Comply with SAE-AS7928.

Terminate stranded conductors smaller than no. 14 in crimp style terminal lugs.

Terminate field conductors no. 12 and smaller with spade type terminals. Terminate field conductors no. 10 and larger with spade type or ring type terminals.

Replace the value for resistivity in the table in the 6th paragraph of section 86-2.09E with:

07-19-13

25×10^{13} Ω per inch, minimum

Add between "the" and "head" in the 3rd sentence of the 2nd paragraph of 86-2.09F:

07-19-13

connector

Replace "project" in the 3rd paragraph of section 86-2.11A with:

10-19-12

work

Replace "Contract" in item 2 in the list in the 11th paragraph of section 86-2.11A with:

10-19-12

work

07-19-13

Delete the 12th paragraph of section 86-2.11A.

Replace section 86-2.11C with:

07-19-13

86-2.11C Electrical Service for Booster Pumps

Provide electrical service from the service point to the booster pump.

Furnish conductors, conduit, and pull boxes from the service point to the booster pump.

Do not use Type 3 conduit unless shown otherwise.

Replace section 86-2.14A with:

07-19-13

86-2.14A General

Deliver material and equipment for acceptance testing to either METS or a testing location as ordered.

Allow 30 days for testing. The Department notifies you when testing is complete. You must pick up the material or equipment from the test site and deliver it to the job site.

If material or equipment is rejected, allow 30 days for retesting. The retesting period starts when replacement material or equipment is delivered to the test site.

If material or equipment submitted for testing does not comply with the specifications, remove it within 5 business days after you are notified that the equipment is rejected. If equipment is not removed within that period, the Department may ship it to you and deduct the shipping cost.

Testing and quality control procedures for traffic signal controller assemblies must comply with NEMA TS standards for traffic control systems.

Replace the 2nd paragraph of section 86-3.02A(1) with:

07-19-13

The Department furnishes the BBS components under section 6-2.03.

Replace the 9th paragraph of section 86-3.02B with:

07-19-13

The couplings between the external cabinet and Model 332L cabinet must include a conduit for power connections between the 2 cabinets. Couplings must include:

1. 2-inch nylon-insulated steel chase nipple
2. 2-inch sealing steel locknut
3. 2-inch nylon-insulated steel bushing

07-19-13

Delete item 1.3 in the list in the 7th paragraph of section 86-3.04A.

Replace the 2nd paragraph of section 86-4.01A with:

07-19-13

The housing must not fail structurally as described in the following table:

Housing Structural Failure

Housing type	Test method	Description of structural failure
Metal	California Test 666	Fracture within the housing assembly or deflection of more than half the lens diameter of the signal section during the wind load test
Plastic	California Test 605	Fracture within the housing assembly or deflection of more than 10 degrees in either the vertical or horizontal plane after the wind load has been removed from the front of the signal face or deflection of more than 6 degrees in either the vertical or horizontal plane after the wind load has been removed from the back of the signal face

Replace the 1st sentence of section 86-4.01A(1) with:

07-19-13

Each metal housing must have a metal visor.

Replace the 1st sentence of section 86-4.01A(2) with:

07-19-13

Each plastic housing must be molded in 1 piece or fabricated from 2 or more pieces and joined into a single piece.

07-19-13

Delete item 1 in the list in section 86-4.01D(1)(b).

Replace the paragraphs in section 86-4.01D(1)(c)(i) with:

07-19-13

LED signal modules must be on the Authorized Material List for LED traffic signals.

The Department tests modules under section 86-2.14A, ANSI/ASQ Z1.4, and:

1. California Test 604 for LED and circular LED signal modules
2. California Test 3001 for arrow, U-turn, and bicycle LED signal modules

The LED signal modules submitted for testing must be typical production units. LEDs must be spread evenly across the module.

The Department may test the modules on all parameters specified in section 86-4.01D.

Replace the 1st and 2nd sentences of the 3rd paragraph of 86-4.01D(2)(b) with:

07-19-13

The electrical connection for each flashing LED signal module must be 4 secured, color-coded, jacketed copper wires. The wire must comply with the NEC.

Replace the heading of section 86-4.02 with:

07-19-13

PROGRAMMED VISIBILITY VEHICLE SIGNAL SECTION

Replace "face" in the 1st paragraph of section 86-4.02 with:

07-19-13

section

Add before the 1st sentence in section 86-4.03A:

07-19-13

The pedestrian signal face must be Type A.

Replace the 1st sentence of the 2nd paragraph of section 86-4.03B with:

07-19-13

The Department tests the pedestrian signal's front screen in a horizontal position with its edges supported.

07-19-13

Delete items 1 and 4 in the list in section 86-4.03I(1)(b).

Replace the paragraphs of section 86-4.03I(1)(c)(i) with:

07-19-13

The LED PSF module must be on the Authorized Material List for LED traffic signals.

The Department tests LED PSF modules under section 86-2.14A, ANSI/ASQ Z1.4, and California Test 606.

The LED PSF modules submitted for testing must be representative of typical production units.

The Department may test the modules on all parameters specified in section 86-4.03I.

Replace item 1 in the list in the 1st paragraph of section 86-4.03I(2) with:

07-19-13

1. Not include reflectors.

Replace item 6 in the list in the 1st paragraph of section 86-4.03I(2) with:

07-19-13

6. Be able to replace signal lamp optical units and pedestrian signal faces with LEDs.

Replace the table titled "Chromaticity Standards (CIE Chart)" in the 16th paragraph of section 86-4.03I(2) with:

07-19-13

Chromaticity Standards (CIE Chart)

Upraised hand	X: not greater than 0.659 or less than 0.600 Y: not greater than 0.390 or less than 0.331 Y= 0.990-X
Walking person	X: not greater than 0.440 or less than 0.280 Y: not greater than 0.0483 + 0.7917(X) or less than 0.0983 + 0.7917(X)

Add between "beacon" and "must" in the 1st sentence of section 86-4.05:

07-19-13

signal face

07-19-13

Delete "face" in item 1 in the list in the 1st paragraph of section 86-4.05.

Replace the row for viscosity in the table in the 2nd paragraph of section 86-5.01A(3)(c) with:

07-19-13

Viscosity, Brookfield Thermosel, no. 27 Spindle, 20 rpm, 190 °C	D 4402	2.5–3.5 Pa·s

Replace the paragraph in section 86-5.01A(3)(d) with:

07-19-13

Use epoxy sealant for repair work in and around sawcuts housing inductive loops.

Replace "all loop conductors" in the 3rd paragraph of section 86-5.01A(4) with:

07-19-13

the detector lead-in cable

Replace "Encase the loop wires" in the 1st sentence of the 3rd paragraph of section 86-5.01A(5) with:

07-19-13

The loop wires must be encased

Replace section 86-5.02 with:

07-19-13

86-5.02 PUSH BUTTON ASSEMBLIES

The housing for a push button assembly must be die-cast or permanent mold-cast aluminum. The assembly must be rainproof and shockproof in any weather condition.

The push button's switch must be a single-pole, double-throw switching unit with screw-type terminals rated 15 A at 125 V(ac). The switch must have:

1. Plunger actuator and a U frame to allow recessed mounting in the push button housing
2. Operating force of 3.5 lb
3. Maximum pretravel of 5/64 inch
4. Minimum overtravel of 1/32 inch
5. Differential travel from 0.002 to 0.04 inch
6. 2-inch minimum diameter actuator

Where a push button is attached to a pole, the housing must be shaped to fit the pole's curvature. Use saddles if needed to make a neat and secure fit.

Where a push button is mounted on top of a 2-1/2-inch-diameter post, fit the housing with a slip fitter and use screws to rigidly secure it to the post.

Install the push button and the sign on the crosswalk side of the pole.

Attach the sign on a Type B push button assembly.

For a Type C push button assembly, mount the instruction sign on the same standard as the assembly using 2 straps and saddle brackets.

Add to section 86-5:

07-19-13

86-5.03 ACCESSIBLE PEDESTRIAN SIGNAL

Reserved

Replace "lthe amp" in item 2 in the list in the 1st paragraph of section 86-6.01A(2) with:

07-19-13

the lamp

^^^

Replace the value for apparent size opening of nonwoven fabric in the table in the 1st paragraph of section 88-1.02E with:

01-20-12

0.012

Replace the table in the 1st paragraph of section 88-1.02G with:

01-20-12

Sediment Filter Bag

Property	Test	Values	
		Woven	Nonwoven
Grab breaking load, lb, 1-inch grip min, in each direction	ASTM D 4632	200	250
Apparent elongation, percent min, in each direction	ASTM D 4632	10	50
Water flow rate, gal per minute/sq ft min and max average roll value	ASTM D 4491	100-200	75-200
Permittivity, sec^{-1} min	ASTM D 4491	1.0	1.0
Apparent opening size, inches max average roll value	ASTM D 4751	0.023	0.012
Ultraviolet resistance, % min retained grab breaking load, 500 hr.	ASTM D 4355	70	70

Replace the table in the 1st paragraph of section 88-1.02H with:

01-20-12

Temporary Cover

Property	Test	Values	
		Woven	Nonwoven
Grab breaking load, lb, 1-inch grip min, in each direction	ASTM D 4632	200	200
Apparent elongation, percent min, in each direction	ASTM D 4632	15	50
Water flow rate, gal per minute/sq ft min and max average roll value	ASTM D 4491	4-10	80-120
Permittivity, sec^{-1} min	ASTM D 4491	0.05	1.0
Apparent opening size, inches max average roll value	ASTM D 4751	0.023	0.012
Ultraviolet resistance, % min retained grab breaking load, 500 hr.	ASTM D 4355	70	70

Replace section 88-1.02P with:

01-18-13

88-1.02P Biaxial Geogrid

Geosynthetics used for biaxial geogrid must be a punched and drawn polypropylene material formed into an integrally formed biaxial grid. When tested under the referenced test methods, properties of biaxial geogrid must have the values shown in the following table:

Biaxial Geogrid

Property	Test	Value
Aperture size, inch[a] min and max	Calipered	0.8-1.3 x 1.0-1.6
Rib thickness, inch min	Calipered	0.04
Junction thickness, inch min	Calipered	0.150
Tensile strength, 2% strain, lb/ft[a] min	ASTM D 6637	410 x 620
Tensile strength at ultimate, lb/ft[a] min	ASTM D 6637	1,310 x 1,970
Ultraviolet resistance, percent min retained tensile strength, 500 hours	ASTM D 4355	100
Junction strength, lb/ft[a] min	ASTM D 7737	1,220 x 1,830
Overall flexural rigidity, mg-cm min	ASTM D 7748	750,000
Torsional rigidity at 20 cm-kg, mm-kg/deg[b] min	GRI:GG9	0.65

[a]Machine direction x cross direction
[b]Geosynthetic Research Institute, Test Method GG9, *Torsional Behavior of Bidirectional Geogrids When Subjected to In-Plane Rotation*

Replace section 88-1.02Q with:

07-19-13

88-1.02Q Geosynthetic Bond Breaker

Geosynthetic bond breaker must be nonwoven; needle punched; not heat treated; polypropylene, polyethylene material.

When tested under the referenced test methods, properties of geosynthetic bond breaker material must have the values shown in the following table:

Geosynthetic Bond Breaker

Property	Test	Value
Mass per unit area, oz/sq yd min	ASTM D 5261	14.7
Thickness at 29 psi, mm min	ASTM D 5199	1.0
Tensile strength at ultimate, lbs/ft min	ASTM D 4595	685
Elongation, percent max	ASTM D 4595	130
Permittivity at 2.9 psi, m/s min	ASTM D 5493	0.0001
Hydraulic transmissivity at 29 psi, m/s min	ASTM D 6574	0.0002
Ultraviolet resistance, percent min retained grab breaking load, 500 hours	ASTM D 4355	60

^^

90 CONCRETE

07-19-13

Replace the 3rd paragraph of section 90-1.01C(7) with:

08-05-11

Submit weighmaster certificates in printed form or, if authorized, in electronic media. Present electronic media in a tab-delimited format on a CD or DVD. Captured data for the ingredients represented by each batch must be line feed carriage return and one line separate record with sufficient fields for the specified data.

Replace the 3rd paragraph of section 90-3.01C(5) with:

08-05-11

Production data must be input by hand into a pre-printed form or captured and printed by the proportioning device. Present electronic media containing recorded production data in a tab-delimited format on a CD or DVD. Each capture of production data must be followed by a line feed carriage return with sufficient fields for the specified data.

Replace the 1st paragraph of section 90-4.01A with:

07-19-13

Section 90-4 includes specifications for fabricating PC concrete members.

Replace the paragraphs in section 90-4.01C with:

07-19-13

90-4.01C(1) General

For reports and logs, type or clearly print the name next to the signature of the person signing the report or log.

Submit expansion test data under section 90-4.02, if required.

90-4.01C(2) Certificates of Compliance

Submit a certificate of compliance for the cementitious material used in PC concrete members. The certificate must be signed by the PC concrete product manufacturer.

Submit a certificate of compliance for each PC concrete member. The certificate of compliance for tier 1 and tier 2 members must be signed by the QC manager. The certificate of compliance for tier 3 members must be signed by the QC Inspector.

90-4.01C(3) Precast Concrete Quality Control Plan

Before performing any precasting activities for tier 1 and tier 2 PC concrete members, submit 3 copies of the project-specific QC plan for the PC plant. The QC plan must supplement the information from the authorized facility audit. Submit a separate QC plan for each plant. Allow 25 days for review.

Each project-specific QC plan must include:

1. Name of the precasting plant, concrete plants, and any testing laboratory to be used.
2. Manual prepared by the precasting plant that includes:
   - 2.1. Equipment description
   - 2.2. Testing procedures
   - 2.3. Safety plan
   - 2.4. Personnel names, qualifications, and copies of certifications
3. QC manager and QC inspector names, qualifications, and copies of certifications.
4. Organizational chart showing QC personnel and their assigned QC responsibilities.
5. Methods and frequencies for performing QC procedures including inspections, material testing, and any survey performed for all components of PC concrete members. Components include prestressing, concrete, grout, reinforcement, steel, miscellaneous metal, and formwork.
6. System for reporting noncompliant PC concrete members to the Engineer.

7. System for identification and tracking repairs and repair methods.
8. Procedure for the reinspection of repaired PC concrete members.
9. Forms for certificates of compliance, daily production logs, and daily reports.

Submit a revised QC plan for any changes to:

1. Concrete plants
2. Material sources
3. Material testing procedures
4. Testing laboratory
5. Procedures and equipment
6. Updated systems for tracking and identifying PC concrete members
7. QC personnel

After authorization, submit 7 copies of each authorized QC plan and make 1 copy available at each location where work is performed.

Allow 7 days for review of a revised QC plan.

90-4.01C(4) Daily Production Log

The QC inspector must provide reports to the QC manager for each day that precasting activities are performed.

The QC manager must maintain a daily production log of PC activities for each day's precasting. PC activities include setting forms, placing reinforcement, setting prestressing steel, casting, curing, post tensioning, and form release. This daily log must be available at the precasting plant. The daily log must include:

1. Plant location
2. Specific description of casting or related activities
3. Any problems or deficiencies discovered
4. Any testing or repair work performed
5. Names of QC inspectors and the specific QC inspections they performed that day
6. Reports for that day's precasting activities from each QC inspector including before, during, and after precast inspections

Immediately notify the Engineer when any precasting problems or deficiencies are discovered, and submit the proposed repair or process changes necessary to correct them.

90-4.01C(5) Precast Concrete Report

Before shipping PC concrete members, submit a PC concrete report. The report must include:

1. Reports of all material tests and any survey checks
2. Documentation that:
   2.1. You have evaluated all tests
   2.2. You corrected all rejected deficiencies
   2.3. Repairs have been reexamined with the required tests and found acceptable
3. Daily production logs
4. Certificates of compliance
5. Documentation of inspections

Each person who performs a material test or survey check must sign the corresponding report and submit the report directly to the QC manager.

Replace the paragraphs in section 90-4.01D with:

07-19-13

90-4.01D(1) General

Quality control and assurance for PC concrete includes:

1. Your QC program
2. Department's acceptance of PC concrete members

PC concrete members are categorized into the following 4 tiers:

1. Tier 1 consists of:
   1.1. Components of bridge structures, including girders, deck panels, bent caps, abutments, slabs, closure wall panels, and piling
   1.2. Prestressed pavement
2. Tier 2 consists of:
   2.1. Components of earth retaining systems
   2.2. Wingwalls
   2.3. Types A, B, and C pipe culvert headwalls, endwalls, and wingwalls
   2.4. Pavement
   2.5. Box culverts
   2.6. Sound wall panels and supports
3. Tier 3 consists of:
   3.1. Pipes
   3.2. Pipe drainage facilities
   3.3. Straight and "L" pipe culvert headwalls except those listed under tier 2
   3.4. Drainage Inlets
   3.5. Flared end sections
4. Tier 4 consists of any member not described as tier 1, tier 2, or tier 3

90-4.01D(2) Quality Control

90-4.01D(2)(a) General

For tier 1 and tier 2 PC concrete members:

1. Fabricate PC concrete members at a plant on the Authorized Facility Audit List
2. Assign a PC concrete QC manager to the plant
3. Assign a QC inspector who is either registered as a civil engineer in the State or:
   3.1. For tier 1, has a Plant Quality Personnel Level II certification from the Precast/Prestressed Concrete Institute
   3.2. For tier 2, has a Plant Quality Personnel Level I certification from the Precast/Prestressed Concrete Institute
4. Prepare a PC concrete QC plan
5. Perform PC concrete materials testing
6. Maintain a daily production log
7. Prepare a PC concrete report
8. Prepare a certificate of compliance

For tier 3 PC concrete members:

1. Assign a QC inspector who has one of the following qualifications:
   1.1. Registration as a civil engineer in the State.
   1.2. Plant Quality Personnel, Level I certification from the Precast/Prestressed Concrete Institute.
   1.3. Competency to perform inspection of PC operations. An inspector is competent if the individual has completed training or has experience in PC operations and inspection.
2. Prepare a certificate of compliance

For tier 4 PC concrete members, prepare a certificate of compliance.

For each ASTM test method specified in this section, the material's test result must comply with the requirement specified for the comparable test in section 90 unless otherwise specified.

If curing compound is used, provide certificate of compliance as specified in section 90-1.01C(5).

If PC concrete is manufactured at an established PC concrete plant, a trial batch and prequalification of the materials, mix proportions, mixing equipment, and procedures under section 90-1.01D(5)(b) are not required.

90-4.01D(2)(b) Quality Control Meeting

After submitting the PC concrete QC plan, hold a meeting to discuss the requirements for PC concrete QC. The meeting attendees must include the Engineer, the PC concrete QC manager, and a representative from each plant performing PC concrete activities for the Contract.

90-4.01D(2)(c) Sampling, Testing, and Inspecting

The QC laboratory testing personnel or the QC inspector must witness sampling. The QC laboratory testing personnel must perform testing.

QC laboratory testing personnel must have the following certifications, as applicable:

1. ACI Strength Testing Technician
2. ACI Concrete Laboratory Testing Technician Level 1
3. ACI Aggregate Testing Technician Level 2

The QC Inspector must perform inspections before, during, and after casting is complete.

QC field testing and inspection personnel must have an ACI Concrete Field Testing Technician, Grade I certification.

For each mix design used for tier 1 and tier 2 PC concrete members, perform sampling and testing at the minimum frequencies shown in the following tables:

Aggregate QC Tests

Property	Test method	Minimum testing frequency
Aggregate gradation	ASTM C136	Once per 400 cu yd of concrete cast or once a week, whichever is more frequent
Sand equivalent	ASTM D2419	
Percent fines under 75 microns[a]	ASTM C117	
Moisture content of fine aggregate	ASTM C566, or electronically actuated moisture meter[b]	1–2 times per each day of pour, depending on conditions

[a]Percent fines under 75 microns test replaces the cleanness test in section 90-1.02C with the requirements of 1.5 percent maximum for "Operating Range" and 2.0 percent maximum for "Contract Compliance." The 5th paragraph of section 90-1.02C(2) does not apply.
[b]Electronically actuated moisture meter must be calibrated once per week per ASTM C566.

Concrete QC Tests

Property	Test method	Minimum testing frequency
Compressive strengthb	ASTM C172/C172M, ASTM C31/C31M, and ASTM C39/C39M	Once per 100 cu yd of concrete cast, or every day of casting, whichever is more frequent
Slump	ASTM C143/C143M	
Temperature	ASTM C1064/C1064M	
Density	ASTM C138	Once per 600 cu yd of concrete cast or each week of batching, whichever is more frequent
Air content	ASTM C231/C231M or ASTM C173/C173M[a]	If concrete is air entrained, once for each set of cylinders, and when conditions warrant

[a]ASTM C173/C173M must be used for lightweight concrete.
[b]Cylinders must be 6 by 12 inches.

If concrete is batched at more than 1 plant, perform the tests at each plant.

Cure test cylinders for determining time of prestressing loading in the same manner as the concrete in the member.

Cure test cylinders for determining compliance with 28-day strength requirements in the same manner as the member until completion of the steam curing process followed by a water bath or moist room at 60 to 80 degrees F until tested.

For PC concrete that is steam cured, concrete designated by compressive strength is acceptable if its compressive strength reaches the described 28-day compressive strength in no more than the maximum number of days specified or allowed after the concrete is cast.

90-4.01D(3) Quality Assurance

For PC concrete that is steam cured, the Engineer evaluates the compressive strength based on individual tests representing specific portions of production.

Add between the 1st and 2nd paragraphs of section 90-4.02:

07-19-13

PC portland cement based repair material must be on the Authorized Material List.

If municipally supplied potable water is used for PC concrete, the testing specified in section 90-1.02D is waived unless requested.

Add to section 90-4.03:

07-19-13

For dimensional tolerances of PC concrete members, comply with the Precast/Prestressed Concrete Institute Concrete Institute's *Tolerance Manual for Precast and Prestressed Concrete Construction, MNL 135-00*.

For tier 1 and tier 2 PC concrete members, apply curing compound using power-operated spraying equipment. You may request application by hand spraying for small quantities of PC concrete members. For tier 3 and tier 4 PC concrete members, the application of curing compound may be hand sprayed.

Replace the item 2 in the list in the 2nd paragraph of section 90-4.03 with:

07-19-13

2. To prevent moisture loss on the exposed surfaces during the presteaming period, cover the concrete as soon as possible after casting or keep the exposed surfaces wet by fog spray, curing compound, or wet blankets.

^^

91 PAINT

10-19-12

Add to section 91-2:

10-19-12

91-2.03 MOISTURE-CURED POLYURETHANE COATING

Reserved

Replace "saint" in the 1st paragraph of section 91-4.05 with:

10-19-12

paint

^^

92 ASPHALTS

07-19-13

Replace "Reserved" in section 92-1.01B with:

07-19-13

modified asphalt binder: Asphalt binder modified with polymers, crumb rubber, or both.

Replace the row for dynamic shear for original binder in the table in the 1st paragraph of section 92-1.02B with:

01-20-12

Dynamic shear, Test temperature at 10 rad/s, ℃	T 315	58	64	64	64	70
min G*/sin(delta), kPa		1.00	1.00	1.00	1.00	1.00
max G*/sin(delta), kPa		2.00	2.00	2.00	2.00	2.00

Replace 2nd paragraph of section 92-1.02B with:

07-19-13

PG modified asphalt binder must comply with the requirements shown in the following table:

PG Modified Asphalt Binder

Property	AASHTO Test Method	Grade		
		PG 58–34 M	PG 64–28 M	PG 76–22 M
Original Binder				
Flash point, min °C	T 48	230	230	230
Solubility, min %	T 44[a]	97.5	97.5	97.5[b]
Viscosity at 135 °C[c], max, Pa·s	T 316	3.0	3.0	3.0
Dynamic shear, Test temperature at 10 rad/s, °C min G*/sin(delta), kPa	T 315	58 1.00	64 1.00	76 1.00
RTFO test[d], Mass loss, max, %	T 240	1.00	1.00	1.00
RTFO Test Aged Binder				
Dynamic shear, Test temperature at 10 rad/s, °C min G*/sin(delta), kPa	T 315	58 2.20	64 2.20	76 2.20
Dynamic shear, Test temperature at 10 rad/s, °C max (delta), degree	T 315	80[e]	80[e]	80[e]
Elastic recovery[f], Test temperature °C min recovery, %	T 301	25 75	25 75	25 65
PAV[g], temperature, °C	R 28	100	100	110
RTFO Test and PAV Aged Binder				
Dynamic shear, Test temperature at 10 rad/s, °C max G*sin(delta), kPa	T 315	16 5000	22 5000	31 5000
Creep stiffness, Test temperature, °C max S-value, MPa min M-value	T 313	-24 300 0.300	-18 300 0.300	-12 300 0.300

[a]The Department allows ASTM D 5546 or ASTM D 7753 instead of AASHTO T 44. Particles recovered from ASTM D 5546 or ASTM D 7753 or AASHTO T 44 must be less than 250 µm.
[b]Report only for spray application.
[c]The Engineer waives this specification if the supplier provides written certification the asphalt can be adequately pumped and mixed at temperatures meeting applicable safety standards.
[d]"RTFO Test" means the asphaltic residue obtained using the Rolling Thin Film Oven Test, AASHTO Test Method T 240 or ASTM D 2872. The residue from mass change determination may be used for other tests.
[e]Test temperature is the temperature at which G*/sin(delta) is 2.2 kPa. A graph of log G*/sin(delta) plotted against temperature may be used to determine the test temperature when G*/sin(delta) is 2.2 kPa. A graph of (delta) versus temperature may be used to determine delta at the temperature when G*/sin(delta) is 2.2 kPa. The graph must have at least two points that envelope G*/sin(delta) of 2.2 kPa and the test temperature must not be more than 6 degree C apart. The Engineer also accepts direct measurement of (delta) at the temperature when G*/sin(delta) is 2.2 kPa.
[f]Tests without a force ductility clamp may be performed.
[g]"PAV" means "Pressure Aging Vessel."

Do not modify PG modified asphalt binder using polyphosphoric acid.

Crumb rubber must be from automobile and truck tires and must be free from contaminants including fabric, metal, minerals, and other nonrubber substances.

PG modified asphalt binder modified with crumb rubber must be homogeneous and must not contain visible particles of crumb rubber.

The supplier of PG modified asphalt binder modified with crumb rubber must:

1. Report the amount of crumb rubber by weight of asphalt binder
2. Certify a minimum of 10 percent of crumb rubber by weight of asphalt binder

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93 LIQUID ASPHALTS

07-19-13

Replace "Celsius" the 1st row in the table in the 8th paragraph of section 93-1.04 with:

07-19-13

Fahrenheit

^^

94 ASPHALTIC EMULSIONS

03-21-14

Replace the 1st paragraph of section 94-1.04 with:

03-21-14

Asphaltic emulsion is measured by weight under the specifications requiring its use. If water is added to the asphaltic emulsion, the quantity of asphaltic emulsion is determined before the addition of water.